#### ULTIMATE STRENGTH ANALYSIS OF STEEL JACKET PLATFORM

By

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#### FINAL PROJECT REPORT

Submitted to the Civil Engineering Programme in Partial Fulfillment of the Requirements for the Degree Bachelor of Engineering (Hons) (Civil Engineering)

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# CERTIFICATION OF APPROVAL

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A project dissertation submitted to the Civil Engineering Programme Universiti Teknologi PETRONAS in partial fulfilment of the requirement for the BACHELOR OF ENGINEERING (Hons) (CIVIL ENGINEERING)

Approved by,

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June 2010

# **CERTIFICATION OF ORIGINALITY**

This is to certify that I am responsible for the work submitted in this project, that the original work is my own except as specified in the references and acknowledgements, and that the original work contained herein have not been undertaken or done by unspecified sources or persons.

AHMAD FAWWAZ BIN AHMAD SOHAIMI

#### ABSTRACT

Currently, there are about more than 80% of offshore platform around Malaysia's block field aged 30-40years which beyond the original design life of 25 years. With the several numbers of the platform services beyond the original design life, structural assessments need to conduct the gauge platform performance throughout for the extended years. There was 2 common method widely used, simplified ultimate strength analysis and static pushover analysis. Simplified ultimate strength defined as when any of member, joint, pile steel strength and pile soil bearing capacity reaches its ultimate capacity. That result the overview of the platform ultimate strength. Static pushover analysis generally concentrates on RSR (Reserve Strength Ratio) and REF (Reserve Resistance Factor) for the ultimate strength. The report summarizes two parts of analysis, first the study of ultimate strength of different leg jacket platform and the second part is the bracing configuration study. The analyses were a non-linear analysis where the load will distribute to an alternative of the steel framework until the structure collapse under allocated condition. It is found that a platform with more legs has higher ultimate strength compared to less number of legs. Hence a bigger jacket platform with eight legs has much stiffen than smaller platform and mostly installed at rough area. Another part of analysis of bracing configuration study where X-bracing contributes highest rigidity to the whole platform by retaining the platform until the highest load reported compare to another configuration.

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# LIST OF ABBREVIATIONS

RON	Research Octane Number
SACS	Structural Analysis Computer System
API	American Petroleum Institute
AISC	American Institute of Steel Construction
PTS	Petronas Technical Specification
WSD	Working Stress Design
MSL	Mean Sea Level
FOS	Factor of Safety
ULR	Ultimate Linear to Ratio
LRFD	Load Resistance Factored Design
SUS	Simplified Ultimate Strength
RSR	Reserve Strength Ratio
REF	Reserve Resistance Factor
RF	Redundancy Factor
DSR	Damage Strength Rating
FE	Finite Element
Hmax	Individual Maximum Wave Height
Tass	Associated Wave Period for Hmax
HAT	Highest Astronomical Tides
LAT	Lowest Astronomical Tides
MSI	Musculoskeletal injuries
HSE	Health, Safety & Environment
OSHA	Occupational Safety Health Act

# CHAPTER 1 INTRODUCTION

#### **1.0 BACKGOUND OF STUDY**

Offshore structure used for oil and gas extraction under the seabed. Common functions provide a safe, dry working environment for the equipment and personnel who operate the platform. Offshore structures are of 2 categories namely fixed platform and floating platform. Examples of a fixed platform are, steel-jacket platform, jack-up and compliant tower while example of floating platform are spar, semi-submersible and FPSO. These platforms been designed for criteria location for the design life. But many platforms in Malaysia aged about 30-40years old. As example, PETRONAS platform located offshore Kerteh which has been under operation for about 30 years and some of the very early platforms are still in service. Over the last 10 years or so, various structural integrity assessments have been carried out on the platforms to gauge its safety and usability. Some of these platforms have been analyzed using pushover analysis while others have not been analyzed at all. It is the intention of this research to analyze some of the platforms, which have not been analyzed in detail, in order to define maintenance and up gradation requirements for their continued utilization. Obtaining latest metocean data and related SACS input file (model) for different type of jacket platform in the Malaysian region from RNZ Integrated(M) Sdn. Bhd. Using different type of jacket platform, analyze for ultimate strength and further research on the reliability of the existing structure.

#### **1.1 PROBLEM STATEMENT**

The increasing of oil price and demand has lead to the increasing oil production. The oil companies are competing for these purposes. They have expanded their aged jacket platform to certain year to extract oil that still remains under the seabed. Various structural integrity assessments carried out to check for platforms performance throughout the expanded design life.

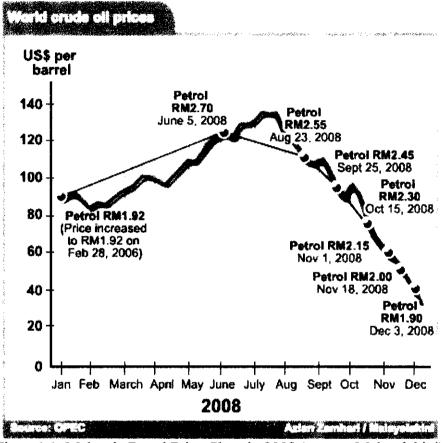


Figure 1.1: Malaysia Petrol Price Chart in 2008 (source: Malaysiakini)

As shown in figure how a price and demand of crude oil of the worldwide. The statistics nowadays shows that the demand of oil as the primary sources of power rely boost up the price and also demand. The trend illustrate by the figure were affected by the political issues regarding the oil price in Malaysia. Looking at the rough picture, today the oil price for the RON 97 had been increased to RM 2.00/ litre. For the worldwide, the oil price suspected will increase to certain number due to the reserve oil block field. As result, the offshore structures with over design life are still in serving of extracting oil and gas to cater for the demand.

With the current oil price worldwide drop to certain number, an exploration of a new oil reservoir is costly compared with the maintaining the existing structure. In managing the cost expenditure, some oil operators spend upon the maintenance of existed old platform for extracting process. This competitive pressure and regulatory constrain are placing increasing demand on effective ultimate strength analysis method develop by researcher to meet with the demand. A study of progressive collapse load upon selected platform in order to study the behaviour of a different legged platform for data comparison.

#### **1.2 OBJECTIVES OF THE PROJECT**

- The develop SACS input file and collapse input file for ultimate strength check of an offshore structure in the Malaysian region.
- To evaluate and compare the ultimate strength of different legged platform.
- To evaluate the differences in term of bracing framework with respect to collapse loading.

#### **1.3 SCOPE OF PROJECT**

The scopes of studies involved SACS modeling of existing structure to check for structural integrity. The scope of the project relies on module below:

- Performing SACS Full Plastic Collapse Analysis
- API, 'Recommended Practice for Planning, Designing and Constructing Fixed Offshore Platforms - Working Stress Design', API RP2A-WSD 21<sup>st</sup> Edition, December 2000.
- Petronas, 'Design of Fixed Offshore Structures', PTS 20.073, December 1983 and 'Supplementary to PTS 20.073', Rev. 4, August 2005.
- AISC, 'Manual of Steel Construction Allowable Stress Design', AISC-ASD 9<sup>th</sup> Edition 1989.

All the environmental conditions which includes wave, wind and current conforming to the selected load cases and combinations used in the original design report shall be reviewed and updated based on the latest available metocean data. Through non-linear analysis of SACS Collapse module, evaluate the difference between working stress design and load resistance factor design. Design limitation based on data received from RNZ (M) Integrated Sdn. Bhd. Metocean data and other relevant input follow as per design. SACS input file were retain as per design basis. Only minor command introduce in the file for the purpose of non-linear analysis of Full Plastic Collapse Analysis.

Reference code of API RP2A-WSD, PTS 20.073 and AISC utilizes in part of modelling with the update information. The code provide a reference in load factor, member and joint design, environmental data, corrosion study and other related information regarding jacket structure.

#### **1.4 RELEVANCY OF THE PROJECT**

This project is relevant to the study of Design of Offshore Structure as well as the study of Ocean and Coastal Engineering. This project is also relevant to the recent issued regarding the oil industry in the country.

The project is feasible as it utilizes a program called SACS 5.2 Executive (Structural Analysis Computer System) and analyzes the data which can be obtained from the projects "Provision for Structural Integrity and Spectral Fatigue Analysis for Five (5) Platforms for Petronas Carigali Sdn Bhd Peninsular Malaysia Operations (PCSB-PMO)". Microsoft Excel as a tool for other type of formulation, in term of wave attack angle, wind speed computation and output data synchronization for the purpose of user friendly.

# LITERATURE REVIEW

**CHAPTER 2** 

#### **2.0 INTRODUCTION**

Review for the study was taken abundantly from journals, books and the internet. Basically, spot to be highlighted for the study of ultimate strength or capacity of a steel jacket platform. Here are some notes taken for the study:

# 2.1 GENERAL INFORMATION OF JACKET PLATFORM

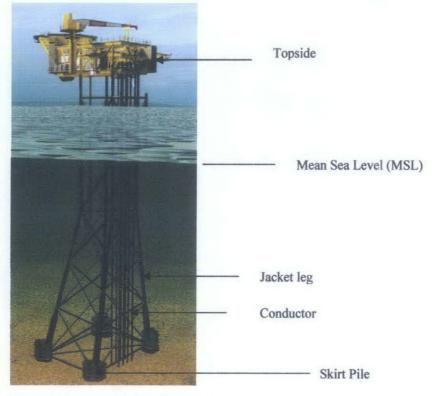


Figure 2.1: Typical Jacket Platform

The figure illustrate example of jacket platform with 4-legged. Majority of the structure installed in shallow water area (30m-500m). The platform categorize into 2 parts:

- Substructure
- Superstructure

Substructure located below mean sea level (MSL) and consists of jacket leg, member, trusses, conductor, anode, caissons and more according to its design. Superstructure for above MSL where locate main deck, cellar deck, helideck, equipment and more with reference to platform function.

#### 2.1.1 Substructure (Jacket)

The jacket (substructure) provide protective layer around the pipes for oil extraction from under the seabed. The jackets also serve as template for initial deriving of pile (the pile driven through the jacket leg).Jacket platform are consists of an open tubular steel space-frame construction and supported by file foundation. Jackets, the tower-like braced tubular structures, generally perform two functions:

The jacket takes loadings from environmental and topside and transfer the load the foundation through pile installed within the jacket leg. The size of jacket leg varying from 11m - 20m diameter to cater for the design load at different depth and location. Jacket platform differentiate through its leg. Basically there were three (3), four (4), six (6), eight (8) and sixteen (16) legs. Bracing within the jacket structure designed to cater for load paths and responsible for the structure redundancy. The world record was the Shell's Bullwinkle platform installed in 1991 with water depth of 412m. The installation methods for the jacket and the piles have a profound impact on the design.

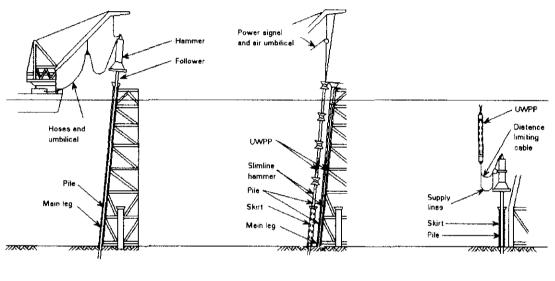
#### 2.1.2 Pile Foundation

The jacket foundation is provided by open-ended tubular steel piles, with diameters up to 2m. The piles are driven approximately 40-80 m, and in some cases 120 m deep into the seabed. The piles driven depend on the soil types at the area, deeper penetration needed for softer soil to avoid settlement of the platform foundation.

Difference design and types specify for each speciality. Generally there were three (3) main design of piling use in the industry. See Figure 2.2 for the illustration of piling. The three type of piling as follows:

Pile-through-leg	Pile inserted within the jacket leg member. The piles penetrate into the soil through jacket leg by hammering from the tip of jacket leg member.
Skirt piles	Pile is installed in guides attached to the jacket leg. Skirt piles can be grouped in clusters around each of the jacket legs.
Vertical skirt piles	Directly installed in the pile sleeve at the jacket base; all other guides are deleted

Table 1: Pile description



A Conventional above-water

8 New underwater techniques C Free-riding subsea

technique

Figure 2.2: Jacket foundation types with conventional and new pile – driving technique

#### 2.1.3 Corrosion Protection

For below MSL, a sacrificial anode (approximate 3 KN each) consists of a zinc/aluminium bar cast about a steel tube and welded on to the structures cathodic protection. As for design, total anodes weight attaches approximately about 5% of the self weight jacket steelwork. Wall thickness of jacket leg at splash zone increase by 12mm to cater corrosion effect due to air and sea water at splash zone, approximately in range of (-3m till 3m) of MSL.

#### 2.1.4 Topsides

Topside or superstructure located above mean sea level with an appropriate air gap. The structure supported by jacket leg connected to the top of piles which extend to seabed and driven into soil for rigidity. The structure made of tubular steel, wide flange, plate girder and other steel member properties. Located equipment for mean of functions listed as follows:

- well control
- support for well work-over equipment
- separation of gas, oil and non-transportable components in the raw product,
- support for pumps/compressors required to transport the product ashore
- power generation
- Accommodation for operating and maintenance staff (manned platform)

Topsides design characterize by two(2) difference properties, which integrated and modularized topside which are positioned either on jacket leg.

Various structural integrity assessments have been carried out upon the jacket platforms around offshore Malaysia field block to gauge its safety and usability for the extended service. Some of these platforms have been analyzed to pushover status to gauge its performance. Research done by several individual or parties result in varies method of determining an aged platform performance. Therefore the literature review discusses the analysis or methods develop by researcher to get the ultimate capacity of a platform. Those analyses are as below:-

- Simplified ultimate strength analysis
- Static Pushover Analysis

#### 2.2 SIMPLIFIED ULTIMATE STRENGTH ANALYSIS

Assessment for an aged structure involves analysis of design basis check, design level analysis and ultimate strength analysis. Checking for the ultimate strength with respect to API RP2A. The indication such as excessive deformation or resistance to total collapse may provide better measure to judge the structure integrity. The structure strength determine from static pushover analysis and cyclic loading for severe storm condition. SACS Collapse program relate the deflection, direct stiffness to solve for geometric and material non-linearity associated with the ultimate load capacity of a structure.

API RP2A-LRFD develop based on reliability based calibration which the platform checked for combined action of extreme wave (storm condition), current and wind that account for joint probability off-occurrence. Define partial FOS=1.35 for the condition. Computed the wave forces with respect to the drag and inertia coefficients (Cd and Cm):

Smooth Cd = 0.65, Cm = 1.60Rough Cd = 1.05, Cm = 1.20

The code also gives equations for calculating load-resistance factor for cylindrical members under tension, compression, bending, shear and etc, including combine loads. The load resistance factor for combined axial tension and bending can be calculated using the provide equation:

$$1 - \cos\left[\frac{\{\Pi(f1)\}}{\{2\phi Fy\}}\right] + \frac{[(fby)^2 + (fbz)^2]^{1/2}}{(\phi b Fbn)} \le 1.0$$

Where,

fby = bending stress about member y-axis (in-plane)

fbz = bending stress about member z-axis (out-plane)

Fbn = nominal bending

 $F_y = yield strengths$ 

Ft = axial tensile stress

 $\Phi_t = \emptyset$  resistance factor for axial tensile strength (=0.95)

 $\Phi_b$  = resistance factor for bending strength (=0.95)

The load-resistance factor for combined axial compression and bending can be calculated from the equations.

$$\frac{(fc)}{(2\phi c F cn)} + \left\{\frac{1}{(2\phi b F bn)}\right\} \left[\left\{\frac{(Cmy fby)}{\left(1 - \frac{fe}{(\phi c F ey)}\right)}\right\}^2\right]^{\frac{1}{2} \le 1.0}$$

And

$$1 - \cos\left[\frac{\{\Pi(f1)\}}{\{2\phi Fy\}}\right] + \frac{[(fby)^2 + (fbz)^2]^{1/2}}{(\phi b Fbn)} \le 1.0$$

 $Fc < \phi c Fxc$ 

Where,

 $C_{my}$  = reduction factor corresponding to the member y-axis  $C_{mz}$  = reduction factor corresponding to the member z-axis  $F_{ey}$  = Euler buckling strength corresponding to the member y-axis  $F_{ez}$  = Euler buckling strength corresponding to the member z-axis  $F_{ey}$  = Fy /  $\lambda y^2$   $F_{ez}$  = Fz /  $\lambda z^2$   $\lambda$  = column slenderness parameter for member in respective axes  $F_{cn}$  = nominal axial compressive strength  $F_c$  = axial compressive strength ue to factored load  $\Phi c$  = resistance factor for axial compressive strength, 0.85

The load resistance factor should be less than equal to 1.0. The equations for strength checks of tubular joints are also given in API RP2A-LRFD.

For assessment of existing platforms, the criteria depend on the category of the platform, which consider life safety, and consequences of failure. Krieger, *et al* has recommended two factors for ultimate strength checks for existing platforms namely:

- Ultimate to Linear Ratio (ULR)
- Reserve Strength Ratio (RSR)

ULR defined that a ratio of the ultimate resistance load to that causing a unity check of 1.0 in the original design and RSR defined as the ration of the ultimate strength load to the 20<sup>th</sup> edition (100-year) design load. For manned platforms with or without significant environmental impact, a ULR of 1.8 and RSR of 1.6 are recommended,

while platforms of minimum consequence a ULR of 1.6 and RSR of 0.8 are recommended

Simplified Ultimate Strength (SUS) is generally estimated based on the smallest of the four base shear values obtained when the first of the following component classes reach its ultimate capacity:-

- a. joints
- b. members
- c. pile steel strength
- d. pile soil bearing capacity

The platform base shear values that satisfy each of these conditions are determined from a linear analysis by using respective API RP2A-LRFD equations with the load and resistance.

In simplified approach, a linear static global analysis of the structure is performed for forces due to the combined action of gravity loads and extreme wave loads (100-year return period) and associated current and wind effects. The structure is loaded with series of monotonically increasing environmental load conditions from all directions of interest. Member and joint forces are obtained from the analysis and for each load condition the strength checks are made for the members, joint and etc using API RP2A-LRFD. The load is increased after each stage until any component of the structure fails or reaches its ultimate strength. The platform attains ultimate strength when any member or joints reach its ultimate capacity. The first member/joint failure is obtained and the load factor corresponding to this is calculated as ratio of the base shears corresponding to the first member failure and the 100-year environmental load. The analysis is further performed by removing the failed member from the model, if alternative load paths are available to bypass a failed member. The analysis is terminated when there is no alternative load path or deformation of the structure exceeds beyond a limit from a functional considerations. The reserve strength ratio is then calculated as the ratio of the base shears corresponding to collapse load and first member failure. Full ultimate strength analysis using non-linearity can be restored to if the simplified ultimate strength analysis does not meet the requirements for regualification.

The analysis conducted upon API RP2A-LRFD that recommends using linear wave theory and Morrison equation to conduct series of calculation regarding wave and current to the structural member. Yield stress of steel member retaining as per design basis requirement. Base shear for first member failure obtained from each attack angle to get the factor of first member failure, factor for collapse load and reserve strength ratio. The output data synchronize into several categories listed as follows:

- Lateral load for 100-year storm condition
- First member failure load, Pmf
- Factor for first member failure
- Collapse load, Pu
- Factor for collapse load
- Deformation corresponding to Pmf
- Deformation corresponding to Pu
- Reserve strength ratio

All the values taken from the output data expect for factor and ratio where the values originate from respective value. The formulas for the factors given as follows:

Factor for first member failure =  $\frac{First member failure load, Pmf}{Lateral load for 100 - year storm condition}$ 

Factor for collapse load =  $\frac{Collapse load, Pu}{Lateral load for 100 - year storm condition}$ 

Reserve strength ratio = 
$$\frac{Factor for collapse load}{Factor for first member failure}$$

Another approach proposed by Vannan et al where a linear static in place analysis done by performing increasing environmental loading until first member or other component failure occurs. Unity check reported above 1.0 allocated as the ultimate strength of the structure. Other simplified methods introduce by Bea and Mortazavi prove to be reasonable estimates platform load capacity relative to results obtains from detailed static pushover analysis.

#### 2.3 STATIC PUSHOVER ANALYSIS

A research conducted regarding the response of jacket structure to any subjected load especially extreme condition (100-year return period storm wave) to estimate to ultimate strength of tubular framed structure. In order word the check for reserve capacity of the structure. An elastic frame analysis is performed, typically with the elements rigidly connected. November 1993, an API preliminary draft for RP 2A-WSD Section 17.0 for the assessment of existing platforms was circulated, in which a sequence of analysis from screening, through design level to ultimate strength assessment is advocated to demonstrate structural adequacy. At the ultimate strength level it is proposed that a platform may be assessed using inelastic, static pushover analysis.

The research begins with Lloyd and Clawson (1984) discussing sources of reserve and residual strength of 'frame behaviour'. Continue on with Marshall (1979) entitled behaviour of elastic element and ultimate strength system. Marshall and Bea (1976) demonstrated reserve safety factor and Kallaby and Millman (1975) for inelastic analysis energy absorption capacity of the Maui A platform under earthquake loading. Recent investigation shows that static pushover analysis generally suffices to demonstrate a structure's resistance to the cyclic loading of the full storm.

Trends for lighter, lift able jackets and new concepts for deepwater provide additional impetus to the study. Fewer members in the splash zone may increase the risk to topsides safety in the event of impact, and the deletion of members with the low elastic utilisations to save weight reduces the capacity for redistribution structural configuration along the alternative load paths. Comparative calculation of reserve capacity for different structural configurations can help ensure that levels of reserve strength and safety embodied within the older designs are maintained. Therefore there has been requirement to develop an understanding and the corresponding analytical tools to be able to predict system reserves beyond individual component failure capacities, in order to demonstrate integrity in the event of such extreme loading scenarios occurring. Reserve strength defined as the ability of structure to sustain loads in excess of the design value. Introduce the term RSR (Reserve Strength Ratio) (Titus and Banon, 1988) and REF (Reserve Resistance Factor) (Lloyd and Clawson, 1984).

$$RSR = \frac{Ultimare\ Platform\ Resistance}{Design\ Load}$$
$$REF = \frac{Environmental\ Load\ at\ Collapse\ (undamaged)}{Design\ Environmental\ Load}$$

Fixed offshore structure spread load to a network of path result that a failure at a single member does not necessary lead to catastrophic structural collapse. Measuring redundancy explain in 2 ways, redundancy factor (RF) and damaged strength rating (DSR). Measurements are load case dependent and any structure may exhibit very different redundancy properties for different loading direction.

Reserve strength evaluated by applying the maximum loading from the extreme event and performing 'pushover' analysis. For and extreme storm, the environmental loading is cyclic, imposed on an underlying dominant direction. The maximum wave is unlikely to be an isolated event, but will be a peak in series of extreme loads. The possibility of cyclic degradation of components which have failed, or approaching failure even though overall structure resistance may remain adequate, therefore needs to be considered. Basic ideas of static pushover analysis is where a single load is applied to any specific location while cyclic analysis is a 'storm load' sequence of particular amplitude applied to the structure. Shakedown effects studied using non-linear FE analysis at SINTEF (Hellan et al, 1991) in provision of low cycle-high stress fatigue. Published in 1993, relies studies of North Sea Jackets, recommend that an extreme event static analysis generally suffices to demonstrate structure's resistance to the cyclic loading of a full storm. The study continues from SINTEF to Shell Research at the Offshore Mechanics and Artic Engineering Conference in 1993 (Stewart er al, Stewart and Tromns, Eberg et al and Hellan et al).

Under loading, structure convert into elasto-plastic range yielding occurs reducing the stiffness and introducing permanent plastic deformations. Under cyclic load, the yields repeats and result in three (3) different forms of response:

• Low cycle fatigue

- Incremental collapse
- Shakedown

#### 2.4 JACKET BRACING FRAMEWORK

SOH C.K (1990), Complexity of an offshore structure rely on the fabrication process where the location of yard to the site (oil block). A preliminary and detail design need to cater the critical activities during bringing the structure from yard to the site. Combination of load-out, lifting and transportation process generate a load that putting the structure in extreme condition where the design will be tested upon the critical joint or member section. For load-out and lifting procedure will be monitored carefully at the barge but during transportation on a barge, there are more uncontrolled variable of sea behaviour. In order to counter with the variable an effective preliminary and final design are needed to gauge the structure performance with respect to storm condition. The procedure in design breakdown into point below:

- Selection of appropriate elevation and member size
- Number of leg and the inclined angle
- Horizontal framing (nos)
- Bracing framing system
- Location of support structure and appurtenances

Main differences in any jacket structure are the features listed above. These features were meant for optimum design of an offshore structure according to its allocated oil block. Rough environment block needed a stronger substructure to hold the whole structure against any extreme condition. An optimum design defined as the whole substructure system in term of size, number of legged and bracing system.

In designing an appropriate, optimum or cost effective substructure system, a series of study had been done on the environmental condition of the specified area. Using the storm condition as the design benchmark in measuring the response of the structure against an extreme may occurred in the area. There are several factors affecting the substructure design, but the main highlighted component is the bracing framework of the structure. Bracing provide a load paths for all the loads to be shared by other member in avoiding a local member failure and result in collapse if the member reach ultimate yield. Bracing framework fabrication generates significant features in term of:

- Cost and time
- Strength

For cost, complexity and more rigid framework requires more steel and thus result in more welding needed forming the bond within the framework. Preparing joint can for member intersections requires more cost where the wall thickness of affected member has to increase to retain the cumulative generated. With more steel, the structure becomes heavier and heavy duty crane is needed to operating lifting procedure. Larger crane consume more energy than the conventional and this resulted in using more resources than usual. Making a complex framework is time-consuming and more manpower needed in attaching and welding the member to fabricate the framework system. In term of strength, a heavy, complex and rigid bracing framework provide a stronger substructure in achieving higher factor of safety. As for cost, time versus strength, an appropriate design for a jacket structure depending on site location and storm condition.

Nelson A (2003), five (5) common bracing configurations applied to the substructure as shown in Figure 2.3:

- X bracing
- Diamond bracing
- Inverted K bracing
- K bracing
- Single diagonal bracing

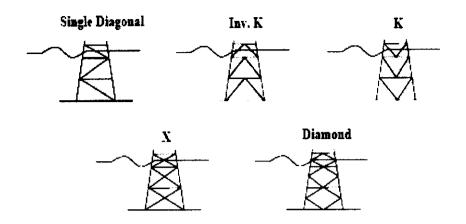


Figure 2.3: Bracing framework schemes

### **2.5 RELATED ENGINEERING SOFTWARES**

Generally, there are two (2) softwares commonly used in designing an offshore platform. The first one SACS (Structural Analysis Computer System) and the other one is USFOS. Usfos, software owned, developed and maintained by Marintek that enhanced in term of progressive collapse analysis of space frame structure. Other than that, functioning for predicting both resistance of structure subject to accidental loads and the residual strength of damaged structure after such loads. The applications listed as follows:

- Pushover analysis
- Accidental loads
- Ship collision
- Fire and explosion
- Reassessment
- Design

Some main features of the program:-

- Buckling and post buckling behaviour
- Local buckling
- Joint flexibility and ultimate strength
- Fracture
- Etc

The output results are performed in Graphical User Interface (GUI). The results are presented as colour fringes on images of the structure, deformed configurations, XY-

plots and in tables. The structure response and collapse process may be visualized step by step and highly stressed and critical members are easily identified.

**SACS<sup>®</sup>**, Structural Analysis Computer System is an integrated structural analysis software package used by the commercial industry world wide. Developed by Engineering Dynamics, Inc., this is the most comprehensive design and analysis package offered to both the offshore and the general structure design industries. Established by three engineers in 1973, EDI converted aerospace-oriented analytical techniques and computer programs into a single integrated *Structural Analysis Computer System* (SACS). In 1974, SACS was made available to private industry on a commercial basis. The system systematically gained worldwide acceptance in the offshore industry, eventually becoming the most widely used computer software for design and analysis of offshore structures. Today, EDI offers the most comprehensive design industries in the form of SACS. The SACS systems is used on every continent and are available as single user and network installations using Windows 9 xs, NT and XP.

SACS software provide user with the capability of large array modelling of a structure from simple two dimensional (2D) space frame analyses to complex three dimensional (3D) finite element analysis. It also features nonlinear static analysis when coupled with PSI module or dynamic response analysis when coupled with the Dynpac, Wave Responses and Dynamic Responses module.

The software provide broad of analysis from linear analysis to complex analysis. The software divides into 3 modules, namely the pre-processor module Pre, the solver module Solve and the post-processor, Post. The Post module separate into several categories as follows:

- Member Check Code
- Member Check location
- Output Report
- Redesign Parameter

The post module features can be directly specified with the documents follows:

- Member check code including: AISC, API RP2A-WSD, API RP2A-LRFD, Norwegian Petroleum Directorate and Danish Offshore
- API and DNV hydrostatic collapse analysis
- API and 2V Bulletins
- Euler buckling check for segmented member
- Automatic member redesign
- Allowable stiffness modifier
- Finite element code check and stiffener stress output

# 2.6 STATISTICAL T-TEST

William M.K. (2006), the t-test statistic analyze whether a two groups data are statistically different from each other. The main feature of the test is to compare the means of two groups selected. Following a normal distribution data or figure of the statistic, provides significant value between the selected data groups. Significance is a statistical term that describes the difference or relationship between the data. Significance value can vary depending on the data scale selected. With the scale a relationship between the data can be compared and tested.

Microsoft Excel (2003), data analysis features provide several of statistic analysis method called the Analysis ToolPak. With data and parameters, series of complex statistic computation computed using programmed spreadsheet assign for each statistic method. The tool provides appropriate statistical or engineering macro functions and then displays the results in an output table. There are several statistical analysis provided in MS Excel as follows:

- ANOVA
- Correlation
- Covariance
- Descriptive Statistics
- Exponential Smoothing
- F-Test Two-Sample for Variance
- Fourier Analysis
- Histogram

- Moving Averages
- Random Number Generation
- Rank and Percentile
- Regression
- Sampling
- t-Test
- z-Test

For the research, utilize the procedure using t-Test in managing the data output generates by the SACS software. The test provides in three (3) of analysis tools that features in different assumptions in evaluating the set of data. The analyses tools as follows:

- t-Test: Two-Sample Assuming Equal Variances
- t-Test: Two-Sample Assuming Unequal Variances
- t-Test: Paired Two Sample For Means

# 2.6.1 t-Test: Two-Sample Assuming Equal Variances

As referred to 'homoscedastic" test, this type of tool performs analysis of twosample with the assumption the two data sets came from distributions with the same variances. The final result of the statistic is to determine whether the samples are come from distributions with equal population means.

# 2.6.2 t-Test: Two-Sample Assuming Unequal Variances

The assumption of the two data sets came from distributions with unequal variances. It is referred to as a heteroscedastic t-test. Use this test when the there are distinct subjects in the two samples. Use the Paired test, described below, when there is a single set of subjects and the two samples represent measurements for each subject before and after a treatment.

The following formula is used to determine the statistic value t.

$$t' = \frac{\mathbf{x} - \mathbf{y} - \Delta \mathbf{0}}{\sqrt{\frac{S_1^2}{m} + \frac{S_2^2}{m}}}$$

The following formula is used to calculate the degrees of freedom, df. Due to different and precise engineering data, a value needed to calculate at least in four (4) decimal places in retaining the accurate calculation. The degree of freedom will provide the final answer in set of nearest integers for the purpose of simple and neat graphical presentation.

$$df = \frac{\left(\frac{S_{1}^{2}}{m} + \frac{S_{2}^{2}}{n}\right)^{2}}{\left(\frac{S_{1}^{2}}{m}\right)^{2}} + \frac{\left(\frac{S_{2}^{2}}{n}\right)^{2}}{n-1}$$

#### 2.6.3 t-Test: Paired Two Sample for Means

Another acronym for this type of tool is paired t-test. With a data regenerate from an equal population means, the test provide a test to a paired of groups data. In other word, the samples can be analyze for before and after effect where if it came from the same set of means. Accumulated measure of the spread of data about the mean, derived from the following formula.

$$S^{2} = \frac{n_{1}S_{1}^{2} + n_{1}S_{1}^{2}}{n_{1} + n_{2} - 2}$$

#### **2.6.4 Analyzing Statistics**

In determining the significant of the data, there are two sets of result interpreted as follows:

Statistic is higher than the critical value	Statistic is lower than the critical value		
Significant value of compared data	The selected data is insignificant		
Reject null hypothesis	Accept null hypothesis		

Table 2: t-Test characteristic

# CHAPTER 3 METHODOLOGY/PROJECT WORK

#### 3.0 METHODOLOGY/PROJECT WORK

The methodology of the research describe in two (2) phase:

- Full Plastic Collapse Analysis of Different Legged Platform
- Bracing Configuration Study

Investigations of the steel-jacket as the shallow water platform are to be done. A thorough search will be made through the internet, from the libraries and receive from RNZ (M) Integrated SDN BHD to collect all available information on the regarding the steel-jacket platform in offshore context. Collections of technical details regarding various platform SACS input file to compare their performances in the form of ultimate check when they are subjected to incremental loads. Simple linear static analysis and collapse analysis will be carried out for the selected model. The results of the analysis will be objectively compared with the actual performance data. Refer Appendix A for Gantt chart of the study.

#### 3.0.1 Platform Overview

#### 3.0.1.1 Platform A

Platform A is a four (4) pile-through-leg drilling platform installed in 1979 and located at Bekok field. It is supported by four piles. The piles are 54" Ø(137.16 cm). The platform supports twelve (12) numbers of 24" Ø (60.96 cm) conductors, two (2) numbers of riser pipes, three (3) numbers of pipe caisson, one (1) number of boatlanding on the Platform South face and two (2) boatlandings on the Platform West face. The topside comprises of the Upper Deck (EL +19202), Lower Deck (EL+12192). The water depth is 70.71 m. Details of the structure and its configuration are summarized as below:

Structure Function	:	Drilling Platform
Installation Date	:	1979
TAD Rig	:	Jack-Up
Water Depth (MSL)	:	70.71 m (209.56 ft)
No. of Piles	:	4
Pile penetration below mudline	:	79.25 m
Number of Conductor	:	12 nos (66.0 cm Ø)
Number of Anode	:	136
Number of Boatlanding	:	3
Number of Caissons	:	3
Number of Riser	:	2
Number of Riser Guard	:	1

Table 3: Platform A description

A 3-dimensional view of the platform A platform is shown in Appendix B.1

# 3.0.1.2 Platform B

Platform B is a three (3) pile-through-leg platform installed in 1977 and located at Betty field. It is supported by three piles. The piles are 30"  $\emptyset$  (76.20 cm). The platform supports four (4) numbers of 10.75"  $\emptyset$  (27.31 cm) risers, one (1) number of boatlanding. Details are summarized as below:

Structure Function	:	Storage Platform
Installation Date	:	1977
Water Depth (MSL)	:	70.93 m (236 ft)
No. of Piles	:	3
Pile penetration below mudline	:	68.00 m
Number of Anode	:	136
Number of Boatlanding	:	1
Number of Riser		4

# Table 4: Platform B descriptions

A 3-dimensional view of the platform B is shown in Appendix B.1

### 3.0.1.3 Platform C

Platform C is an eight (8) pile-through-leg drilling platform installed in 1979 and located at Bekok field. It is supported by 8 piles. The piles are 54" Ø(137.16 cm). The platform supports thirty two (32) numbers of 24" Ø (60.96 cm) conductors, ten (10) numbers of riser pipes, one (1) pipe caisson, and one (1) number of boatlanding. The topside comprises of the Upper Deck (EL +21184), Lower Deck (EL+14021). The water depth is 67.21 m. Details of the structure and its configuration are summarized as below:

Structure Function	:	Drilling Platform
Installation Date	:	1979
TAD Rig	:	Jack-Up
Water Depth (MSL)	:	67.21 m (209.56 ft)
No. of Piles	:	8
Pile penetration below mudline	:	109.73 m
Number of Conductor	:	32 nos (66.0 cm Ø)
Number of Boatlanding	:	1
Number of Caisson	:	1
Number of Risers	1:	10
Number of Riser Guards	:	2

Table 5: Platform C descriptions

A 3-dimensional view of the platform C is shown in Appendix B.1

#### **3.1 METHODOLOGY FLOW CHART**

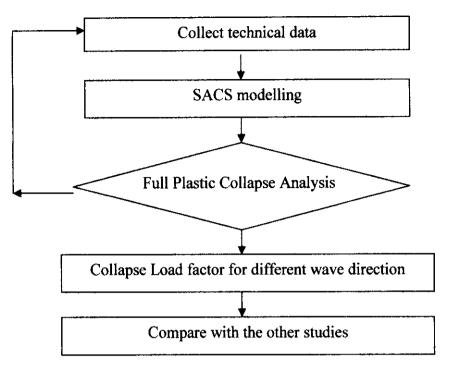


Figure 3.1 Flow chart of full plastic collapse analysis

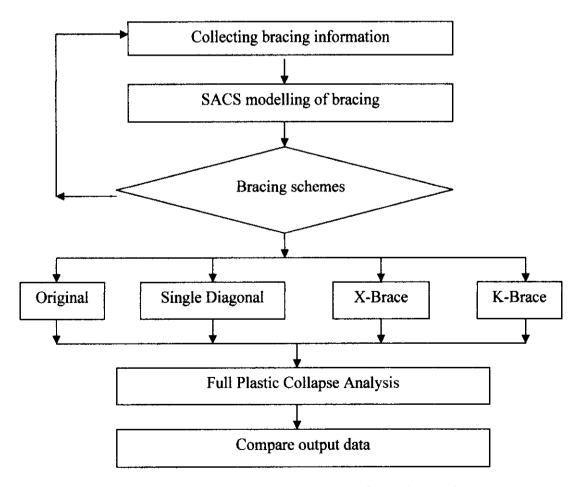


Figure 3.2: Flow chart of bracing configuration study

## 3.2 FULL PLASTIC COLLAPE ANALYSIS FOR DIFFERENT LEGGED PLATFORM

SACS modelling commence on platform A, B and C by adjusting the original model reference the site visit finding and latest drawing. The dead and live load of the SACS retained as per design basis. Minor adjustment made to the model in term of latest metocean data for the area. Latest data of maximum wave height (Hmax), associate period (Tass), wind speed, current speed and tidal height, HAT and LAT introduce to the model. The environmental loading impact to the platform cater for eight (8) which define in figure below. Only storm condition applied to the platform according to the metocan data as for maximum load acting to the structure. Using stokes's 5<sup>th</sup> theory define in API RP2A-WSD page 14 for wave/current loading computation. For the purpose of analysis, eight (8) models created for platform A and platform C while twelve (12) models were prepared for platform B since the leg arrangement is tripod and 12 direction need to be covered. Refer Appendix B.3 for reference code regarding wave.

The SACS Collapse module is a non-linear finite element analysis system for structures. Solve for the geometric and non-linear material by associating with the ultimate load capacity by using large, deflection, iterative, direct stiffness solution technique. The method was, the member divide in to several sub-segments along the length and sub-areas to define the cross section. The method allow for gradual plasticification along the member length. Tubular connection flexibility, capacity and failure revise as empirically.

The linear analysis model modified to be suitable for collapse analysis. Design the model to cater for only storm condition wave/current in order to get the strength of the structure in maximum loading criteria, analyze the model for SACS COLLAPSE analysis. The model revises from the linear static analysis part, where the same models will be using for collapse analysis. The directions of wave/current for the models define in Appendix B.4. Design collapse input file according to design basis for load sequence and load increment. Retain the other properties in the collapse input file as per default design. See Appendix B.5 for the collapse input file for the analysis.

The SACS model modified to localize wave, current attack angle for respective direction. For a tripod leg jacket model, 12 models generate to cater for all 12 attack angle as defined in the metocean data. For the remaining four and eight legs platform designed to cater only eight directions as defined in respective metocean data. The main issued to analyze for all direction to evaluate which direction contribute the highest load to collapse. A series of incremental load defined in collapse input file will generate collapse load by utilizing the module of FULL PLASTIC COLLAPSE ANALYSIS in the SACS software. Upon completing the analysis, interprets the output data for:

- Base shear and overturning moment
- Basic load case summary
- Load combination summary
- First member failure load
- Collapse load

Develop factor for first member fail and reserve strength ratio based on base shear and collapse load according the respective output. Using the collapse view module to view out the platform collapsed with its properties. Extract the data mentioned above to determine the structure ultimate strength with respect to its attack direction.

### **3.3 BRACING CONFIGURATION STUDY**

Improvising the original project by commend on remodelling the jacket bracing arrangement. As existence in design basis that, there was another type of bracing arrangement as follows:

- X bracing
- Y bracing
- Single diagonal bracing
- K bracing
- Diamond bracing

These types of bracing provide different share of all load transferred. As for the issues, the original model of platform A and will be remodelled to cater for all type

of bracing. Utilizing linear analysis, this new bracing design will undergo stability check in term of UC value of the respective bracing. Allowable value of UC<1.0 indicate that the new designs are acceptable for the collapse analysis.

Modelling the sacs input file (model) of platform A in term of structural bracing framework. Using the identical bracing properties of:

- Size
- Shape
- Wall thickness

New set of bracing framework design for the platforms for all faces designated by Row A, Row B, Row 1 and Row 2. All 4 faces defined as per drawing represent the no of leg of the platform. In modelling, retaining the same member properties for the new bracing member in order to analyze the strength of the structure using the same bracing size but differ in framework schemes. Adding new joint for allocated at the critical location especially modelling for X-bracing and K-bracing where the member intersection at the middle. Refer Appendix B.6 for the view of bracing configuration of platform A.

Applying the non-linear analysis method, a series of incremental load defined in collapse input file will generate collapse load by yielding the steel characteristic to plastic yield. The non-linear analysis computed the load share by the other bracing due to the load paths created by the framework. As for SACS software using module of Full Plastic Collapse Analysis to compute as non-linear until the structure has no alternative load paths available, member reach plasticity and structure collapsed. Upon completing the analysis, interprets the output data for:

- Base shear and overturning moment
- Collapse Solution Summary
- Collapse Load
- Maximum deflection
- RSR

With all bracing model been analyze and compare, a study of the behaviour of the jacket structure with response to the collapse load by the affect of bracing framework

strength. All the output from all bracing model tabulate and compare the different in maximum load needed to make the respective member fails.

### 3.4 HAZARD ANALYSIS

Modelling and analysis using computer software invites glare effect to the eyes and also induced MSI (Musculoskeletal injuries) that include muscles, bones, tendons, blood vessels, nerves and other soft tissues. Spend quite time at a workstation without proper ergonomics apparatus will induced severe damaged to the body. As precaution set up the workstation according to HSE recommendation, OSHA (1994) as follows:

	Lights		Furniture					
•	Video display devices: 300-400 lux	•	Adjustable height					
	(30-40 foot candles)	•	Able to support body					
•	Retain image quality	•	Table tops: 27 ins high for typing and					
•	Shield from direct or intense/bright		29 ins from other tasks					
	light: use drapes, dark film, louvers.	• Leg room:27 ins x 27 ins						
•	Minimize glare; use screen filters	Height adjustable chairs:						
•	Desktop which have matte finish or		<ul> <li>15-20 ins above the floor</li> </ul>					
	dark in color, are less visually		• Seat pan 16 ins wide minimum,					
	fatiguing than those of glossy,		18-19 ins preferred					
	reflective finish		<ul> <li>Seat padding should not</li> </ul>					
			compress more than one ins when					
			seated					

Table 6: Hazard analysis

### **CHAPTER 4**

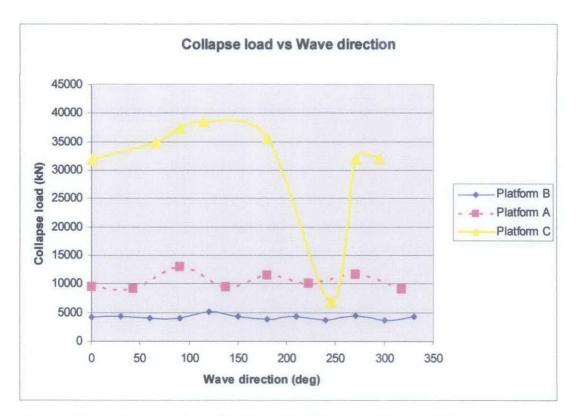
### **RESULTS & DISCUSSIONS**

### 4.0 PLATFORM A, B AND C ULTIMATE STRENGTH

Upon completion on all three platforms mentioned above, the result of respective models been interpret and resulted in table and diagrams illustrated below.

Platform	Wave Direction (deg)	First Member Fail Load (kN)	Collap <del>se</del> Load (kN)	RSR	Factor For Collapse
	0.00	3298.68	4136.79	1.25	2.39
	30.00	4017.31	4331.39	1.08	2.60
	60.00	3207.17	4020.10	1.25	2.27
	90.00	2678.52	3956.14	1.48	2.31
	120.00	3296.41	5142.96	1.56	2.91
8	150.00	3522.15	4260.86	1.21	2.56
0	180.00	3299.57	3880.84	1.18	2.24
	210.00	2775.55	4262.99	1.54	2.56
	240.00	3208.95	3618.19	1.13	2.04
	270.00	4134.51	4491.81	1.09	2.62
	300.00	3300.51	3736.09	1.13	2.11
	330.00	2694.31	4287.16	1.59	2.57
· · ·	0.00	9230.77	9488.49	1.03	4.05
	42.11	7844.51	9174.21	1.17	3.35
	90.00	12226.07	12974.80	1.06	2.18
•	137.89	8140.04	9489.52	1.17	2.76
A	180.00	10582.18	11465.45	1.08	3.17
	222.11	8396.79	10078.17	1.20	2.54
	270.00	10746.32	11690.44	1.09	5.72
	317.89	7944.94	9018.19	1.14	4.10
	0.00	30781.52	31730.06	1.03	9.69
	65.00	33858.24	34839.44	1.03	5.10
	90.00	32464.96	37317.46	1.15	3.00
С	115.00	34632.22	38437.97	1.11	4.74
U	180.00	32551.37	35644.39	1.10	9.52
	245.00	6809.40	6809.40	1.00	1.56
	270.00	30910.52	31968.74	1.03	6.68
	295.00	26443.94	32003.96	1.21	7.16

Table 7: Summarization for different legged structure



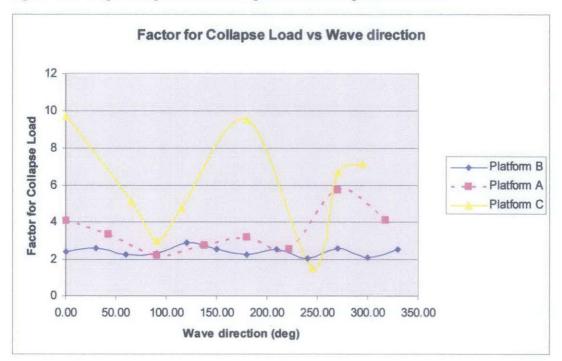


Figure 4.1: Graph comparison of collapse load for respective models



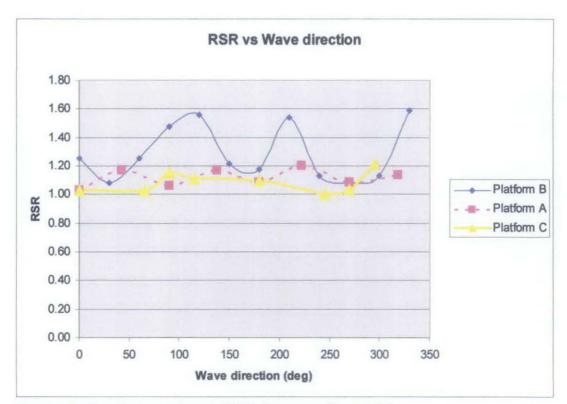


Figure 4.3: Graph comparison of RSR for respective models

By referring to all diagrams above, clearly illustrate that the number of jacket leg affected the collapse loads. Supporting rule for the statement is the existence of alternative load paths created through jacket leg member and jacket bracing provide more stiffness to the structure. Based on the first clearly indicate a comparison between the respective load needed for structure collapse with respect to number of legs and location. Furthermore, the trend of the graph shows that platform C with eight legged structure needed more extra load compared to platform A with 4 legs and platform C, tripod type. More leg, meaning more structure welded to make up a complex, rigid and strong substructure system. The theory of having complex and bigger structure proves by the behaviour of platform C in retaining the structure against incremental load until collapse. RSR diagram indicate that platform C with tripod leg resulted in more strength reserve compared to more redundant structure. The design was made for three leg jacket structure to have more reserve strength before the structure reach critical point of ultimate load.

Observing the table and first figure regarding the collapse load to the wave direction, a tabulate data of respective information regarding the critical attack angle of wave direction to the structure:

Platform	Wave attack angle (deg)								
I IAUUI III	Minimum	Maximum							
В	90.00	270.00							
A	42.11	90.00							
С	245.00	115.00							

Table 8: Collapse load with respect to wave direction

The maximum column of the tabulate data interprets that the structure can retain to highest load before it collapse. Vice versa to the sentence meant that, the base shear force generated by the wave at the angle is much less than the other direction. The minimum column of the tabulate data illustrates the critical angle to the structure where minimum load required for the structure the fails. In other word, fewer loads needed to make the structure tremble, fail and collapse. The structure at the angle face having a weak spot where the wave generated forces can weaken the structure rigidity on the affected faces and disperse the affect to other area an thus resulting global collapse.

A study regarding the relationship of the individual wave generated forces to the collapse load. The variable where highest individual base shear will contribute to the collapse load at the angle. Refer to Appendix C.1 for comparison the base shear generated by respective attack angle for each platforms. For platform C the highest base shear generated at the angle of 300 deg to the platform. Differently interpret by the non-linear analysis where the minimum load for structure to collapse at the angle of 90 deg while at 270 deg, more load needed. More on that, for platform A, the highest reported base shear at 90 deg where the result was parallel to the final value where higher load is needed for the structure to fail. Lastly, platform C metocean data report that angle of 90 deg is the critical wave forces but different measured by the analysis where to structure is not critical when attack by the angle.

For different legged platforms, different behaviour, response and rigidity of the structure acted upon the incremental load. Complexity and rigidity is the main criteria for a platform to have a definite ultimate strength.

#### 4.1 BRACING CONFIGURATION STUDY

Upon completion on all three bracings schemes for platform A, the result of respective models been interpret and resulted in table and diagrams illustrated below. The entire diagrams purposely to compare and differentiate the behaviour of platform A structure with the different type of bracing framework. Refer appendices of overall analysis.

Configuration	Wave Direction (deg)	First Member Fail Load (kN)	Collapse Load (kN)	RSR	Factor for Collapse					
	0.00	9230.77	9488.49	1.03	4.05					
	42.11	7844.51	9174.21	1.17	3.35					
ēs	90.00	12226.07	12974.80	1.06	2.18					
ğ	137.89	8140.04	9489.52	1.17	2.76					
<b>D</b>	180.00	10582.18	11465.45	1.08	3.17					
Design Basis	222.11	8396.79	10078.17	1.20	2.54					
an	270.00	10746.32	11690.44	1.09	5.72					
	317.89	7944.94	9018.19	1.14	4.10					
	0.00	10410.32	10630.12	1.02	4.27					
	42.11	8311.26	9144.72	1.10	3.15					
×	90.00	11604.26	15280.47	1.32	2.44					
X-Bracing	137.89	8449.42	10042.01	1.19	2.75					
<b>I</b> Ci	180.00	11397.21	11584.19	1.02	3.02					
Bu	222.11	8049.95	10862.32	1.35	2.58					
	270.00	10816.69	12445.34	1.15	5.76					
	317.89	7960.63	9885.51	1.24	4.24					
	0.00	9215.76	9493.51	1.03	4.06					
Si	42.11	7826.34	9029.29	1.15	3.31					
hBu	90.00	10424.99	11221.38	1.08	1.89					
Single Diagonal	137.89	8296.00	9125.70	1.10	2.66					
liae	180.00	10557.71	11097.71	1.05	3.08					
Jon	222.11	8568.37	10600.58	1.24	2.68					
a	270.00	9683.77	9751.93	1.01	4 79					
	317.89	7929.77	9527.65	1.20	4.34					

Table 9: Bracing summarize data

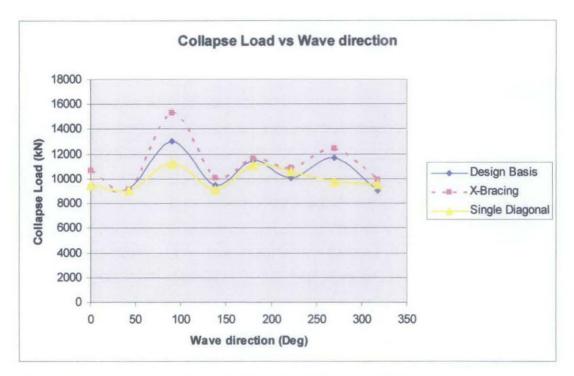


Figure 4.4: Graph comparison of collapse load for respective bracing

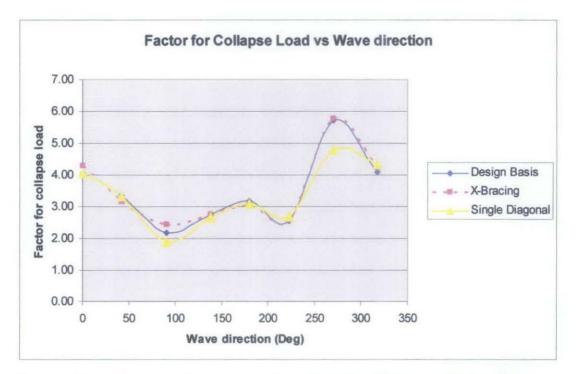


Figure 4.5: Graph comparison of factor for collapse load for respective bracing

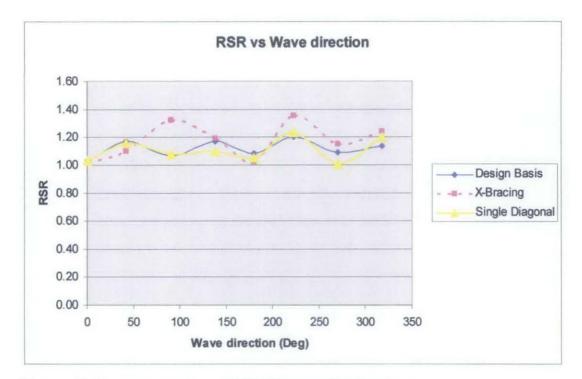


Figure 4.6: Graph comparison of RSR for respective bracing

By observing and studying all diagrams above, clearly illustrate that the X-bracing provide more stiffness the platform A compared to other type. X-bracing scheme provide more steel framework that provides more load paths and redundancy to the substructure. At the angle of 90 degree, clearly shown that the collapse load for all types resulted the similar peak but different in several factor. Individually, platform A response to have the ultimate collapse load at 90 degree with the same as highest base shear reported at 90 degree in the previous analysis. Lesser ultimate load showed by single diagonal bracing where less framework line created by a single cross member indicate that low in the available load paths for load to be shared by the other bracing. RSR diagram indicate that the X-bracing resulted in more strength reserve compared to the other bracing schemes. The bracing provide more reserve strength before the structure reach critical point of ultimate load. By comparing all three (3) figures above, the design basis performance reported between X-brace and Single diagonal brace.

Observing the table and figures regarding the collapse load in term of bracing configurations to the wave direction, a summarize data regarding performance of respective bracing schemes of platform A:

Draoing	Collapse	Load (kN)	RSR			
Bracing	Minimum	Maximum	Minimum	Maximum		
Configuration	(direction)	(direction)	(direction)	(direction)		
Design Basis	9018.19	12974.80	1.03	1.20		
	(317.89°)	(90.00°)	(0.00°)	(222.11°)		
X-bracing	9144.72	15280.47	1.02	1.35		
	(42.11°)	(90.00°)	(180.00°)	(222.11°)		
Single Diagonal	9029.29	11221.38	1.01	1.20		
Bracing	(42.11°)	(90.00°)	(270.00°)	(317.89°)		

Table 10: Bracing configuration summarization results

With the summarize data, the platform more stiffness for incoming wave at 90 degree. The situation is where higher load computed to make the structure collapse from the 90 direction than the other directions. The situation supported by all bracing configurations provide with the highest load at wave direction of 90 degree which are parallel to individual base shear generated.

Interpreting the tabulate data above, a polar in term of strength of each bracing clearly shown by the single diagonal is the weakest, X-bracing provide highest rigidity while the original or design basis in between of the two braces. These circumstances indicate that the design for platform A is adequate and effective to the environmental area of the site location. As for conclusion, the original design of platform A is cost effective and suitable with the surrounding area.

Due to leg arrangement of platform A at ROW B, the other bracing of Kbracing, Inverted K-bracing and Diamond bracing are inappropriate. The problem is where Launch Cradle that used for sliding the jacket onto barge installed to the structure. With the launch cradle attach to the substructure, there was no horizontal member framing at designed elevation. The horizontal was offset by several dimensions to cater the launch cradle framing. With one face of the jacket structure not suitable for the remaining bracing, conclude for not affecting the original jacket structure by adding horizontal member for K-member can intersect. Refer Appendix B.6 for detail.

#### **4.2 STATICTISAL ANALYSIS**

Statistical analysis commence on the data of the analyses to evaluate its effectiveness and significances. Variable of data need a statistic analysis to determine the data significance and determine the outliers data for better accurate results. Using t-test to evaluate t-stat and t critical 2-tail. Refer Appendix C.5 for the results.

#### 4.2.1 Platform A, B and C collapse comparison

Utilizing t-test in the Microsoft Excel, all variable in the graph tested for validity of data distribution. Based on Appendix C.5 all resulted in significant data difference due to different type of leg arrangement resulted more dispersed data compared to the same model data. With highest t-Stat reported at -6.72 and the computed t Critical two-tail, 2.36, the t-test indicate significant data disperse by the three platforms. As shown by tabulate result, the data between platform A, B and C are widely dispersed from each other. As conclusion, clear different performance between all platforms.

#### 4.2.2 Bracing Configuration

Using the data of RSR column, refer to Appendix C.5 for the output computed by t test of Microsoft Excel. The t Critical two-tail was reported as 2.36 while the highest t-Stat computed at 1.87 by the relationship between x-bracing and single diagonal bracing. With t-Stat in range of coupling  $\pm$  2.36 of the normal distribution, the overall data of the analysis is insignificant. The data is not dispersed and within the normal distribution. In order way, there were no outliers in the data for analysis part B.

The situation is where the usage of the same platform with characteristic of the same environment data but only different in bracing framework resulted in less spread data. There was a limit created by the overall platforms with respect to the RSR value. Regardless the difference in bracing framework, the overall data computed is within the range for concluded as insignificant.

#### CHAPTER 5

#### **CONCLUSION AND RECOMMENDATION**

#### **5.0 CONCLUSION**

Platforms beyond design life need an assessment for the ultimate capacity check for further service extracting crude oil under the seabed. An effective method carried out to check for the platform reliability in next few years of the extended services. For the first part of analysis, clearly can conclude that larger no of legs affect to overall strength where platform C an eight-legged platforms result highest ultimate load compared to the other 4-legs and tripod. For the overall result, the Reserve Strength Ratio (RSR) ranging from 1.0 to 1.6 while the collapse load factor range from 2.0 till 6.0. The highest ultimate load reported at platform C with 38437.97 kN at wave angle of 115°. The highest RSR for the first part of the project computed at 1.59 of platform B at 330° direction..

The first part of the analysis achieve to first two objectives stated before. By completing the analysis of platforms A, B and C which entitled or represents the major and common platforms installed in the country. The location of each platform at different oil blocks in the region mainly to check the environment factor effect on the structures. With platform A and C located offshore Kerteh and platform B offshore Bintulu are sufficient to cater the differences between the two environment conditions. With all the technical data and SACS model acquired, commencing on developing the SACS collapse input file as command or coding in defining the incremental load, exclude the topsides member as defined as elastic through out the collapse analysis and other properties in fulfilling the second objective by generating result though the SACS software.

Difference properties of each selected platforms of three, four and eight legs provide the performance of each structure with respects to environment and maximum load. The study of the effect resulted by 8-legged structure compare to the others provide significance difference in term on how the platform response to such extreme loading. With the local storm condition applied and incremental load, the selected structures were tested until collapse. Furthermore, by interpreting and evaluating the data computed with the Full Plastic Collapse Analysis module, the differences in term how 8-legged, 4-legged and 3-legged jacket structure response to such incremental load define in the module. The results been evaluated to measure the ultimate strength of different legged platform in order to achieve the stated objective.

The second part of the project consist the study of bracing configuration of a jacket substructure to the collapse load. The introduction of the second part which consisting of the effect of bracing framework to the ultimate load in order to improvise the project validity, quality and data. As mentioned in theory of literature review and practical in the methodology section, the study scope includes developing a set of bracing framework system of a model in organizing a systematic approach on a single platform but differ in the bracing framework of the substructure.

In order of fulfilling the third and final objective, platform A been selected and undergone series of modification in term of bracing schemes. As conclusion the highest collapse load achieved with X-bracing model at 15820.47 kN of load before the structure collapse. Maximum RSR computed at 1.32, wave attack angle of 90 degree. For overall view of platform A bracing schemes, the RSR ranging from 1.01 to 1.35 while the collapse load factor from 2.18 to 5.76. Excluding the bracing type K-bracing, Inverted K-bracing and Diamond bracing due to no horizontal member at specified elevation due to the installed launch cradle for transportation and lifting procedure

#### **5.1 RECOMMENDATION**

As for recommendations for the future studies, collapse analysis for API RP2A-LRFD needed to be compare with the WSD design in term of the same method of analysis. With different code, the results expected to be similar but slightly different in behaviour of the load paths and method of computation. API RP2A-LRFD approach to solution by governing the load factor in computing and it's also came with difference constant such as drag, inertia and others. Comparison with the LRFD and WSD code can make up factors which differentiate one code to another. Furthermore, the trend of highly utilize the LRFD code in analyzing the collapse load fully demanding the comparison in which code provide better accuracy and also cost effective for maintenance factor.

The other suggestions for improving the project by analyze the models for Linear Static Analysis that also known as the Simplified Ultimate Strength (SUS) Analysis. This version of analysis comprising the same procedure by increment the load combination of storm wave until one the component fail or meets capacity:

- joints
- members
- pile steel strength
- pile soil bearing capacity

Then the analysis furthered by removing these fails components to allow alternative load paths existed in the framework. Then the analysis completed when the software cannot find solution meaning no load paths available or exceed deformation. The corresponding data is the collapse load where the analysis terminate and the base shear generated by the directional waves. As the analysis being studied and applied in the industry, the demand to check the analysis performance compared to the nonlinear analysis of full plastic collapse. With these data, a comparison can develop a factor to specify the margin resulted by each methods.

For the second part, in order to have better data comparison all type of conventional bracing should be tested and evaluated. The exclusion of K-bracing, Inverted K-bracing and Diamond bracing in the analysis resulted in narrowed option where X-bracing resulted stronger strength compared to original design and single diagonal. In order to rectify with the issues, selecting similar platform which appropriate for the bracings mentioned above. Other than that, a comparison with another legged platform significantly provide data on how bracing configuration affecting the whole structure strength. Usable platform B and C are recommended for bracing configuration study. Taking the overall data, a study on how leg affecting the bracing schemes performance.

For the last recommendation in improving the project, comparing the results gain from SACS software to another related offshore-structures software as example USFOS. Comparing the data generate by both software can enhanced the specific utilization and speciality. Checking on how both software responses to such analysis conducted by a user. USFOS speciality in progressive collapse analysis of space frame structure, but SACS also provide the module for full plastic collapse analysis. More on that, referring and concentrating on the software how much margin develop by using the model for the same type of analysis. Study on how SACS software differs from USFOS software in solving the progressive collapse analysis.

### CHAPTER 6

#### **ECONOMIC BENEFIT**

In order to measure the feasibility of the project, a study of project objectives and results for the purpose in industry interests and benefits. Content from introduction part, literature review, methodology, results and discussion will be utilized in order to meet with industry demand. A relationship between two bodies to forecast and predict the availability and requirements in order to meet with demand and also benefits. The related research methodology and results are the primary content in developing the interest of project to the industry demand.

Several business elements and other that relevant to the economic values recognised and verify throughout the project. Among the main output is mainly regarding the jacket platform design and fabrication. In order to achieved or meet an effective design, desirable cost and strength, a comparative investigation upon the case study commenced upon three (3) difference platform in order get a benchmark value or structure behaviour. In order to predict to behaviour of an offshore structure with respect to extreme condition is applicable for generate or introducing the factor of safety or reserve strength ratio. The purpose of benchmarking the model in the project is mainly to study and compare the difference of current and previous practices in term of design and fabrication in term of cost, time and effectiveness.

Relating the data output of several analyses, the data of an extreme condition of a specific location established. Accordance to metocean data and SACS output, values of base shear computed at respective direction. The software allows user to compare which direction generate large force to the structure. With these data, counter measure of reinforcing the face or integrity management upon the critical side of the platform.

As recommendation, further studies into the project potentially generate the reliability index of an aged structure. Due to large number of aged structure offshore Malaysia's field, the questionable issues of the current strength of the platform beyond the pre-determined life-span. Currently, more than 80% of offshore platform around Malaysia's block field aged 30-40 years which is beyond the original design life of 25 years. Upon Structural Integrity Management (SMI), a lot of analysis and improvising the platform to enhanced the platform capacity and usability due to extended service. The SMI mainly to check the platform current performance, the critical member and joint, installing new equipment and modification needed. As reliability index provide factor or ratio that enable user or operator to estimate the platform reliability even though already undergone grouting or other modification.

There are a lot of bracing schemes widely used in any jacket structure for substructure arrangements. Difference bracing provide different strength and differentiate the behaviour of a structure to another with respect to extreme condition. The strongest and costly bracing is the X-bracing that resulted in higher strength compared to other as per case study. A cost effective bracing scheme is needed which is suitable with the environmental condition of the area. Comparison study of all type of bracing configuration reveals the behaviour of the structure.

The other benefit of the project is more method, code reference and also software applicable for establishing relationship and also comparison. Other design may utilize different code reference to assist the structure. Difference code may affect the final result slightly difference to other by a factor. The issues of WSD and LRFD code in purpose of checking ultimate load to the platform. The other issue is the difference between outputs of simplified ultimate strength to the static pushover analysis in determines the collapse value. Develop comparison software of SACS and USFOS in determining the collapse load.

### **CHAPTER 6**

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## **CHAPTER 7**

## **APPENDICES**

APPENDIX A	PROJECT CHART FLOW
APPENDIX B	SACS MODELING
APPENDIX C	SACS OUTPUT DATA

GANTT CHART OF THE PROJECT

APPENDIX A

# **Final Year Project 1**

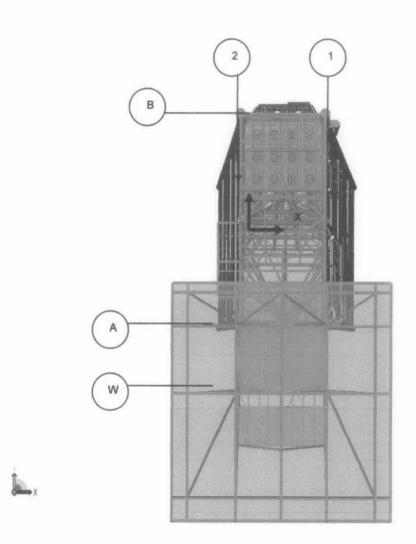
No	Detail/Week	1	2	3	4	5	6	7	8	9		10	11	12	13	14
1	Selection of Project Topic										]					
2	Preliminary Research Work			_												
	(Related Article/Journal)												l			
3	Article/Journal Summarization															
4	Literature Review drafting															
5	Submission of Progress Report															
6	SACS Collapse module training															
7	SACS modeling										ak					
8	SACS ouput analysis										Bre					
9	Submission of Interim Report										er					
	Final Draft										lest					
10	Weekly meeting with supervisor										Mid-Semester Break	-				
11	FYP Workshop										] -bi		_			
	1. FYP1 briefing										Σ					
	2. IEM Talk															
	3. Technical Writing										]					
	4. IRC Workshop															
	5. Laboratory Workshop										]					
	6. Referencing Workshop															
	7. Poster presentation															
	8. Session with HOD															
	9. HSE Workshop															

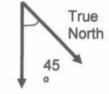
# **Final Year Project 2**

No	Detail/Week	1	2	3	4	5	6	7		9	10	11	12	13	14	15	16
1	Bev A modeling																
2	Bev A collapse analysis																
3	Tiong A modeling																
4	Tiong A collapse analysis																
5	Submission of Progress Report								ak								
6	Jacket Bracing configuration study								Break								
7	Bekok B bracing modification								er l								
8	Bracing data interpretation								lest								
9	Poster preparation								em								
10	Submission of Dissertation Report								Mid-Se								
11	Weekly meeting with supervisor								M	-							
12	Statistic Workshop	-															
13	Graduate School Workshop																
14	Poster Exhibition																
15	Implementation of IBS project			Γ													
16	Oral Presentation																

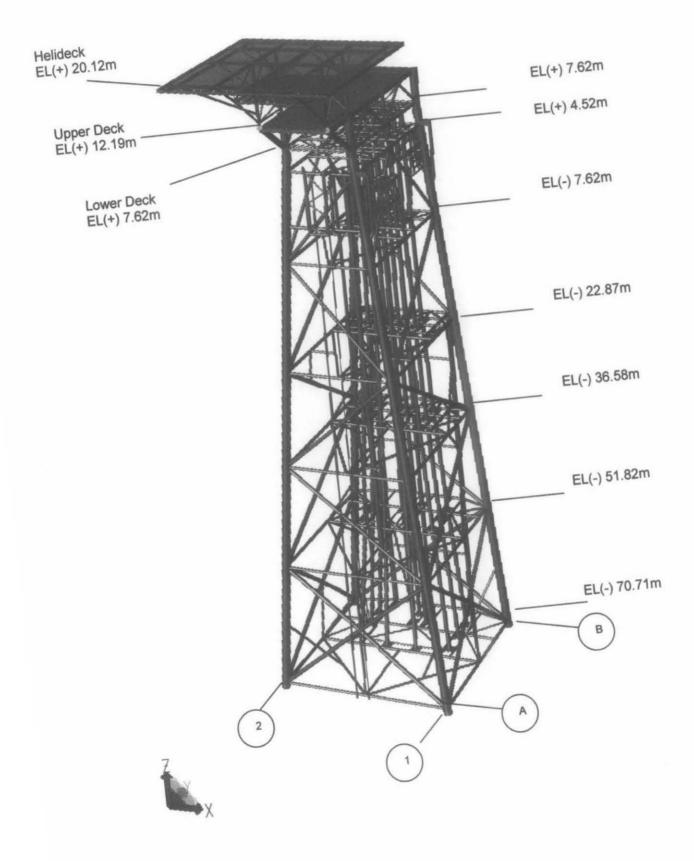
APPENDIX B.1	PLATFORM 3D VIEW
	A) PLATFORM A
	B) PLATFORM B
	C) PLATFORM C
APPENDIX B.2	METOCEAN DATA
	A) PMO
	B) SKO
APPENDIX B.3	API RP2A WAVE THEORY
APPENDIX B.4	WAVE/CURRENT ATTACK ANGLE
APPENDIX B.5	COLLAPSE INPUT FILE
APPENDIX B.6	BRACING SCHEMES
	A) DESIGN BASIS
	B) X-BRACING
	C) SINGLE DIAGONAL BRACING
APPENDIX B.7	PLATFORM A K-BRACING PROBLEM

# A) PLATFORM A 3D VIEW

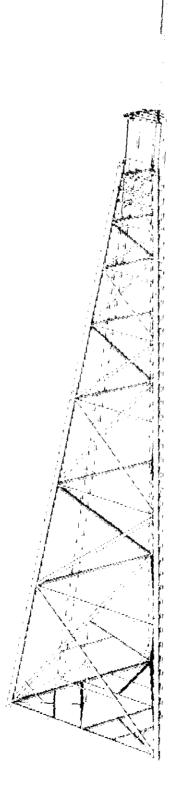




Platfo	r	n	1
North			

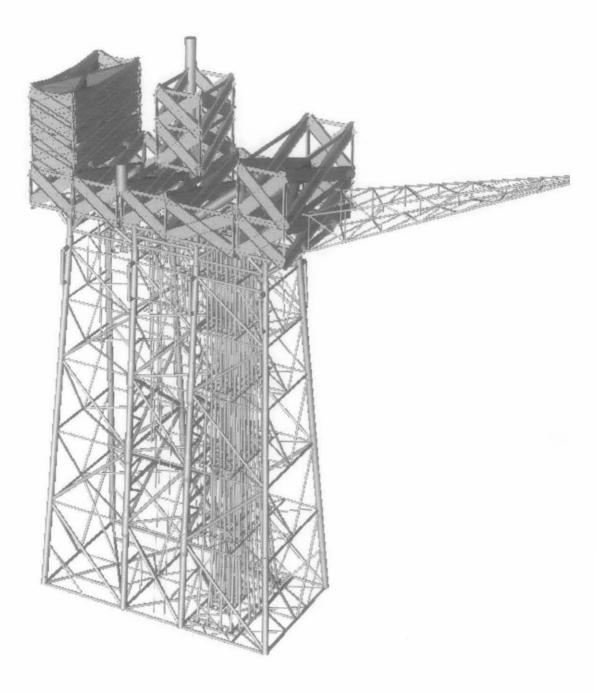


B) PLATFORM B 3D VIEW



C) PLATFORM C 3D VIEW

.



# A) PMO METOCEAN DATA

Wave	100-Year Directional Wave (deg)											
Parameter	0	42.11	90	137.89	180	222.11	270	317.89				
Maximum Height, Hmax (m)	6.3	6.3	11.4	7.6	7.6	7.6	5.0	6.3				
Associated Period, Tass (s)	7.3	7.3	9.3	8.4	8.4	8.4	6.6	7.3				

Table B.2.1: Wave data for Platform A

Table B.2.2: Wave data for Platform C

Wave	100-Year Directional Wave (deg)											
Parameter	0	65	90	115	1 <b>8</b> 0	245	270	295				
Maximum Height, Hmax (m)	5.8	7.3	10.1	8.2	5.8	5.8	5.8	5.8				
Associated Period, Tass (s)	8.0	8.5	10.0	9.0	8.0	8.0	8.0	8.0				

**B) SKO METOCEAN DATA** 

Wave	100-Year Directional Wave									
Parameter	N	NE	E	SE	S	SW	W	NW		
Maximum Height, Hmax (m)	10.0	9.0	5.1	5.1	5.1	6.9	9.0	10.0		
Associated Period, Tass (s)	9.7	9.4	8.3	8.3	8.3	8.6	9.4	9.7		

Table B.2.1: Wave data for Platform B

# API RP2A WAVE THEORY

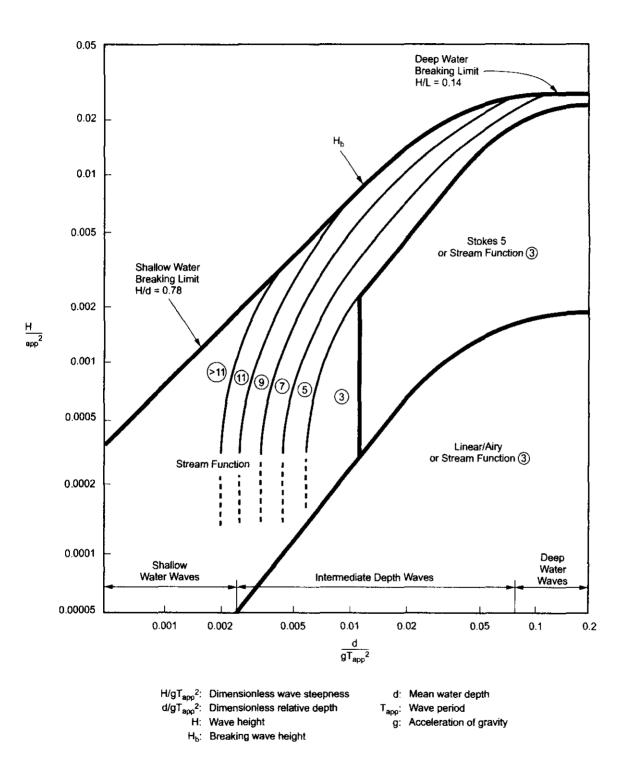
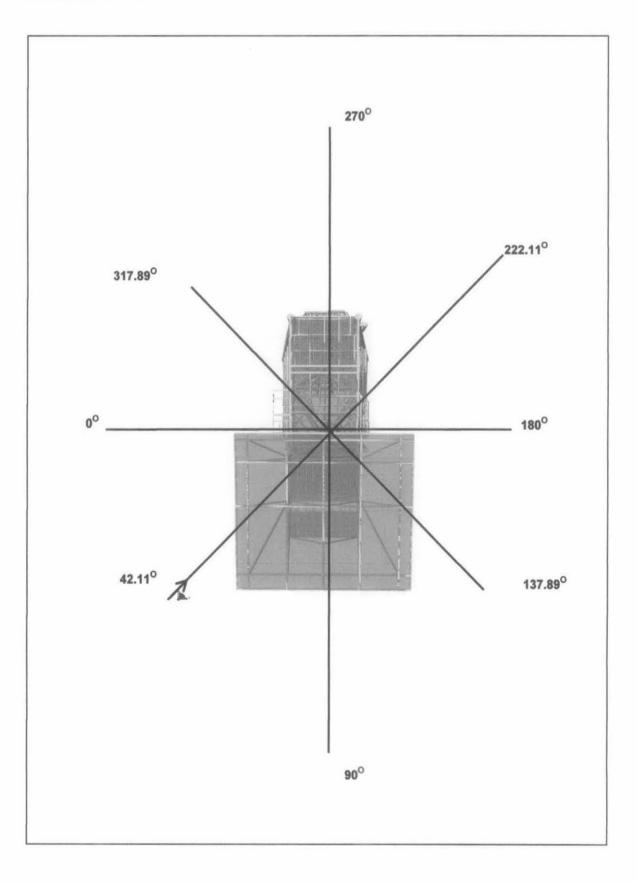


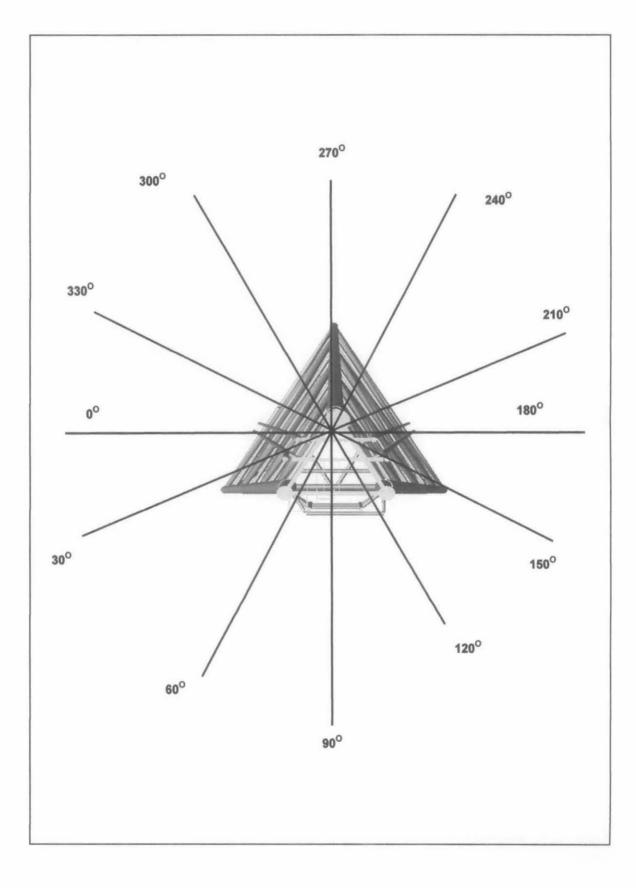
Figure 2.3.1-3—Regions of Applicability of Stream Function, Stokes V, and Linear Wave Theory (From Atkins, 1990; Modified by API Task Group on Wave Force Commentary)

WAVE/CURRENT ATTACK ANGLE

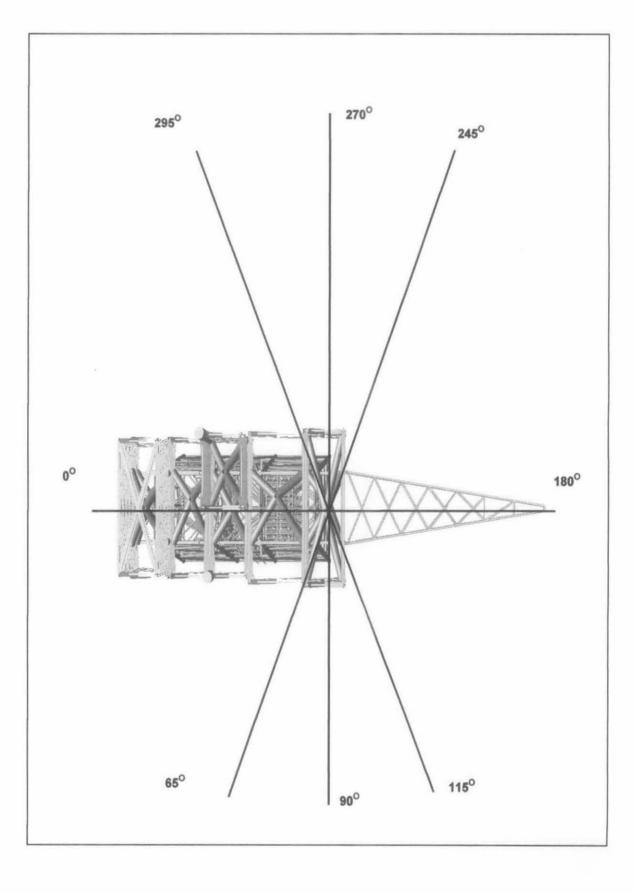
Title	Wave/current attack angle
Platform	Platform A
Appendix	B.4



Title	Wave/current attack angle
Platform	Platform B
Appendix	B.4



Title	Wave/current attack angle
Platform	Platform C
Appendix	B.4



COLLAPSE INPUT FILE

### **Platform** A

SFMG 0.010.001 0.011000.0.002 CLPOPT 40 8 40 CN LBJFPPJS CLPRPT PIRIMIMP JISMMSPW CLPS 1 0.5 1.0 100 100 LDSEO LS1 5. GRPELA BB0 BB1 BB2 BB3 BB4 BB5 BB6 BB7 BB8 BB9 BC1 BC2 C1 C2 C3 GRPELA H10 H11 H12 HB1 HB2 HB3 HB4 HC1 HC2 HC3 HH0 HH1 HH2 HH3 HH4 GRPELA HH5 HH6 HH7 HH8 HH9 HT0 HT1 HT2 HT3 HT4 HT5 HT6 HT7 HT8 HT9 GRPELA HV1 HW LB1 LB2 LB3 LB4 LB5 LB6 T1 T2 T3 T4 T5 T6 T9 TR1 TR2 TR3 WB1 WB2 WB3 WB4 WB5 WB6 WB7 WB8 WB9 WBB WC1 WC2 GRPELA GRPELA WC3 WC4 WC5 WC6 WT1 WT2 WT3 WT4 WT5 WT6 WT7 WT8 WT9 BC3 BC4 W.B AA1 AT2 AT2 AT3 AT4 AT5 AT6 AT7 AT8 AT9 ATT ATU CGF CN1 GRPELA GRPELA CN2 CN3 CN4 CN5 CN6 CN7 LB1 LB2 LB3 LB4 LB5 LB6 RR1 RR2 WBB PGRELA CDP LAP PL1 UDP UP1 END

### **Platform B**

CLPOPT 40 8 40 CN LBJFPPJS SFMG 0.010.001 0.011000.0.002 CLPRPT P1R1M1MP J1SMMSPW LDSEQ LS1 CLPS 1 0.5 1.0 100 100 5. END

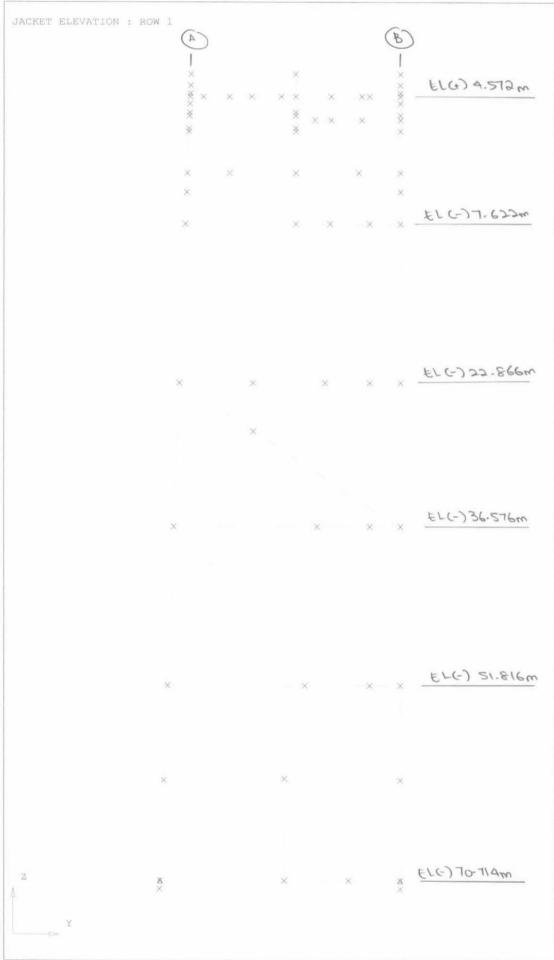
### **Platform C**

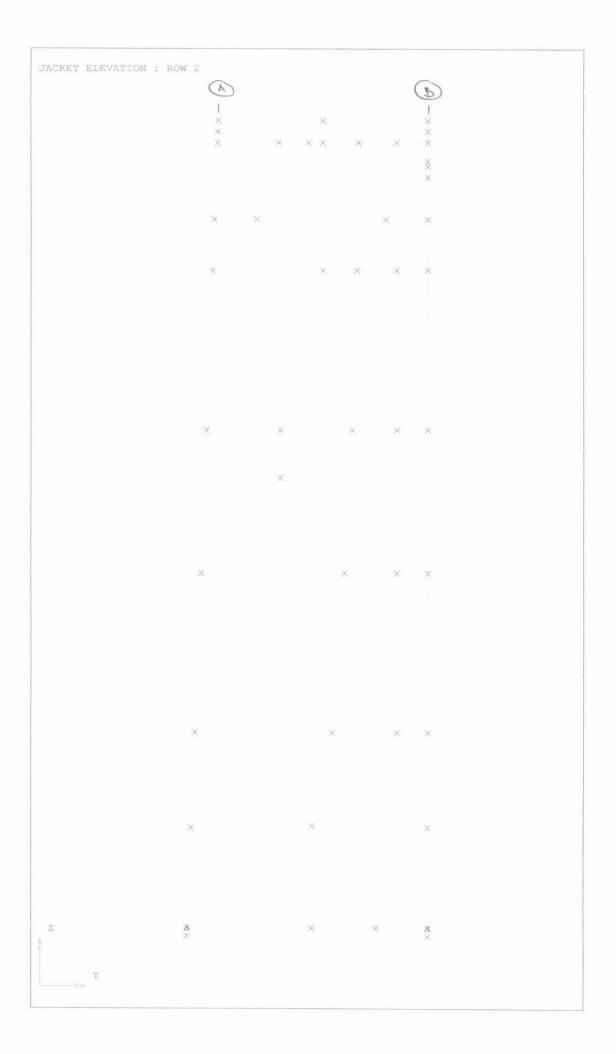
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 40
 8 40
 CN
 LBJFPPJS
 SFMG
 0.010.001
 0.011000.0.002

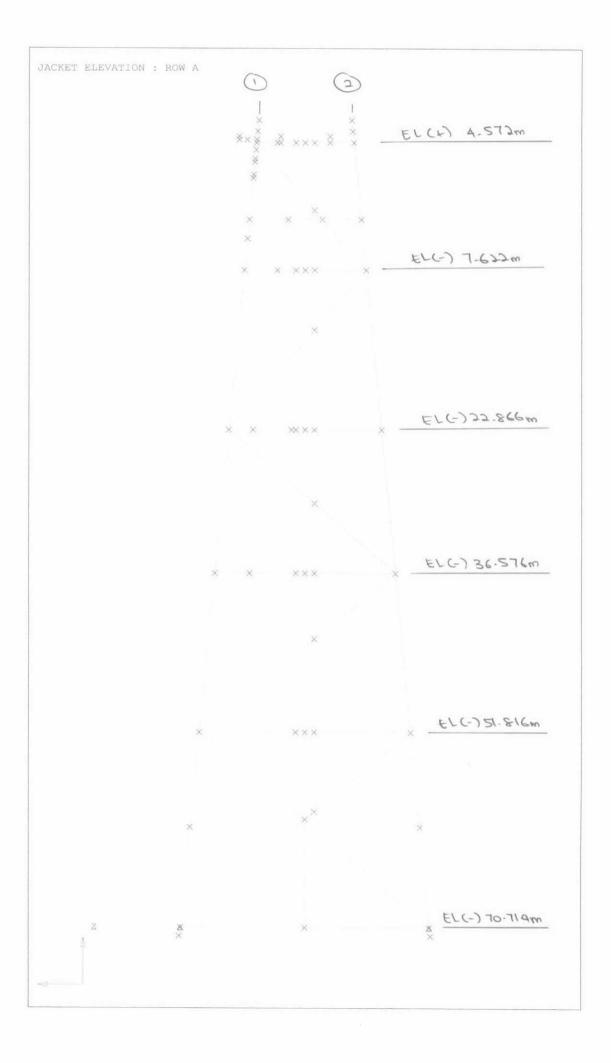
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 P1R1M1MP
 J1SMMSPW
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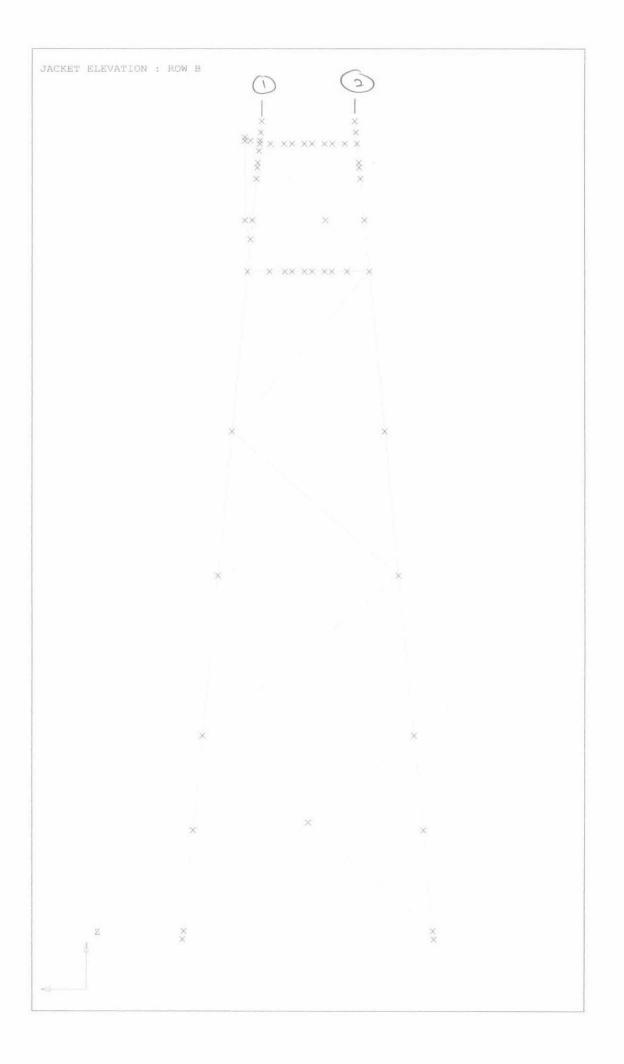
**BRACING SCHEMES** 

# A) DESIGN BASIS

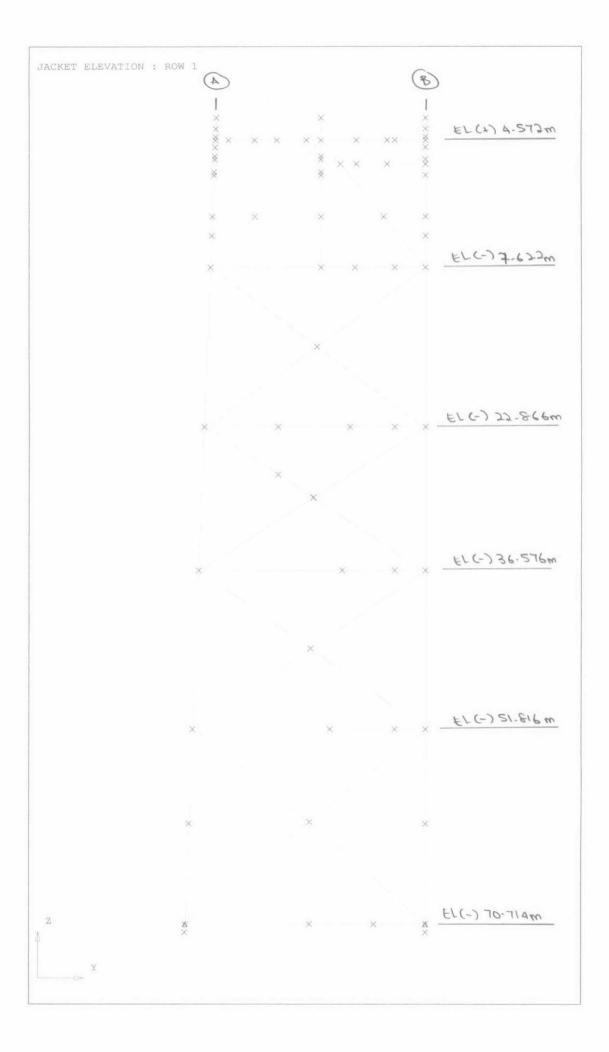


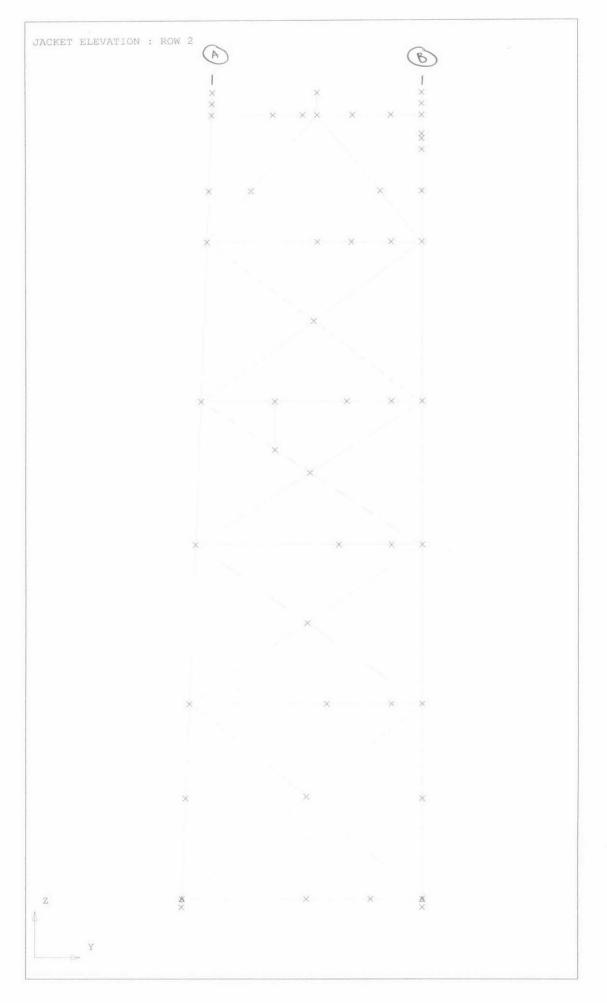


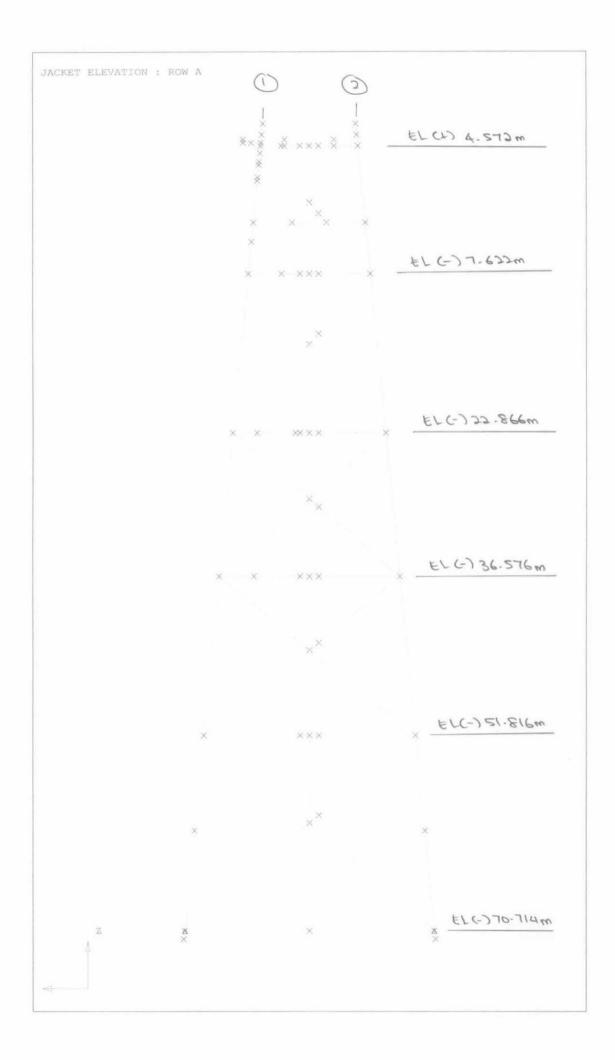


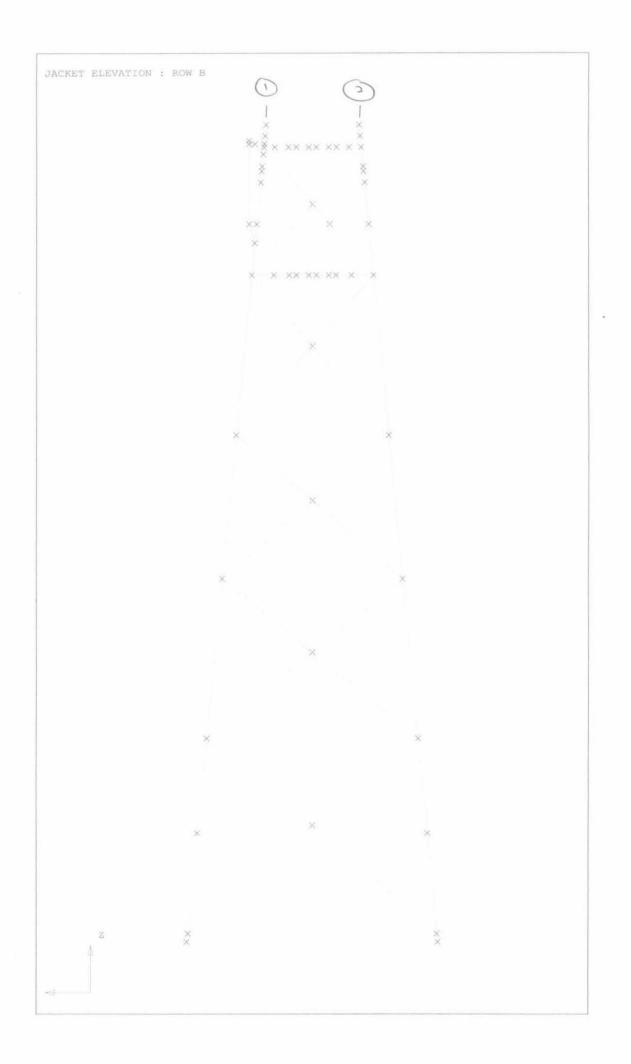


B) X-BRACING

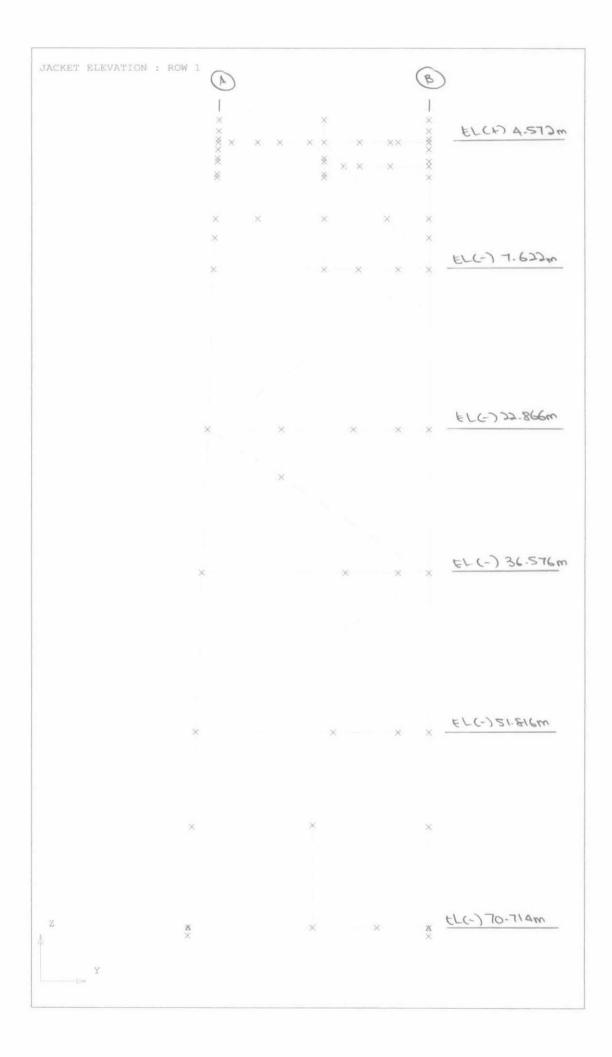


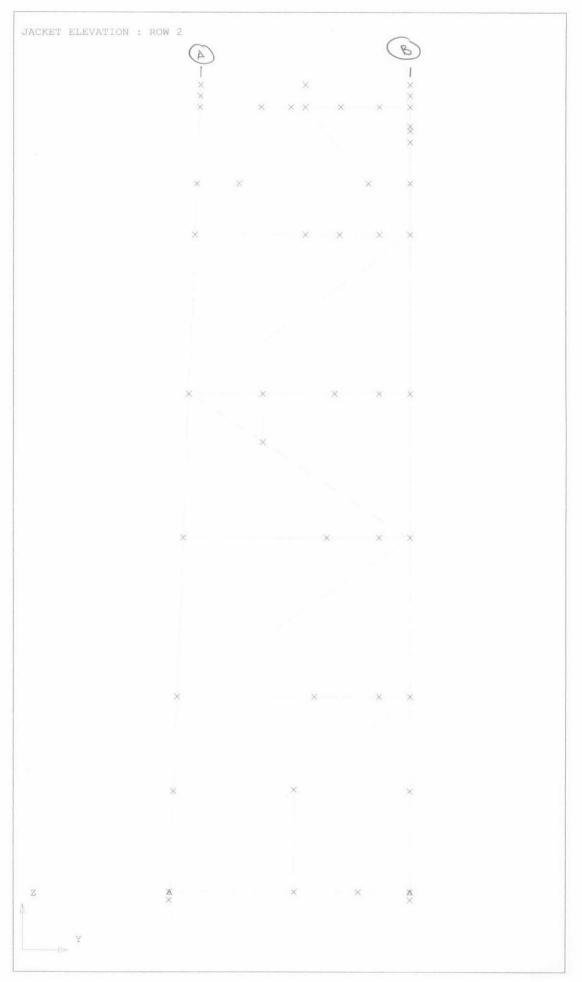


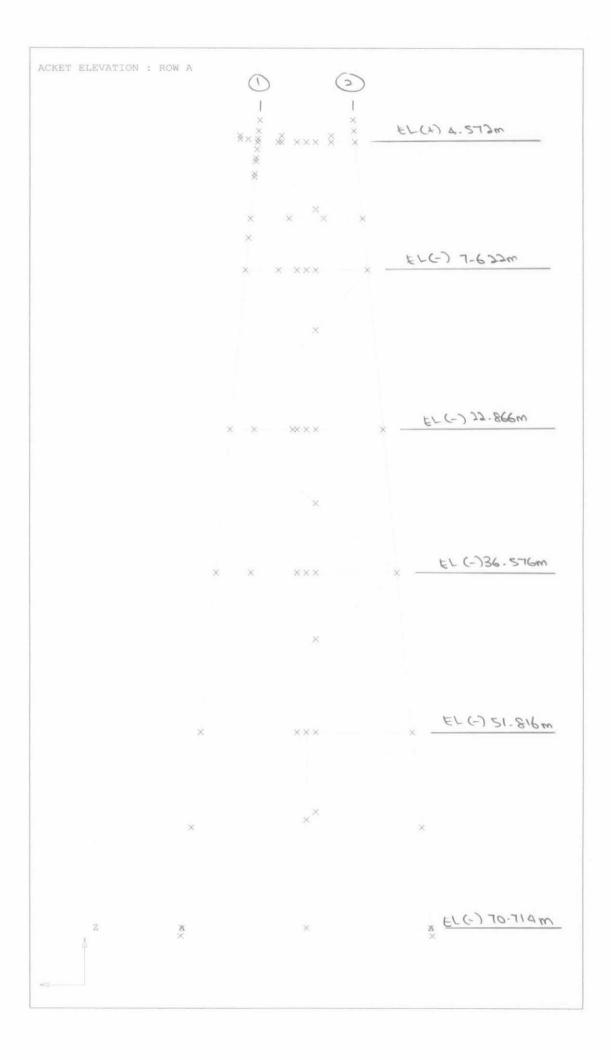


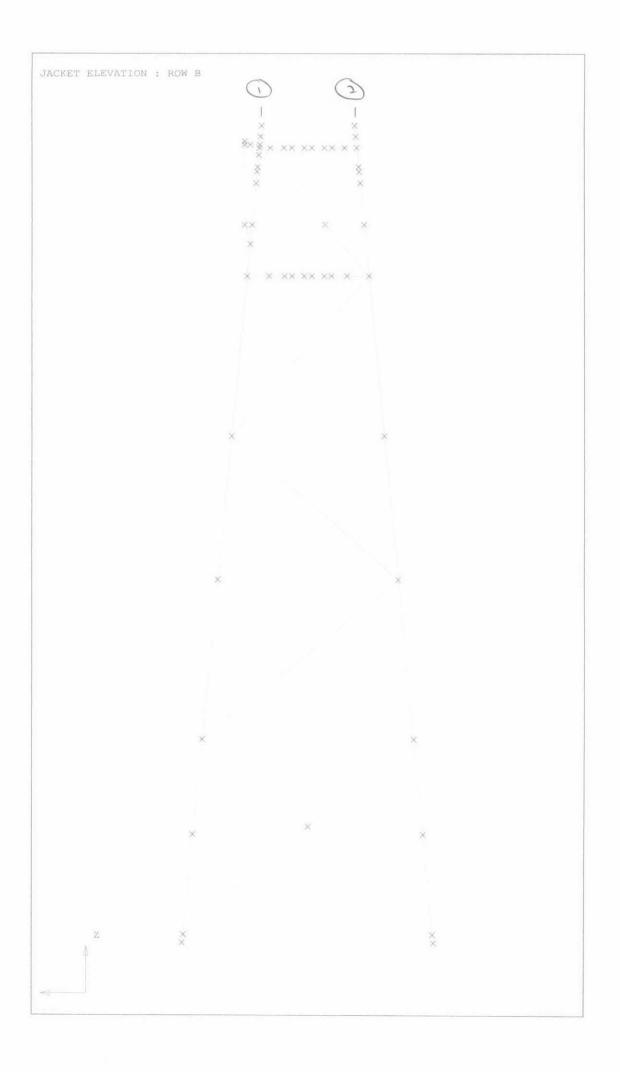


C) SINGLE DIAGONAL BRACING

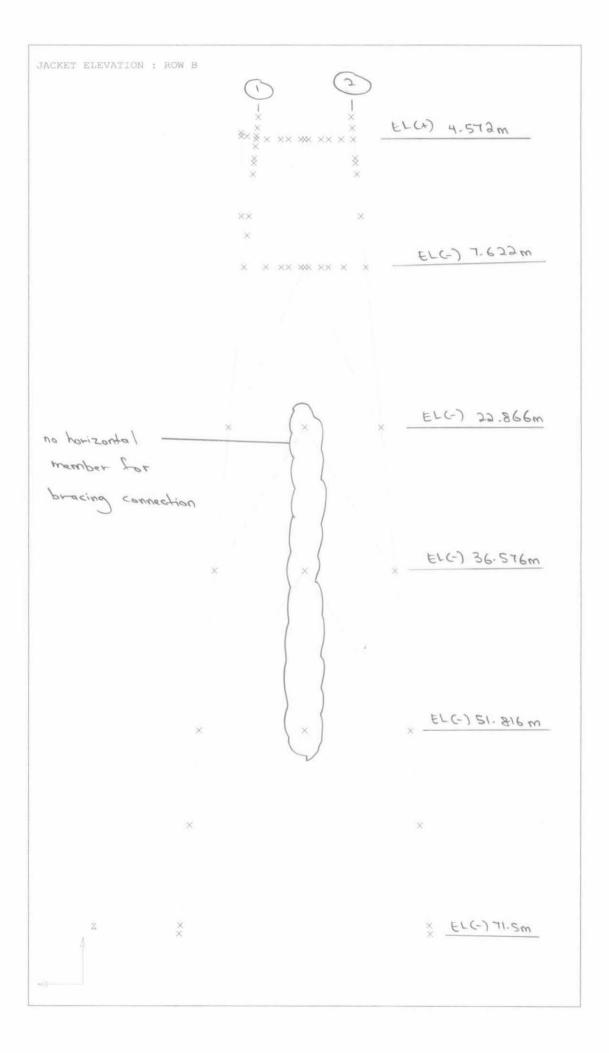








PLATFORM A K-BRACING PROBLEM



### APPENDIX C

APPENDIX C.1	BASIC LOAD CASE SUMMARY
	A) PLATFORM A
	B) PLATFORM B
	C) PLATFORM C
APPENDIX C.2	COLLAPSE OUTPUT SUMMARY
	A) PLATFORM A
	B) PLATFORM B
	C) PLATFORM C
APPENDIX C.3	TABULATE DATA OF BOTH METHOD
	A) PLATFORM A,B & C COLLAPSE COMPARISON
	<b>B) BRACING CONFIGURATION</b>
APPENDIX C.4	COLLAPSE RESTART FILE
	A) PLATFORM A
	B) PLATFORM B
	C) PLATFORM C
APPENDIX C.5	STATISTICAL ANALYSIS OF DATA
	A) PLATFORM A,B & C COLLAPSE COMPARISON
	B) BRACING CONFIGURATION

BASIC LOAD CASE SUMMARY

A) PLATFORM A

0	DEG
---	-----

#### LOAD LOAD FX FY DEAD BUOYANCY FZ MX MY MZ CASE LABEL LOAD (KN) (KN) (KN) (KN-M) (KN-M) (KN-M) (KN) (KN) -0.004 -7555.792 8004.73 1565.62 12531.61 0 -3690.873 -27084.5 117.613 -896.628 11997.2 -17.479 -554.484 2681.48 -232.77 -88.96 1464.73 -240.935 4185.64 -639.039 10352.6 -183.34 Ö • -466.248 1876.04 -5.467 Ō Ö -381.952 1359.35 -348.66 ō -399.092 366.972 53.356 108.99 0 9372.12 1272.9 61.824 -5163.02 ō ō 0 977.501 Õ Ő -974.018 -18.143 -151.232 490.182 611.712 0.14 -854.79 Õ Õ 2343.861 12.021 -131.725 -919.131 -4230

#### SEASTATE BASIC LOAD CASE SUMMARY REALTIVE TO MUDLINE ELEVATION

BASE SHEAR 2343.89 KN OVERTURNING MOMENT 137319 KN-M

42.11 DEG

#### SEASTATE BASIC LOAD CASE SUMMARY REALTIVE TO MUDLINE ELEVATION

÷

LOAD	LOAD	FX	FY	FZ	MX	MY	MŻ	DEAD	BUOYANCY
CASE	LABEL							LOAD	
		(KN)	(KN)	(KN)	(KN-M)	(KN-M)	(KN-M)	(KN)	(KN)
1	1	0	-0.004	-7555.792	8004.73	1565.62	0	20087	12531.61
2	2	0	0	-3690.873	-27084.5	117.613	0	0	0
3	3	0	0	-896.628	11997.2	-17.479	· 0	0	0
4	4	0	0	-554.484	2681.48	-232.77	0	0	0
5	5	0	0	-88.96	1464.73	Ö	0	0	0
6	6	0	0	-240.935	4185.64	0	0	0	0
7	7	0	0	-639.039	10352.6	-183.34	0	0	0
8	8	0	0	-466.248	1876.04	-5.467	0	0	0
9	9	0	0	-381.952	1359.35	-348.66	0	0	0
10	10	0	0	-399.092	366.972	53.356	0	0	0
11	11	<u>1</u> 08.99	0	0	0	9372.12	1272.9	0	0
12	12	0	61.824	0	-5163.02	0	0	0	0
13	13	0	0	0	0	977.501	0	0	0
14	14	0	0	0	-974.018	-18.143	0	0	0
15	15	0	0	-151 232	490.182	611.712	0	0	0
16	16	0	0.14	0	-854.79	0	Ö	0	0
17	100	2069.664	1792.19	-156.673	-101744	117985	-3585	0	0

BASE SHEAR 2737.78 KN OVERTURNING MOMENT 155796 KN-M

REALITVE TO MODLINE ELEVATION											
LOAD	LOAD	FX	FY	FZ	MX	MY	MZ	DEAD	BUOYANCY		
CASE	LABEL							LOAD			
		(KN)	(KN)	(KN)	(KN-M)	(KN-M)	(KN-M)	(KN)	(KN)		
1	1	0	-0.004				0	20087	12531.61		
2	2	· 0	0	-3690.873	-27084.5	117.613	0	0	0		
3	3	0	0	-896.628	11997.2	-17.479	0	0	0		
4	4	0	0	-554.484	2681.48	-232.77	0	0	0		
5	5	0	0	-88.96	1464.73	0	0	0	0		
6	6	0	0	-240.935	4185.64	, <b>O</b>	0	0	0		
7	7	0	0	-639.039	10352.6	-183.34	Ö	0	0		
8	8	0	0	-466.248	1876.04	-5.467	0	0	0		
9	9	0	0	-381.952	1359.35	-348.66	0	0	0		
10	10	0	0	-399.092	366.972	53.356	0	0	0		
11	11	108.99	0	0	0	9372.12	1272.9	0	0		
12	12	0	61.824	0	-5163.02	0	0	0	0		
13	13	0	0	0	0	977.501	0	0	0		
14	14	0	0	0	-974.018	-18.143	0	0	0		
15	15	0	0	-151.232	490.182	611.712	0	Q	0		
16	16	0	0.14	0	-854.79	0	0	0	0		
17	100	4.512	5958.8	-446.409	-342472	566.674	2381.2	0	0		

#### SEASTATE BASIC LOAD CASE SUMMARY REALTIVE TO MUDLINE ELEVATION

BASE SHEAR 5958.8 KN OVERTURNING MOMENT 342472 KN-M

137.89 DEG

#### SEASTATE BASIC LOAD CASE SUMMARY REALTIVE TO MUDLINE ELEVATION

LOAD	LOAD	FX	FY	FZ	MX	MY	MZ.	DEAD	BUOYANCY
	LABEL							LOAD	
		(KN)	(KN)	(KN)	(KN-M)	(KN-M)	(KN-M)	(KN)	(KN)
								. <u></u>	
1	1	0	-0.004	-7555.792	8004.73	1565.62	0	20087	12531.61
2	2	0	0	-3690.873	-27084.5	117.613	0	0	0
3	3	0	0	-896.628	11997.2	-17.479	0	0	0
4	4	0	0	-554.484	2681.48	-232.77	0	0	0
5	5	0	0	-88.96	1464.73	0	0	0	0
6	6	0	0	-240.935	4185.64	0	0	0	0
7	7	0	0	-639.039	10352.6	-183.34	0	0	0
8	8	0	0	-466.248	1876.04	-5.467	0	0	0
9	9	0	0	-381.952	1359.35	-348.66	0	0	0
10	10	0	0	-399.092	366.972	53.356	0	0	0
11	11	108.99	0	0	0	9372.12	1272.9	0	0
12	12	0	61.824	0	-5163.02	0	0	0	0
13	13	0	0	0	0	977.501	0	0	0
14	14	0	0	0	-974.018	-18.143	0	0	0
15	15	0	0	-151.232	490.182	611.712	0	0	0
16	16	0	0.14	0	-854.79	0	0	0	0
17	100	-2603.964	2254.1	-182.276	-126804	-146654	6397.9	0	0

.

BASE SHEAR3444.07 KNOVERTURNING MOMENT193873 KN-M

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REALITYE TO MODELINE ELEVATION										
LOAD	LOAD	FX	FY	FZ	MX	MY	MZ	DEAD	BUOYANCY	
CASE	LABEL							LOAD		
		(KN)	(KN)	(KN)	(KN-M)	(KN-M)	(KN-M)	(KN)	(KN)	
1	1	0	-0.004	~7555.792	8004.73	1565.62	0	20087	12531.61	
2	2	0	0	-3690.873	-27084.5	117.613	0	0	0	
3	3	0	0	-896.628	11997.2	-17.479	0	0	0	
4	4	0	0	-554.484	2681.48	-232.77	0	0	0	
5	5	0	0	-88.96	1464.73	0	0	0	0	
6	6	0	0	-240.935	4185.64	0	0	0	0	
7	7	0	0	639.039	10352.6	-183.34	0	0	0	
8	8	0	0	-466.248	1876.04	-5.467	0	0	0	
9	9	0	0	-381.952	1359.35	-348.66	0 1	0	0	
10	10	0	0	-399.092	366,972	53.356	🧹 🛛 0	0	0	
11	11	108.99	0	0	0	9372.12	1272.9	0	0	
12	12	0	61.824	0	-5163.02	Ő	0	0	0	
13	13	0	0	0	0	977.501	0	0	0	
14	14	0	0	0	-974.018	-18.143	0	0	0	
15	15	0	0	-151.232	490.182	611.712	0	0	0	
16	16	. 0	0.14	.0	<u>-854.79</u>	0	0	0	0	
17	100	-3616.522	8.039	-62.949	-485.079	-204112	6274.8	0	0	

### SEASTATE BASIC LOAD CASE SUMMARY REALTIVE TO MUDLINE ELEVATION

BASE SHEAR	3616.53 KN
OVERTURNING MOMENT	204112 KN-M

222.11 DEG

#### SEASTATE BASIC LOAD CASE SUMMARY REALTIVE TO MUDLINE ELEVATION

LOAD	LOAD	FX	FY	FZ	MX	MY	MZ	DEAD	BUOYANCY
	LABEL							LOAD	
		(KN)	(KN)	(KN)	(KN-M)	(KN-M)	(KN-M)	(KN)	(KN)
		•							
1	1	0	-0.004	-7555.792	8004.73	1565.62	0	20087	12531.61
2	2	0	0	-3690.873	-27084.5	117.613	0	0	0
3	3	0	0	-896.628	11997.2	-17.479	0	0	0
4	4	0	0	-554.484	2681.48	-232.77	0	0	0
5	5	0	0	-88.96	1464.73	0	0	0	0
6	6	0	0	-240.935	4185.64	0	0	0	0
7	7	0	0	-639.039	10352.6	-183.34	0	0	0
8	8	0	0	-466.248	1876.04	-5.467	0	0	0
9	9	0	0	-381.952	1359.35	-348.66	0	0	0
10	10	0	0	-399.092	366.972	53.356	0	0	0
11	11	108.99	0	0	0	9372.12	1272.9	0	0
12	12	0	61.824	0	-5163.02	0	0	0	0
13	13	0	0	0	0	977.501	0	0	0
14	14	0	0	0	-974.018	-18.143	0	0	0
15	15	0	0	-151.232	490.182	611.712	0	0	0
16	16	0	0.14	0	-854.79	0	0	Ó	0
17	100	-3026.835	-2577	-39.562	142775	-168190	4960.2	0	0

BASE SHEAR3975.24 KNOVERTURNING MOMENT220619 KN-M

		Lans 2		TIVE TO MC					
LOAD	LOAD	FX	FY	FZ	MX	MY	MZ	DEAD	BOUYANCY
CASE	LABEL							LOAD	
		(KN)	(KN)	(KN)	(KN-M)	(KN-M)	(KN-M)	(KN)	(KN)
1	1	0	-0.004		8004.73	1565.62	0	20087	<u>1</u> 2531.61
2	2	0	0	-3690.873	-27084.5	117.613	0	0	0
3	3	0	0	-896.628	11997.2	-17.479	0	0	0
4	4	0	0	-554.484	2681.48	-232.77	0	0	0
5	5	0	0	-88.96	1464.73	0	0	0	0
6	6	· 0	0	-240.935	4185.64	, 0	0	0	0
7	7	0	0	-639.039	10352.6	-183.34	0	0	0
8	8	0	0	-466.248	1876.04	-5.467	0	0	0
9	9	0	0	-381.952	1359.35	-348.66	0	0	0
10	10	0	0	-399.092	366.972	53.356	0	0	0
11	11	108.99	0	0	0	9372.12	1272.9	0	0
12	12	0	61.824	0	-5163.02	0	0	0	0
13	13	0	0	0	0	977.501	0	0	0
14	14	0	0	0	-974.018	-18.143	0	0	0
15	15	0	0	-151.232		611.712	0	0	0
16	16	0	0.14	0	-854.79	0	0	0	0
17	100	-0.173	-2043.8	-9.149	112977	-78.247	-582.5	0	0

#### SEASTATE BASIC LOAD CASE SUMMARY REALTIVE TO MUDLINE ELEVATION

BASE SHEAR	2043.84 KN
OVERTURNING MOMENT	112977 KN-M

#### 317.89 DEG

#### SEASTATE BASIC LOAD CASE SUMMARY REALTIVE TO MUDLINE ELEVATION

j.

LOAD	LOAD	FX	FY	FZ	MX	MY	MZ	DEAD	BOUYANCY
CASE	LABEL							LOAD	
		(KN)	(KN)	(KN)	(KN-M)	(KN-M)	(KN-M)	(KN)	(KN)
1	1	0	-0.004	-7555.792	8004.73	1565.62	0	20087	12531.61
2	2	0	0	-3690.873	-27084.5	117.613	0	0	0
3	3	0	0	-896.628	11997.2	-17.479	0	0	0
4	4	0	0	-554.484	2681.48	-232.77	0	0	0
5	5	0	0	-88.96	1464.73	0	0	0	0
6	6	0	0	-240.935	4185.64	0	0	0	0
7	7	0	0	-639.039	10352.6	-183.34	0	0	0
8	8	0	0	-466.248		-5.467	0	0	0
9	9	0	0	-381.952	1359.35	-348.66	0	0	0
10	10	0	0	-399.092	366.972	53.356	0	0	0
11	11	108.99	0	0	0	9372.12	1272.9	0	0
12	12	0	61.824	0	-5163.02	0	0	0	0
13	13	0	0	0	0	977.501	0	0	0
14	14	0	0	0	-974.018	-18.143	0	0	0
15	15	0	0	-151.232	490.182	611.712	0	0	0
16	16	0	0.14	0	-854.79	0	0	0	0
17	100	1666.946	-1436.8	-49.595	82520.9	96268.6	-4178	0	0

BASE SHEAR2200.71 KNOVERTURNING MOMENT126796 KN-M

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B) PLATFORM B

0 deg

SEASTATE BASIC LOAD CASE SUMMARY											
RELATIVE TO MUDLINE ELEVATION											
LOAD	LOAD LABEL	FX	FY	FZ	MX	MY	MZ	DEAD LOAD	BUOYANCY		
		(KN)	(KN)	(KN)	(KN-M)	(KN-M)	(KN-M)	(KN)	(KN)		
1		0	0	-1742.768	-130.103	-2.465	0	3143.794	1401.026		
2	2	0	0	-401.648	-324.613	-30.646	0	0	0		
3	3	0	0	-94.325	20.274	0	0	0	0		
4	4	0	0	0	0	0	0	0	0		
5	5	0	0	0	0	0	0	0	0		
6	6	19.5	0	0	0	1628.542	-0.02	0	0		
7	7	12.145	0	0	0	1168.995	-29.869	0	0		
8	8	0	22.5	0	-1879.087	0	0	0	0		
9	9	0	12.145	0	-859.121	<b>`</b> 0	0	0	0		
10	10	0	0	0	0	0	0	0	0		
11	11	0	0	0	0	0	0	0	0		
12	12	0	0	0	0	0	0	0	0		
13	13	0	0	0	0	0	0	0	0		
14	14	0	0	0	0	0	0	0	0		
15	15	0	0	0	0	0	0	0	0		
16	16	0	0	0	0	0	0	0	0		
17	17	0	0	0	0	0	0	0	0		
18	18	0	0	0	0	0	0	0	0		
19	19	0	0	0	0	0	0	0			
20	100	1729,448	0.817	-20.268	-43.479	84248.52	-451.158	0	0		

Base Shear 1729.448 KN Overturning Moment 84248.53 KN-M

30 deg

SEASTATE BASIC LOAD CASE SUMMARY										
RELATIVE TO MUDLINE ELEVATION										
LOAD	LOAD	FX	FY	FZ	MX	MY	MZ	DEAD	BUOYANCY	
CASE	LABEL							LOAD		
		(KN)	(KN)	(KN)	(KN-M)	(KN-M)	(KN-M)	(KN)	(KN)	
							-			
1.	1	0	0	-1742.768	-130.103	-2.465	0	3143.794	1401.026	
2	2	0	0	-401.648	-324.613	-30.646	0	0	0	
3	3	0	0	-94.325	20.274	0	0	0	0	
4	4	0	0	0	0	0	0	0	0	
5	5	0	0	0	0	0	0	0	0	
6	6	19.5	0	0	0	1628.542	-0.02	0	0	
7	7	12.145	0	0	0	1168.995	-29.869	0	0	
8	8	0	22.5	0	-1879.087	0	0	0	0	
9	9	0	12.145	0	-859.121	0	0	0	0	
10	10	0	0	0	0	0	0	0	0	
11	11	0	0	0	0	0	0	0	0	
12	12	0	0	0	0	0	0	0	0	
13	13	0	0	0	0	0	0	0	0	
14	14	0	0	0	0	0	0	0	0	
15	15	0	0	0	0	0	0	0	0	
16	16	0	0	0	0	0	0	0	0	
17	17	0	0	0	0	0	0	0	0	
18	18	0	0	0	0	0	0	0	0	
19	19	0	0	0	0	Ó	0	0	0	
20	20	0	0	0	0	0	0	0	0	
21	100	_f433.036	846.232	-18.951	-44420.5	74528.24	-272.995	0	0	

Base Shear1664.242 KNOverturning Moment86761.97 KN-M

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			051051	C DAGIO I	010 0105	011044400	,		
					OAD CASE				
				and the second se	JDLINE ELE				0.00
LOAD	LOAD	FX	FY	FZ	MX	MY	MZ	DEAD	BUOYANCY
CASE	LABEL							LOAD	
		(KN)	(KN)	(KN)	(KN-M)	(KN-M)	(KN-M)	(KN)	(KN)
	4			4740 700	400 400	0.405		0440 704	1 101 000
1	1	0	0		-130.103	and the second se	0	3143.794	1401.026
2	2	0	0	-401.648	-324.613	_	0	0	0
3	3	0	0	-94.325	20.274	0	0	0	0
4	4	0	0	0	0	0	0	0	0
5	5	0	0	0	0	0	0	0	0
6	6	19.5	0	0	0	1628.542	-0.02	Ö	0
7	7	12.145	0	0	0	1168.995	-29.869	0	0
8	8	0	22.5	0	-1879.087	0	0	0	0
9	9	0	12.145	0	-859.121	0	0	0	0
10	10	0	0	0	0	0	0	0	0
11	11	0	0	0	0	0	0	0	0
12	12	0	0	0	0	0	0	0	0
13	13	0	0	0	0	0	0	0	0
14	14	0	0	0	0	0	0	0	0
15	15	0	0	0	0	0	0	0	0
16	16	0	0	0	0	0	0	0	0
17	17	0	0	0	0	0	0	0	0
18	18	0	0	0	0	0	0	0	0
19	19	0	0	0	0	0	0	0	0
20	100	870.378	1539.713	-14.42	-75957.42	42404.62	-60.962	0	0

Base Shear Overturning Moment 1768.693 KN 86992.42 KN-M

90 deg

SEASTATE BASIC LOAD CASE SUMMARY											
					IDLINE ELE						
LOAD	LOAD	FX	FY	FZ	MX	MY	MZ	DEAD	BUOYANCY		
CASE	LABEL							LOAD	1		
		(KN)	(KN)	(KN)	(KN-M)	(KN-M)	(KN-M)	(KN)	(KN)		
	1	0	0	-1742.768	-130.103	-2.465	0	3143.794	1401.026		
2	2	0	0		-324.613	-30.646	0	0	0		
3	3	0	0	-94.325	20.274	0	0	Ō	ō		
4	4	0	0		0	0	0	0	0		
5	5	0	0	0	0	0	0	0	0		
6	6	19.5	0	0	0	1628.542	-0.02	0	0		
7	7	12.145	0	0	0	1168.995	-29.869	0	0		
8	8	0	22.5	0	-1879.087	0	0	0	0		
9	9	0	12.145	0	-859.121	0	0	0	0		
10	10	0	0	0	0	0	0	0	0		
11	11	0	0	0	0	0	0	0	0		
12	12	0	0	0	0	0	0	0	0		
13	13	0	0	0	0	0	0	0	0		
14	14	0	0	0	0	0	0	0	0		
15	15	0	0	0	0	0	0	0	0		
16	16	0	0	0	0	0	0	0	0		
17	17	0	0	0	0	0	0	0	0		
18	18	0	0	0	0	0	0	0	0		
19	19	0	0	0	0	0	0	0	0		
20	100	-0.392	1712.928	-26.264	-90279.21	-43.817	0.968	0	0		

Base Shear 1712.928 KN Overturning Moment 90279.22 KN-M

SEASTATE BASIC LOAD CASE SUMMARY										
				-	JDLINE ELE					
LOAD	LOAD	FX	FY	FZ	MX	MY	MZ	DEAD	BUOYANCY	
CASE	LABEL							LOAD		
		(KN)	(KN)	(KN)	(KN-M)	(KN-M)	(KN-M)	(KN)	(KN)	
1	1	0	0	-1742.768		-2.465	0	3143.794	1401.026	
2	2	0	0	-401.648	-324.613	-30.646	0	0	0	
3	3	0	0	-94.325	20.274	0	0	0	0	
4	4	0	0	0	0	0	0	0	0	
5	5	0	0	0	0	0	0	0	0	
6	6	19.5	0	0	0	1628.542	-0.02	0	0	
7	7	12.145	0	0	0	1168.995	-29.869	0	0	
8	8	0	22.5	0	-1879.087	0	0	0	0	
9	9	0	12.145	0	-859.121	, O	0	0	0	
10	10	0	0	0	0	0	0	0	0	
11	11	0	0	0	0	0	0	0	0	
12	12	0	0	0	0	0	0	0	0	
13	13	0	0	0	0	0	0	0	0	
14	14	0	0	0	0	0	0	0	0	
15	15	0	0	0	0	0	0	0	0	
16	16	0	0	0	0	0	0	0	0	
17	17	0	0	0	0	0	0	0	0	
18	18	0	0	0	0	0	0	0	0	
19	19	0	0	0	0	0	0	0	0	
20	100	-871,174	1540.159	-17.243	-75952.54	-42498.8	70.272	0	0	

Base Shear1769.473 KNOverturning Moment87034.11 KN-M

150 deg

SEASTATE BASIC LOAD CASE SUMMARY											
			+ +	TIVE TO MU							
LOAD	LOAD	FX	FY	FZ	MX	MY	MZ	DEAD	BUOYANCY		
CASE	LABEL							LOAD			
		(KN)	(KN)	(KN)	(KN-M)	(KN-M)	(KN-M)	(KN)	(KN)		
1	1	0	0		-130,103		0	3143.794	1401.026		
2	2	0	0		-324.613		0	0	0		
3	3	0	0	-94.325	20.274	0	0	0	0		
4	4	0	0	0	0	0	0	0	0		
5	5	0	0	0	0	Ō	0	0	0		
6	6	19.5	0	0	0	1628.542	-0.02	0	0		
7	7	12.145	0	0	0	1168.995	-29.869	0	0		
8	8	0	22.5	0	-1879.087	0	0	0	0		
9	9	0	12.145	0	-859.121	0	0	0	0		
10	10	0	0	0	0	0	0	0	0		
11	11	Ō.	0	0	0	0	0	0	0		
12	12	0	Ó	0	0	0	0	0	0		
13	13	0	0	0	0	0	0	0	0		
14	14	0	0	0	0	0	0	0	0		
15	15	0	0	0	0	0	0	0	0		
16	16	0	0	0	0	0	0	0	0		
17	17	0	0	0	0	0	0	0	Ō		
18	18	Ō	0	0	0	0	0	0	0		
19	19	0	0	0	0	0	0	0	0		
20	100	-1434.237	846.138	-18.752	-44358.67	-74635.63	292.861	0	0		

Base Shear 1665.228 KN Overturning Moment 86822.63 KN-M

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SEASTATE BASIC LOAD CASE SUMMARY											
					JDLINE EL						
LOAD	LOAD	FX	FY	FZ	MX	MY	MZ	DEAD	BUOYANCY		
CASE	LABEL							LOAD			
		(KN)	(KN)	(KN)	(KN-M)	(KN-M)	(KN-M)	(KN)	(KN)		
				4740 700	400 400	2.405		0440 704	1/04 000		
1	1	0	0		-130.103		0	3143.794	1401.026		
2	2	0	0	-401.648	-324.613		0	0	0		
3	3	0	0	-94.325	20.274		0	0	0		
4	4	- 0	0	0	0	0	0	0	0		
5	5	0	0	0	0	0	0	0	0		
6	6	19.5	0	0	0	1628.542	-0.02	0	0		
7	7	12.145	0	0	0	1168.995	-29.869	0	0		
8	8	0	22.5	0	-1879.087	0	0	0	0		
9	9	0	12.145	0	-859.121	O,	0	0	0		
10	10	0	0	0	0	0	0	0	0		
11	11	0	0	0	0	0	0	0	0		
12	12	0	0	0	0	0	0	0	0		
13	13	0	0	0	0	0	0	0	0		
14	14	0	0	0	0	0	0	0	0		
15	15	0	0	0	0	0	0	0	0		
16	16	0	0	0	0	0	0	0	0		
17	17	0	0	0	0	0	0	0	0		
18	18	0	0	0	0	0	0	0	0		
19	19	0	0	0	0	0	0	0	0		
20	100	-1729.941	1.311	-17.276	-18.783	-84283.04	456.065	0	0		

Base Shear1729.941 KNOverturning Moment84283.04 KN-M

210 deg

SEASTATE BASIC LOAD CASE SUMMARY											
			RELAT	IVE TO MU	<b>JDLINE ELE</b>	EVATION					
LOAD	LOAD	FX	FY	FZ	MX	MY	MZ	DEAD	BUOYANCY		
CASE	LASEL							LOAD			
		(KN)	(KN)	(KN)	(KN-M)	(KN-M)	(KN-M)	(KN)	(KN)		
1	1	0	0	-1742.768	-130.103		0	3143.794	1401.026		
2	2	0	0	-401.648	-324.613	-30.646	0	0	0		
3	3	0	0	-94.325	20.274	0	0	0	0		
4	4	0	0	0	0	0	0	0	0		
5	5	0	0	0	0	0	0	0	0		
6	6	19.5	0	0	0	1628.542	-0.02	0	0		
7	7	12.145	0	0	0	1168.995	-29.869	0	0		
8	8	0	22.5	0	-1879.087	0	0	0	0		
9	9	0	12,145	0	-859 121	0	0	0	0		
10	10	0	0	0	0	0	0	0	0		
11	11	0	0	0	0	0	0	0	0		
12	12	0	0	0	0	0	0	0	0		
13	13	0	0	0	0	0	Û	0	0		
14	14	0	0	0	0	0	0	0	0		
15	15	0	0	0	0	0	0	0	0		
16	16	0	0	0	0	0	0	0	0		
17	17	0	0	0	0	0	0	0	0		
18	18	0	0	0	0	0	0	0	0		
19	19	0	0	0	0	0	0	0	0		
20	100	-1433.949	-845.971	-27.759	44415.64	-74626.91	280.036	0	0		

Base Shear 1664.895 KN Overturning Moment 86844.25 KN-M

	SEASTATE BASIC LOAD CASE SUMMARY RELATIVE TO MUDLINE ELEVATION											
LOAD CASE	LOAD LABEL	FX	FY	FZ	MX	MY	MZ	DEAD LOAD	BUOYANCY			
		(KN)	(KN)	(KN)	(KN-M)	(KN-M)	(KN-M)	(KN)	(KN)			
1	1	0	0	-1742.768	-130.103	-2.465	0	3143.794	1401.026			
2	2	0	0	-401.648	-324.613	-30.646	0	0	0			
3	3	0	0	-94.325	20.274	0	0	0	C			
4	4	0	0	0	0	0	0	0				
5	5	0	0	0	Ō	0	0	0	0			
6	6	19.5	0	0	0	1628.542	-0.02	0	C			
7	7	12,145	0	0	0	1168.995	-29.869	0				
8	8	0	22.5	0	-1879.087	0	0	0	0			
9	9	0	12.145	0	-859.121	Ū	0	0	(			
10	10	.0	0	0	0	0	0	0	(			
11	11	0	0	0	0	0	0	0	C			
12	12	0	0	0	0	0	0	0	(			
13	13	0	0	0	0	0	0	0				
14	14	0	0	0	0	0	0	0	(			
15	15	0	0	0	0	0	0	0	C			
16	16	0	0	0	0	0	0	0	C			
17	17	0	0	0	0	0	0	0	C			
18	18	0	0	0	0	0	0	0	0			
19	19	0	0	0	0	0	0	0				
20	100	-869.027	-1541.813	-15.601	76040.34	-42373.26	62.365	0	(			

Base Shear 1769.857 KN Overturning Moment 87049.56 KN-M

270 deg

SEASTATE BASIC LOAD CASE SUMMARY											
				-	JOLINE ELE						
LOAD	LOAD	FX	FY	FZ	MX	MY	MZ	DEAD	BUOYANCY		
CASE	LABEL							LOAD			
		(KN)	(KN)	(KN)	(KN-M)	(KN-M)	(KN-M)	(KN)	(KN)		
1	1	0	0	1742.768	-130.103		0	3143.794	1401.026		
2	2	0	0	-401.648	-324.613	-30.646	0	0	0		
3	3	0	0	-94.325	20.274	0	0	0	0		
4	4	0	0	0	0	0	0	0	0		
5	5	0	0	0	0	0	0	0	0		
6	6	19.5	0	0	0	1628.542	-0.02	0	0		
7	7	12.145	0	0	0	1168.995	-29.869	0	0		
8	8	0	22.5	0	-1879.087	0	0	0	0		
9	9	0	12.145	0	-859.121	0	0	0	0		
10	10	0	0	0	0	0	0	0	0		
11	11	0	0	0	0	0	0	0	0		
12	12	0	0	0	0	0	0	0	0		
13	13	0	0	0	0	0	Ó	0	0		
14	14	0	0	0	0	0	0	0	0		
15	15	0	0	0	0	0	0	0	0		
16	16	0	0	0	0	0	0	0	0		
17	17	. 0	0	0	0	0	0	0	0		
18	18	0	0	0	0	0	0	0	0		
19	19	0	0	0	0	0	0	0	0		
20	100	1.176	-1715.552	-12.378	90399.77	106.002	6.213	0	0		

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Base Shear 1715.552 KN Overturning Moment 90399.83 KN-M

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SEASTATE BASIC LOAD CASE SUMMARY											
					JDLINE ELE	*					
LOAD	LOAD	FX	FY	FZ	MX	MY	MZ	DEAD	BUOYANCY		
CASE	LABEL							LOAD			
		(KN)	(KN)	(KN)	(KN-M)	(KN-M)	(KN-M)	(KN)	(KN)		
1	1	0	0	-1742.768	-130.103	-2.465	0	3143.794	1401.026		
2	2	0	0	-401.648	-324.613	-30.646	0	0	0		
3	3	0	0	-94.325	20.274	0	0	0	0		
4	4	0	0	0	0	0	0	0	0		
5	5	0	0	0	0	0	0	0	0		
6	6	19.5	0	0	0	1628.542	-0.02	0	0		
7	7	12.145	0	0	0	1168.995	-29.869	0	0		
8	8	0	22.5	0	-1879.087	0	0	0	0		
9	9	0	12.145	0	-859.121	,0	0	Ó	0		
10	10	0	0	0	0	0	0	0	0		
11	11	0	0	0	0	0	0	0	0		
12	12	0	0	0	0	0	0	0	0		
13	13	0	0	0	0	0	0	0	0		
14	14	0	0	0	0	0	0	0	0		
15	15	0	0	0	0	0	0	0	0		
16	16	Ő	0	0	0	0	0	0	0		
17	17	0	0	0	0	0	0	0	0		
18	18	0	0	0	0	0	0	0	0		
19	19	0	0	0	0	0	0	0	0		
20	100	870.264	-1543.076	-12.643	76093.23	42499.33	-61.657	0	0		

Base Shear1771.565 KNOverturning Moment87157.17 KN-M

330 deg

SEASTATE BASIC LOAD CASE SUMMARY											
		_			DLINE ELE		_				
LOAD	LOAD	FX	FY	FZ	MX	MY	MZ	DEAD	BUOYANCY		
CASE [	LABEL							LOAD	( )		
		(KN)	(KN)	(KN)	(KN-M)	(KN-M)	(KN-M)	(KN)	(KN)		
1	1	0	0		-130.103	-2.465	0	3143.794	1401.026		
2	2	0	0	-401.648	-324.613	-30.646	0	0			
3	3	0	0	-94.325	20.274	0	0	0			
4	4	0	0	0	0	0	0	0	0		
5	5	0	0	0	0	0	0	0	0		
6	6	19.5	0	0	0	1628.542	-0.02	0	0		
7	7	12.145	0	0	0	1168.995	-29.869	0	0		
8	8	0	22.5	0	-1879.087	0	0	0	0		
9	9	0	12.145	0	-859.121	0	0	0	0		
10	10	0	0	0	0	_0	0	0	0		
11	11	0	0	0	0	0	Ö	0	0		
12	12	0	0	0	0	0	0	0	0		
13	13	0	0	0	0	0	0	0	0		
14	14	0	0	0	Ő	0	0	0	0		
15	15	0	0	0	0	0	0	0	0		
16	16	0	0	0	0	0	0	0	0		
17	17	0	0	0	0	0	0	0	0		
18	18	0	0	0	0	0	0	0	0		
19	19	0	0	0	0	0	0	0	0		
20	100	1434.307	-847.227	-27.72	44460.6	74669.44	-285.598	0	0		

Base Shear Overturning Moment

1665.842 KN 86903.8 KN-M APPENDIX C.1 C) PLATFORM C

	SEASTATE BASIC LOAD CASE SUMMARY										
			RELA	TIVE TO MI	JDLINE ELE	VATION					
LOAD	LOAD	FX	FY	FZ	MX	MY	MZ	DEAD	BUOYANCY		
CASE	LABEL							LOAD			
		(KN)	(KN)	(KN)	(KN-M)	(KN-M)	(KN-M)	(KN)	(KN)		
1	1	-0.001	-0.002	-21550.17	602.065	91823.97	-0.036	53292.47	31742.344		
2	2	-0.001	-0.003	-21341.38	913.463	92086.56	-0.038	53292.45	31951.346		
3	3	Ö	0	-5993.295	12694.13	10106.8	Ö	0	0		
4	4	0	0	521.589	-1882.398	-1800.032	Ö	0	0		
5	5	0	0	-1025.357	1015.729	706.716	0	0	0		
6	6	0	0	-29662.7	-33156.4	46953.47	0	0	0		
7	7	0	0	-11373.35	-15079.5	159451.4	Ō	0	0		
8	8	0	0	-19362.62	-14889.95	-264298.3	Ő	0	0		
9	9	0	0	0	0	0	0	0	0		
10	10	0	0	0	0	0	0	0	0		
11	29	2467.033	-0.336	-6.44	34.155	223544.1	-1915.494	0	0		
12	30	-0.336	1459,773	-1.892	-130311.5	-25.831		0	0		
13	60	0	0	-5337.798	0.001	-34166.18		0	00		
14	61	. 0	0	-981	0	-6279.183	and the second	0	0		
15	62	0	0	0	0	39280.79	0	0	0		
16	63	0	0	0	39280.79	0	0	0	0		
17	100	3272.917	-138,688	48.629	6460.609	157933.2	-1151.046	0	0		

BASE SHEAR3275.854 KNOVERTURNING MOMENT158065.2 KN-M

65 DEG

			SEASTA	TE BASIC L	OAD CASE	SUMMARY	,		
			RELA	TIVE TO MI	JDLINE ELE	VATION			
LOAD	LOAD	FX	FY	FZ	MX	MY	MZ	DEAD	BUOYANCY
CASE	LABEL							LOAD	
		(KN)	(KN)	(KN)	( <u>KN-M</u> )	(KN-M)	(KN-M)	(KN)	(KN)
1	1	-0.001	-0.002	-21550.17	602.065	91823.97	-0.036	53292.47	31742.344
2	2	-0,001	-0.003	-21341.38		92086.56	the second s	53292.45	
3	3	0	0	-5993.295	12694.13	10106.8	0	0	0
4	4	0	0	521.589	-1882.398	-1800.032	0	Ő	0
5	5	0	0	-1025.357	1015.729	706.716	Ō	0	0
6	6	0	0	-29662.7	-33156.4	46953.47	0	0	0
7	7	0	0	-11373.35	-15079.5	159451.4	0	0	0
8	8	0	0	-19362.62	-14889.95	-264298.3	0	0	0
9	9	0	0	0	0	0	· 0	0	0
10	10	0	0	0	0	0	0	0	0
11	29	2467.033	-0.336	-6.44	34.155	223544.1	-1915.494	0	0
12	30	-0.336	1459.773	-1.892	-130311.5	-25.831	-1878.116	0	0
13	60	0	0		0.001	-34166.18	0	0	0
14	61	0	0	- <del>9</del> 81	0	6279.183	0	0	0
15	62	0	0	0	0	39280.79	0	0	0
16	63	0	0	0	39280.79	0	0	0	0
17	100	2003.297	6537.204	-341.862	-362201.1	106738.1	29685.84	0	0

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BASE SHEAR OVERTURNING MOMENT 6837.268 KN 377601.2 KN-M

			-	-	OAD CASE		,		
LOAD CASE	LOAD LABEL	FX	FY	FZ	MX	MY	MZ	DEAD LOAD	BUOYANCY
		(KN)	(KN)	(KN)	(KN-M)	(KN-M)	(KN-M)	(KN)	(KN)
1	1	-0.001	-0.002	-21550.17	602.065	91823.97	-0.036	53292.47	31742.344
2	2	-0.001	-0.003	-21341.38	913.463	92086.56	-0.038	53292.45	31951.346
3	3	0	0	-5993.295	12694.13	10106.8	0	0	0
4	4	0	0	521.589	-1882.398	-1800.032	0	0	0
5	5	0	0	-1025.357	1015.729	706.716	Ō	0	0
6	6	0	0	-29662.7	-33156.4	46953.47	0	0	0
7	7	0	0	-11373.35	-15079.5	159451.4	0	0	0
8	8	0	0	-19362.62	-14889.95	-264298.3	0	0	0
9	9	0	0	0	0	0	0	0	0
10	10	0	0	0	0	0	0	0	0
11	29	2467.033	-0.336	<u>-6</u> .44	34.155	223544.1	-1915.494	0	0
12	30	-0.336	1459,773	-1.892	-130311.5	-25.831	-1878.116	0	0
13	60	0	0	-5337.798	0.001	-34166.18		0	0
14	61	0	0	-981	0	-6279.183	0	0	÷
15	62	0	0	0	0	39280.79	0	0	0
16	63	0	0	0	39280.79	0	Ö	0	0
17	100	-449.589	12451.11	-609.837	-674823.9	-17611.53	57954.25	0	0

BASE SHEAR 12459.22 KN OVERTURNING MOMENT 675053.6 KN-M

115 DEG

					OAD CASE	SUMMARY			
LOAD CASE	LOAD LABEL	FX	FY	FZ	MX	MY	MZ	DEAD LOAD	BUOYANC
		(KN)	(KN)	(KN)	(KN-M)	(KN-M)	(KN-M)	(KN)	(KN)
1	1	-0.001	-0.002	-21550.17	602.065	91823.97	-0.036	53292.47	31742.34
2	2	-0.001	-0.003	-21341.38	913.463	92086.56	-0.038	53292.45	31951.34
3	3	0	0	-5993.295	12694.13	10106.8	0	0	
4	4	0	0	521.589	-1882.398	-1800.032	Ó	0	-
5	5	0	0	-1025.357	1015.729	706.716	0	0	
6	6	0	0	-29662.7	-33156.4	46953.47	0	0	
7	7	0	0	-11373.35	-15079.5	159451.4	Ō	0	
8	8	0	0	-19362.62	-14889.95	-264298.3	Ő	0	
9	9	0	0	0	0	0	0	0	
10	10	0	0	Ó	0	0	0	0	
11	29	2467:033	-0.336	-6.44	34.155	223544.1	-1915.494	0	
12	30	-0.336	1459.773	-1.892	-130311.5	-25.831	-1878.116	0	
13	60	0	0	-5337.798	0.001	-34166.18	Ō	0	
14	61	0	0	-981	0	-6279.183	Ő	0	
15	62	0	0	0	0	39280.79	0	0	
16	63	0	0	0	39280.79	0	0	0	
17	100	-3111.851	7490.794	-448.211	-397515.4	-153990	36023.64	0	

BASE SHEAR OVERTURNING MOMENT 426299.7 KN-M

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8111.449 KN

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			SEASTA	TE BASIC L	OAD CASE	SUMMARY	/		
			RELA	TIVE TO MI	JOLINE ELE	EVATION			
LOAD	LOAD	FX	FY	FZ	MX	MY	MZ	DEAD	BUOYANCY
CASE	LABEL							LOAD	
		(KN)	(KN)	(KN)	(KN-M)	(KN-M)	(KN-M)	(KN)	(KN)
1	1	-0.001	-0.002	-21550.17	602.065	91823.97	-0.036	53292.47	31742.344
2	2	-0.001	-0.003	-21341.38	913.463	92086.56	-0.038	53292.45	31951.346
3	3	0	0	-5993.295	12694.13	10106.8	0	0	0
4	4	0	0	521.589	-1882.398	-1800.032	0	0	0
5	5	0	0	-1025.357	1015.729	706.716	0	0	0
6	6	0	0	-29662.7	-33156.4	46953.47	0	0	0
7	7	0	0	-11373.35	-15079.5	159451.4	0	0	0
88	8	0	0	-19362.62	-14889.95	-264298.3	0	0	Q
9	9	0	0	0	0	0	0	0	0
10	10	0	0	0	0	0	0	0	0
11	29	2467.033	-0.336	-6.44	34.155	223544.1	-1915.494	0	0
12	30	-0,336	1459.773	-1.892	-130311.5	-25.831	-1878.116	0	0
13	60	0	0	-5337.798	0.001	-34166.18	0	0	0
14	61	0	0	-981	0	-6279.183	0	0	0
15	62	0	0	0	0	39280.79	0	0	0
16	63	0	0	0	39280.79	0	0	0	0
17	100	-3709.154	512.145	44.166	-24747.74	-183839.9	3707.86	0	0

BASE SHEAR OVERTURNING MOMENT 3744.345 KN 185498.2 KN-M

245 DEG

						SUMMARY	/		
					JDLINE ELE				
LOAD	LOAD	FX	FY	FZ	MX	MY	MZ	DEAD	BUOYANCY
CASE	LABEL (					l I		LOAD	
		(KN)	(KN)	(KN)	(KN-M)	(KN-M)	(KN-M)	(KN)	(KN)
1	1	-0.001	-0.002	-21550.17	602.065	91823.97	-0.036	53292.47	31742.344
2	2	-0.001	-0.003		913.463	92086.56	-0.038	53292.45	31951.346
3	3	0	0	-5993.295	12694.13	10106.8	0	0	0
4	4	0	0	521.589	-1882.398	-1800.032	0	0	0
5	5	0	0	-1025.357	1015.729	706.716	0	0	0
6	6	0	0	-29662.7	-33156.4	46953.47	0	0	0
7	7	0	0	-11373.35	-15079.5	159451.4	0	0	0
8	8	0	0	-19362.62	-14889.95	-264298.3	0	0	0
9	9	0	0	0	0	0	0	0	0
10	10	0	0	0	0	0	0	0	0
11	29	2467.033	-0.336	-6.44	34.155	223544.1	-1915.494	0	0
12	30	-0.336	1459.773	-1.892	-130311.5	-25.831	-1878.116	0	0
13	60	0	0	-5337.798	0.001	-34166.18	0	0	0
14	61	0	0	-981	0	-6279.183	0	0	0
15	62	0	0	0	0	39280.79	0	0	0
16	63	0	0	0	39280.79	0	0	Ô	0
17	100	-1523.64	-4077.495	-96.909	221108.1	-75227.88	-16086.08	0	0

BASE SHEAR OVERTURNING MOMENT 4352.866 KN 233555.2 KN-M

			SEASTA	TE BASIC L	OAD CASE	SUMMARY	/		
			RELA	TIVE TO MI	UDLINE EL	EVATION			
LOAD	LOAD	FX	FY	FZ	MX	MY	MZ	DEAD	BUOYANCY
CASE	LABEL							LOAD	
		(KN)	(KN)	(KN)	(KN-M)	(KN-M)	(KN-M)	(KN)	(KN)
	1	-0.001	-0.002	-21550.17	602.065	91823.97	-0.036	53292.47	31742.344
2	2	-0.001	-0.003					53292.45	
3		-0.001	-0.000	-5993.295	12694.13			00202.40	01901.040
4	4	0	0	521.589	-1882.398	the second se		0	
		· · · · · · · · · · · · · · · · · · ·	0	-1025.357	1015.729				0
5	5	0					the second s	0	0
6	6	0	0	-29662.7	-33156.4	46953.47	0	0	
7	7	0	0	-11373.35	-15079.5		0	0	0
8	8	0	0	-19362.62	-14889.95	-264298.3		00	0
9	9	0	0	0	0	0	0	0	0
10	10	0	0	0	0	Ō	0	0	0
11	29	2467.033	-0.336	-6.44	34.155	223544.1	-1915.494	0	0
12	30	-0.336	1459.773	-1.892	-130311.5	-25.831	-1878.116	0	0
13	60	0	0	-5337.798	0.001	-34166.18	0	0	0
14	61	0	0	-981	0	-6279.183		0	0
15	62	0	0	0	0	39280.79	0	0	0
16	63	0	0	0	39280.79	0	0	0	0
17	100	153,954	-4782.235	-275.377	260650.2	7885.17	-19253.19	0	C

BASE SHEAR	4784.712 KN
OVERTURNING MOMENT	260769.4 KN-M

295 DEG

					OAD CASE		/		
LOAD	LOAD	FX	FY	FZ	JDLINE ELE MX	VATION MY	MZ	DEAD	BUOYANCY
CASE	LABEL		••		,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,			LOAD	boornation
		(KN)	(KN)	(KN)	(KN-M)	(KN-M)	(KN-M)	(KN)	(KN)
1	1	-0.001	-0.002	-21550.17	602.065	91823.97	-0.036	53292.47	31742.344
2	2	-0.001	-0.003	-21341.38	913.463	92086.56	-0.038	53292.45	31951.346
3	3	0	0	-5993.295	12694.13	10106.8	0	0	0
4	4	0	0	521.589	-1882.398	-1800.032	0	0	0
5	5	0	0	-1025.357	1015.729	706.716	0	0	0
6	- 6	0	0	-29662.7	-33156.4	46953.47	0	0	0
7	7	0	0		-15079.5			<u> </u> 0	0
8	8	0	0	-19362.62	-14889.95	-264298.3		0	0
9	9	0	0	0	00	0	0	0	0
10	10	0	0	0	0	0	0	0	0
11	29	2467.033	-0.336		34.155	223544_1	-1915.494	0	0
12	30	<u>-0.336</u>	1459.773		-130311.5		-1878.116	0	0
13	60	0	0	-5337.798	0.001	-34166.18		0	0
14	61	0	0	-981	0	-6279.183		0	0
15	62	0	0	0	0	39280.79		0	0
16	63	0	0	0	39280.79	0	0	0	0
17	100	1829.113	-4078.759	<u>-151.393</u>	218746.6	89854.16	-20656.04	0	0

BASE SHEAR OVERTURNING MOMENT

<sup>4470.115</sup> KN 236482.2 KN-M

APPENDIX C.2

COLLAPSE OUTPUT SUMMARY

APPENDIX C.2

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A) PLATFORM A

	LOAD LOAD NO. MAXIMUM DEFLECTION								UMMAR		UTION DA	ТА	REACTION SUMMATION		
INCR	LUAD CASE	FACTOR		DEFL.	JOINT	DOF	ROT.	JOINT	DOF	MAX.	UTION DA	DOF	FX	FY FY	FZ
INCR	CASE	FACTOR	LUUPa	CM	JOINT		KOI.		DOF	DIGITS	20141	DUr	KN .	KN	KN
	CLPS	0.5	39	-10.734	993V	DY	0.01105	9945	RY	4	993V	DZ	-1224.2	-6.29	7759.05
	CLPS	1	39	-20.985		DY	0.02221	9945			993V	DZ	-2443.94	-13.15	15519.35
3	100	0.05	39			DY	0.02147	9945			993V	DZ	-2560.96	-13.73	15526.00
4	100	0.1	39			OY	0.021		RZ		993V	DZ	-2677.97	-14,31	15532.76
	100	0.15	19			DY	-0.0218		RŽ		993V	DZ	-2794.99	-14.89	
6	100	0.2	11			DY	-0.0226		RZ		993V	DZ	-2912.01	-15.47	15546.16
7	100	0.25	5			DY	-0.0235	9940			993V	DZ	-3029.02	-16.04	
8	100	0.3	5	24.52		DY	-0.0243	9940			993V	DZ	-3146.04	-16.62	15559.56
	100	0.35	5	-25,1		DY	-0.0251	9940			993V	DZ	-3263.06	-17.19	
10	100	0.4	5			DY	-0.0259	9940			993V	DZ	-3380.07	-17.76	15572.95
11	100	0.45	7	-26.253		DY	-0.0267	9940			993V	DZ	-3497.09	-18.33	15579.65
12	100	0,5	9	-26.825		DY	-0.0275	9940			993V	DZ.	-3614.11	-18.9	15588.34
13	100	0.55	9			DY	-0.0283	9940			993V	DZ	-3731.12	-19.47	15593.04
14	100	0.6	9			DY	-0.0291	9940			993V	DZ	-3848.14	-20.04	
15	100	0.65	9	-28,529	993V	DY	-0.0299	9940	RZ	4	993V	DZ	-3965.16	-20.6	15606.42
16	100	0.7	9			DY	-0.0306	9940			993V	DZ	-4082.18	-21.17	15813.11
17	100	0.75	11	-29.652	993V	DY	-0.0314	9940	RZ	4	993V	DZ	-4199.19	-21.73	15619.79
18	100	0.8	13	-30.21		DY	-0.0322	9940	RZ	4	993V	DZ	-4316.21	-22.29	15626.48
19	100	0.85	15	-30.765	993V	DY	-0.033	9940	RZ	4	993V	DZ	-4433,23	-22.85	15633.16
20	100	0.9	23	-31,318	993V	DY	-0,0337	9940	RZ	4	993V	DZ	-4550.25	-23.41	15639.85
21	100	0, <del>9</del> 5	39	-31.868	993V	DY	-0.0345	9940	RZ	4	993V	DZ	-4667.27	-23.96	15646.53
22	100	1	39	32.511	993U	DX	-0.0353	9940	RZ		993V	DZ	-4784.28	-24.52	15653.21
23	100	1.05	3	33,244	993U	DX	-0.036	9940	RZ	4	993V	DZ	-4901.3	-25.07	15659.89
24	100	1.1	39	33.978	993U	DX	-0.0368	1993E	RZ	4	993V	DZ	-5018.32	-25.62	15666.57
25	100	1.15	3	34.709	993U	DX	-0.0375	993E	RZ	4	993V	DZ	-5135.34	-26.18	15673.24
26	100	1.2	39	35.439	993U	DX	-0.0383	993E	RZ	4	993V	DZ	-5252.36	-28.73	15679.92
27	100	1.25	39	36,204	993O	DX	-0.039	993E	RZ	4	993V	DZ	-5369.38	-27.27	15686.59
28	100	1.3	39	37.038	993O	DX	-0.0398	993E	RZ	4	993V	DZ	-5486.4	-27.82	15693.27
29	100	1.35	39	37.871		DX	-0.0405		RZ		993V	DZ	-5603.42	-28.37	15699.94
30	100	1.4	39	38.704		DX	-0.0412		RZ		993V	DZ	-5720.43	-28.91	
31{	100	1.45	39	39.535	9930	DX	0.042	993E	RZ	4	993V	DZ	-5837.45	-29.46	15713.28
32	100	1.5	39	40.365		DX	-0.0427		RZ		993V	DZ	-5954.47	-30	
33	100	1.55	39	41.394	9936		-0.0434		RZ		993V	DZ	-6071.49	-30.54	15726.61
34	100	1.6	39	42.493	9936		-0.0441	993E	RZ		993V	DZ	-6188.51	-31.08	
35	100	1.65	39	43.591	9936		-0,0448	993E	RZ	. 4	993V	DZ	-6305.53	-31.62	15739.94
36	100	1.7	39	44.689	9936	DX	-0.0455	993E	RZ	4	993V	DZ	-6422.55	-32.15	15746.6
37	100	1.75	39	45.785	9936		-0.0463		RZ		993V	DZ	-6539.57	-32.69	15753.26
38	100	1.8	39	46.881	9936		-0.047		RZ		993V	DZ.	-6656.59	-33.22	15759.92
39	100	1.85	39	47.975	9936		-0.0477		RZ		993V	DZ	-6773.61	-33.76	
40	100	1.9	39	49.069	9936		-0.0484		RZ		993V	DZ	-6890.62	-34.29	
41	100	1.95	39	50.162	9936		-0.0491		RZ		993V	DZ	-7007.64	-34.82	15779.89
42	100	2	39	51.254	9936		-0.0497		RZ		993V	DZ	-7124.66	-35.35	15786.55
43	100	2.05	39	52.345	9936		-0.0504		RZ		993V	DZ	-7241.68	-35.88	15793.2
44	100	2.1	39	53.435	9936		-0.0511		RZ		993V	DZ	-7358.7	-36.41	15799.85
45	100	2.15	39	54.524	9936		-0.0518		RZ		993V	DZ	-7475.72	-36.93	15806.5
46	100	2.2	39	55.814	9936		-0.0525		RZ		993V	DZ	7592.74	-37.46	
47	100	2.25	39	56.702	9936		-0.0532		RZ		993V	DZ	-7709.76	-37.98	15819.8
48	100	2.3	39	57.79	9936		-0.053B		RZ		993V	DZ	-7826.78	-38.5	15826.44
49	100	2.35	39	58.878	9936		-0.0545		RZ		993V	DZ	-7943.8	-39.02	15833.08
50	100	2.4	39	59.966	9936		-0.0552		RZ.		993V	DZ	-8060.81	-39.54	
51	100	2.45	39		9936		-0.0558		RZ			DZ	-8177.81		15846.38
52	100	2.5	39		9936		-0.0565		RZ			DZ	-8294.82		15853.03
53	100	2.55	39	63.283	9936		-0.0571		RZ		993V	DZ	-8411.83		15859.67
54	100	2.6	39		9936		-0.0578		RZ	the second s	993V	DZ	-8528.84		15866.32
55	100	2.65	39		9938		-0.0585		RZ		993V		-8645.84		15872.97
56	100	2.7	39	66.722	9936		-0.0591		RZ		993V	DZ	-8762.83		15879.62
57	100	2.75	39	67.93	9936		-0.0598		RZ		993V	DZ	-8879.8		15886.28
58	100	2.8	39	69.17	9936		-0.0604		RZ		993V	DZ	-8996.77		15892.95
59	100	2.85	39 39	70,443	9936 4153		-0.061		RZ			DZ	-9113.75		15899.62
			- 44	86,571	4153	UX	+0.06	993E	RZ	4	993V	DZ	-9230.65	-+0.341	15906.88
60	100	2.9							<u> </u>		0031	D7	0225 25	44.50	15000.07
	100 100 100	2.95	39	140.287 186.188	4053 4053	DX	-0.0648 0.21317		RŽ.		993V 993V	DZ DZ	-9325.25 -9415.82		15930.85 15956.64

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42.11 deg	1 deg	a
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	LOAD	LOAD	NO.	MAXIM	UM DEFL		APSE SO MAXII	NUM ROT			UTION DA	TA	REACT	ION SUMM	ATION
INCR	CASE	FACTOR	LOOPS	DEFL.	JOINT	DOF	ROT.	JOINT	DOF	MAX	JOINT	DOF	FX	FY	FZ
		TACTOR	20010	CM ·	UGH1;			30111	00,	DIGITS	001111		KN	ĸN	KN
	CLPS	0.5	39	20.126		DY	0.01658		RZ		993V	DZ	-1060.69	-911,18	7771,
	CLPS	1	39	39.745		DY	0.03295		RZ		993V	DZ	-2118.64	-1821.58	15544.
3	100	0.05	39		993V	DY	0.03464		RZ		993V	DZ	-2221.92	-1910.97	15552
4	100	0.1	39			DY	0.03633		RŻ		993V	DZ	2325.21	-2000.36	155
5	100	0.15	39	46.795		DY	0.03801		RZ		993V	DZ	-2428.48	-2089.75	
6	100	0.2	39			DY	0.03969		RZ		993V	DZ	-2531.75	-2179.14	
7	100	0.25	39			DY	0.04137		RŻ		993V	DZ	-2635.02	-2268.54	
8	100	0.3	39	53.801		DY	0.04304		RZ		993V	DZ	-2738.28	-2357.93	
9	100	0.35	39			DY	0.04471		RZ		993V	DZ	-2641.53	-2447.32	
10	100	0.4	39	58.448		DY	0.04637		RZ		993V	DZ	-2944.78	-2536.72	1560
11	100	0.45	39	60,764	993V	DY	0.04803	993E	RZ	4	993V	DZ	-3048.02	-2626.11	15615
12	_100	0.5	39	63.075	993V	DY	0.04968	993E	RZ	4	993V	DZ	-3151.26	-2715.51	15623
13	100	0.55	39	65.38	993V	DY	0.05133	993E	RZ	4	993V	DZ	3254.49	-2804,9	15631
14	100	0,6	39	67.681	993V	DY	0.05298	993E	RZ	4	993V	DZ	-3357.71	-2894,3	1563
15	100	0.65	39	69.976	993V	DY	0.05462	993E	RZ	4	993V	DZ	3460.93	-2983.7	15647
16	100	0,7	39	72.267	993V	DY	0.05625	993E	RZ	4	993V	DZ	-3564.14	-3073.09	
17	100	0.75	39			DY	0.05789		RZ		993V	DZ	-3667.35	-3162,49	
18	100	0.8	39			DY	0.05951		RZ		993V	DZ	-3770.55	-3251.88	
19	100	0.85	39	79,106	993V	DY	0.06114	993E	RZ		993V	DZ	-3873.75	-3341.28	1567
20	100	0.9	39			DY	0.06275		RZ		993V	DZ	-3976.94	-3430.68	
21	100	0.95	39	83.639	993V	DY	0.06437	993E	RZ	4	993V	DZ.	-4080.12	-3520.08	
22	100	1	39	85.897	993V	DY	0.06598	993E	RZ	4	993V	DZ	-4183.3	-3609.47	15702
23	100	1.05	39	88.15	993V	DY	0.06758	993E	RZ	4	993V	DZ	-4286.47	-3698.87	
24	100	1,1	39	90.397		DY	0.06918	993E	RZ	4	993V	DZ	-4389.64	-3788.27	
25	100	1.15	39			ĎΥ	0.07078		RZ		993V	DZ.	-4492.8	-3877.66	
26	100	1.2	39	94.875	993V	DY	0.07237		RZ		993V	DZ	-4595.96	-3967.06	
27	100	1.25	39	97.105		DY	0.07396		RZ		993V	DZ	-4699.11	-4056.46	
28	100	1.3	39	99.33		DY	0.07554		RZ		993V	DZ	-4802.25	-4145.85	
29	100	1.35	39	101.55		DY	0.07712		RZ		993V	DZ	-4905.39	-4235.25	15
30	100	1.4	39			DY	0.07869		RZ		993V	DZ	-5008.52	-4324.64	1576
31	100	1.45	39	105.974		DY	0.08026		RZ		993V	DZ	5111.65	-4414.04	
32	100	1.5	39	108.18		DY	0.08182		RZ		993V	DZ	5214.77	-4503.43	1578
33	100	1.55	39	110.386		DY	0.08338		RZ		993V	DZ	-5317,88	-4592.82	1578
34	100	1.6	39	112.542	993V	DY	0.08493		RZ		993V	DZ	-5421.01	-4682.23	
35	100	1.65		115.064		DY	0.08658		RZ		993V	DZ	-5523.87	-4771,55	15805
36	100	1.7	39	116.864	993V	DY	0.08808		RZ		993V	DZ	-5827.17	-4861.14	
37	100	1.75		118.974		DY	0.08953		RZ		993V	DZ	-5730.34	-4950.43	
38	100	1.8		120.338		DY	0.09127		RZ	the second se	993V	DZ	-5833.59		
39	100	1.85		124,146		DY	0.09254		RZ		993V	DZ	-5935.94		
40	100	1.9		127,483		DY	0.09396		RZ		993V	DZ	-6038.18	-5216.68	
41	100	1.95	39			DY	0.09542		RZ		993V	DŽ	-6139.42	-5304.98	
42	100	2	39			DY	0.1049	1442			993V	DZ	6228.97	-5383.56	
43	100	2.05		123.437		DY	0.0988				993V	DZ	-6339,89	-5487.48	
44	100	2.1		154.551		DY	0.12721	1442			993V	DZ	-6426.2	-5553,35	
45	100	2.15		153.422		DY	0.17242	2648			993V	DZ	-6535.43		
46	100	2,2		132,686		DY	0.29656	1165			993V	DZ	-6646.9	-5752.63	
47	100	2.25		154,666		DY	0.17627	2648			993V	DZ	-6738.01	-5824,74	
48	100	2.3		149,244		DY	-0.1397	2647			993V	DZ	6843.71	-5920.58	
49	100	2.35		159.476		DY	0.79898				993V	DZ	-6939,54	-6000.75	

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						COLL	APSE SO	LUTION S	SUMMAR'	Y					j
	LOAD	LOAD	NO.	MAXIM	UM DEFL	ECTION	MAXIN	IUM ROT	ATION	SOL	UTION DA	TA	REACT	ION SUMM	IATION
INCR	CASE	FACTOR	LOOPS	DEFL.	JOINT	DOF	ROT.	JOINT	DOF	MAX.	JOINT	DOF	FX	F۲	FZ
1				CM						DIGITS	Í		KN	KN	KN
				_											
1	CLPS	0.5	37	102.127	993V	DY	0.09548	993E	RZ	4	993V	DZ	2.99	-3006.31	7916.68
2	CLPS	1	39	193.009	993V	DY	0.18371	993E	RZ	4	993V	DZ	15.72	-6003.55	15835.56
3	100	0.05	39	201.613	993V	DY	0.19177		RŻ		993V	DZ	17.41	-6300.27	15858.15
4	100	0.1	39	210.021		DY	0 19964	993E	RZ	4	993V	DZ	19.16	-6596.96	15880.74
5	100	0.15	39	218.231	993V	DY	0.20731	993E	RZ	4	993V	DZ	20.99	-6893.63	15903.34
6	100	0.2	39	226.243		DY	0.21478		RZ	4	993V	DZ	22.88	-7190.26	15925.96
7	100	0.25	39	234.058		DY	0.22205		RZ	4	993V	DZ	24.83	-7486.87	15948.57
8	100	0.3	39	241,677	993V	DY	0.22912		RZ	4	993V	DZ	26.83	-7783.44	15971.2
9	100	0.35	39	249,105		DY	0.23598		RZ.	the second s	993V	DZ	28.89	-8079.99	15993.84
10	100	0.4	39	256.346	993V	DY	0.24265		RZ	4	993V	DZ	30.99	-8376.52	16016.48
11	100	0.45	39	263,402	993V	DY	0.24912		RZ	4	993V	DZ	33.14	-8673.01	16039.14
12	100	0.5	39	270.283	993V	DY	0.2554	993E	RZ	4	993V	DZ	35.34	-8969.48	16061.81
13	100	0.55	39	276.994	993V	DY	0.26149	993E	RZ	4	993V	DZ	37.58	-9265.92	16084.48
14	100	0.6	39	283.552	993V	DY	0.2674	993E	RZ	4	993V	DZ	39.85	-9562.33	16107.18
15	100	0.65	39	289.977	993V	DY	0.27313	993E	RZ	4	993V	DZ	42,16	-9858.68	16129.9
16	100	0.7	39	296.38	993V	DY	0.27864	993E	RZ	4	993V	DZ	44.46	-10155	16152.65
17	100	0.75	39	302.577	993V	DY	0.28409	993E	RZ	4	993V	DZ	46.92	10451.2	16175.45
18	100	0.8	39	307.154	993V	DY	0.28914	993E	RZ	4	993V	DZ	49.05	-10748.4	16197.6
19	100	0.85	39	314.045		DY	0.29465		RZ	4	993V	DZ	52.48	-11043.9	16220.83
20	100	0.9	39	317.898		DY	0.29944		RZ		993V	DZ	54.29	-11340.9	16243
21	100	0,95	39	322.846		DY	0.30405		RZ		993V	DZ	56.69	-11638.5	16265.01
22	100	1	39	328.122		DY	0.30889		RZ		993V	DZ	59.57	-11933.9	16288.23
23	100	1.05	39	339.518		DY	0.31348		RZ		993V	DZ	62.02	-12225.9	16314.14
24	100	1.1	39	345.223		DY	0.31834		RZ		993V	DZ	63.46	-12521.5	16337.25
25	100	1,15	39	572.434	993V	DY	0.41945	2707		4	993V	DZ	53.23	-12686.7	16458.95
26	100	1.2	6	603.765	993V	DY	0.40653	2707	RY	4	993V	DZ	52.07	-12974 7	16489.15

## 137.89 deg

						COLL	APSE SO	LUTION S	UMMAR	Y					
	LOAD	LOAD	NO.	MAXIM	UM DEFL	ECTION	MAXIN	<b>IUM ROT</b>	ATION	SOL	UTION DA	TA	REACT	ION SUMM	ATION
INCR	CASE	FACTOR	LOOPS	DEFL. CM	JOINT	DOF	ROT.	JOINT	DOF	MAX. DIGITS	JOINT	DOF	FX KN	FY KN	FZ KN
1	CLPS	0.5	27	41.98	9947	DY	0.042	993E	RZ	4	993V	DZ	1328.88	-1140.25	7783.76
2		1	39	84.916	9947	DY	0.08565		RZ	4		DZ	2657.38	-2275.76	15567.83
3	100	0.05	39	89.153	9947	DY	0.09011	993É	RZ	4	993V	DZ	2787.57	-2387.98	15576.83
4	100	0.1	39	93,383	9947	DY	0.09458	993E	RZ.	4	993V	DZ	2917.79	-2500.2	15586.04
5	100	0.15	39	97.605	9947	DY	0.09904	993E	RZ	4	993V	DZ	3048.02	-2612.41	15595.2
6	100	0.2	39	101.816	9947	DY	0.10351	993E	RZ	4	993V	DŻ	3178.27	-2724.61	15604.4
7	100	0.25	39	106.015	9947	DY	0.10797	993E	RZ	4	993V	DZ	3308.54	-2836.81	15613.3
8	100	0.3	39	110.202	9947	DY	0.11243	993E	RZ	4	993V	DZ	3438.82	-2948.99	15622.93
9	100	0.35	39	114.373	9947	DY	0.11687	993E	RZ	4	993V	DZ	3569.11	-3061.17	15632.17
10	100	0,4	39	118.527	9947	DY	0.12131	993E	RŻ	4	993V	DZ	3699.42	-3173.33	15641.42
11	100	0.45	39	122.663	9947	DY	0.12574	993E	RZ	4	993V	DZ	3829.75	-3285.49	15650.67
12	100	0.5	39	126.779	9947	DY	0.13016	993E	RZ.	4	993V	DZ	3960.08	-3397.64	15659.93
13	100	0.55	39	130.873	9947	DY	0.13456	993E	RŽ		993V	DZ	4090.43	-3509.78	15669.19
14	100	0.6	39	134.943	9947	DY	0.13895	993E	RZ	4	993V	DZ	4220.8	-3621.91	15678.46
15	100	0.65	39	138.988	9947	DY	0.14332	993E	RZ		993V	DZ	4351.18	-3734.03	15687.74
16	100	0.7	39	143.106	993V	DY	0.14767	993E	RZ	4	993V	DZ	4481.57	-3846.14	15697.03
17	100	0.75	39	147.319	993V	DY	0.15199	993E	RZ	4	993V	DZ	4611.97	-3958.24	15706.32
18	100	0.8	39			DY	0.15629	993E	ŔΖ	4	993V	DZ	4742.38	-4070.33	15715.61
19	100	0.85	39			DY	0.16057	993E	RZ	4	993V	DZ	4872.81	-4182.42	15724.9
20	100	0.9	39		993V	DY	0.16481		RZ		993V	DZ	5003.25	-4294.49	15734.2
21	100	0.95	39	163.847	993V	DY	0.16903	993E	RZ	4	993V	DZ	5133.7	-4406.56	15743.54
22	100	1	39	167.89		DY	0.17321		RZ		993V	DZ	5264.16	-4518.62	
23	100	1.05	39			DY	0.17736		RZ		993V	DZ	5394.63	-4630.67	15762.19
24	100	1.1	39	175.824		DY	0.18149		RZ.		993V	DZ	5525.14	-4742.72	15771.5
25	100	1.15	39	179,967		DY	0.18542		RZ		993V	DZ	5655,39	-4854.74	15780.94
26	100	1.2	39	183.429		DY	0.18972		RZ		993V	DZ	5786.25	-4966,77	15790.14
27	100	1.25	39	186.679		DY	0.19341		RZ		993V	DZ	5916.82	-5079.52	15799.22
28	100	1.3	39	191.255		DY	0.19763		RZ		993V	DZ	6047.17	-5190.72	15808.93
29	100	1.35	39	196,268		DY	0.2016		RZ		993V	DZ	6176.8	-5301.64	15819.06
30	100	1.4	39	197.807		DY	0.20597		RZ		993V	DZ	6308.42	-5414.4	15827.68
31	100	1.45	39			DY	0.209		RZ		993V	DZ	6438.62	-5527.45	15836.81
32	100	1.5	39	204.416		DY	0.21306	993E	RZ	· · · · · · · · · · · · · · · · · · ·	993V	DZ	6569.61	-5639.43	15845.99
33	100	1.55	39			DY	0.21687	993E	RZ		993V	DZ	6700.13	-5751.24	15855.46
34	100	1.6	39	208.99		DY	0.2206		RZ		993V	DZ	6830.66	-5864.41	15864.37
35	100	1.65	39			DY	0.22499		RZ		993V	DZ	6958.94	-5975.09	15875.82
36	100	1.7	39	225.071		DY	-0.3424	1159			993V	DZ	7081.31	-6078.19	15892.48
37	100	1.75	34	254.818	893V	DY	-2.9293	1459	ΗY	4	993V	DZ	7206.15	-6174.33	15910.08

#### 222.11 deg

						COLL	APSE SO	LUTION S	SUMMAR'						
	LOAD	LOAD	NO.	MAXIM	UM DEFL	ECTION	MAXIM	<b>JUM ROT</b>	ATION		<b>JTION DA</b>			ION SUMM	
INCR	CASE	FACTOR	LOOPS	DEFL.	JOINT	DOF	ROT.	JÓINT	DOF	MAX.	JOINT	DOF	FX KN	FY	FZ KN
				СМ						DIGITS			KN	KN	NN
1	CLPS	0.5	12	-44.931	993V	DY	-0.0345	9937	RZ			DZ	1540.97	1300.83	7711.06
2	CLPS	1	26	-90.593	993V	DY	-0.07		RZ	4		DZ	3082.81	2595.27	15419.49
3	100	0.05	8	-94.783	993V	DY	-0.0735	993Z	RZ	4	993V	DŽ	3234.29	2723.61	15421.17
4	100	0,1	8	-98.978	9 <del>9</del> 3V	DY	-0.0771	993Z	RZ	4	993V	DZ	3385.77	2851.94	15422.85
5	100	0.15	8	-103.18	993V	DY	-0.0807	993Z	RZ	4	993V	DZ	3537.27	2980.27	15424.53
6	100	0,2	8	107.38	993V	DY	-0.0843	993Z	RZ	4	993V	DZ	3688.78	3108,58	15426.2
7	100	0.25	8	-111.59	993V	DY	-0.0879	993Z	RZ	4	993V	DZ	3840.31	3236.88	15427,87
8	100	0.3	8	-115.8	993V	DY	-0.0915		RZ	4	993V	DZ	3991.85	3365.18	15429.54
9	100	0.35	8	-120.01	993V	DY	-0.0951		RZ	4	993V	DΖ	4143.4	3493.46	15431.2
10	100	0.4	8	-124.21		DY	-0.0987		RZ		993V	DZ	4294.98	3621.74	15432.86
11	100	0.45	8	-128.42		DY	-0.1024		ŔŻ		993V	DZ	4446.54	3750	15434.52
12	100	0.5	8	132.62		DY	-0.106		RZ			DZ	4598.12	3878.26	
13	100	0.55	8	136.82	993V	DY	-0,1096		ŔŻ		993V	DZ	4749,72	4006.51	15437.82
14	100	0.6	8	-141.01	993V	DY	-0.1133		RZ	4	993V	DZ	4901.33	4134.74	15439.47
15	100	0.65	10	-145.2	993V	DY	-0,1169	993E	RZ	4	993V	DZ	5052.96	4262.97	15441.12
16	100	0.7	10	-149.38	993V	DY	-0.1206	993E	RZ	4	993V	DZ	5204.59	4391.19	15442.76
17	100	0.75	10	-153.55	993V	DY	-0.1242	993E	RZ	4	993V	DZ	5356.23	4519.39	15444.4
18	100	0.8	12	-157.7	993V	DY	-0.1278	993E	RZ	4	993V	DZ	5507.89	4647.59	15446.03
19	100	0.85	12	-161.85	993V	DY	-0.1315	993E	RZ	4	993V	DZ	5659.55	4775,78	15447.6
20	100	0.9	39	166.03	993V	DY	-0.1351	993E	RZ	4	993V	DZ	5811.2	4903.92	15449.3
21	100	0.95	39	-170.19	993V	DY	-0.1387	993E	RZ	4	993V	DZ	5962.86	5032.01	15450.98
22	100	1	39	• 173.33	993V	DY	-0.1422	993E	RZ	4	993V	DZ	6114.73	5160.92	15452.22
23	100	1.05	39	-177.17	993V	ĎΥ	-0.1461	993E	RZ	4	993V	DZ	6267.01	5288.8	15453.78
24	100	1.1	39	-183.24	993V	DY	-0.1497	993E	ŔŻ	4	993V	DZ	6417.18	5415.34	15456.47
25	100	1.15	39	-187.79	993V	DY	-0.1533	993E	RŽ	4	993V	DZ	6568.82	5543.23	15458.3
26	100	1.2	39	-189.45	993V	DY	-0.1571	993E	RZ	4	993V	DZ.	6720.9	5672.26	15459.22
27	100	1.25	39	-194.63	993V	DY	-0.1604	993E	RŽ	4	993V	DZ	6870.49	5799.02	15461.8
28	100	1.3	39	-201.72	993V	DY	-0.1654	993E	RZ.	4	993V	DZ	7017,34	5921.11	15467.0
29	100	1.35	39	-208.27	993V	DY	-0.1689		RZ	4	993V	DZ	7166,17	6046.81	15470.83
30	100	1.4	39	-249.52	993V	DY	-0.174	993E	ŔŻ	4	993V	DZ	7296.45	6143.38	15495.2
31	100	1.45	39	-234.45	993V	DY	-0.1799	993E	RZ	4	993V	DZ	7450.36	6274.65	15492.99
32	100	1.5	39	-295.82	993V	DY	-0.2526	1451	RY	4	993V	DZ	7574.29	6345.92	15531.2
33	100	1.55	6	-296.42	993V	DY	-0.4309	1351	RX	4	993V	DZ	7726.05	6471.3	15534.3

## 270 deg

•	LOAD	LOAD	NO.	MAYIM	UM DEFL			IUM ROT	SUMMAR		UTION DA	TA	REACT	ION SUMM	ATION
INCR	CASE	FACTOR	LOOPS	DEFL.		DOF	ROT.	JOINT	DOF	MAX.	JOINT	DOF	FX	FY	FZ
	UNSE	FACTOR		CM	301141	Dor		30111	50,	DIGITS	30/141		ĸŇ	ĸN	KN
1	CLPS	0.5	39	-46.783	993V	DY	0.0389	993E	RZ	4	993V	DZ	1.78	1051.04	7696.0
2	CLPS	1	24	-92.937	993V	DY	-0.0772	993E	RZ	4	993V	DZ	7.04	2098.32	15392.
3	100	0.05	4	-97.088	993V	DY	-0.081	993Ë	RZ	4	993V	DZ	7.42	2200.21	15392.
4	100	0.1	6	-101.23	993V	DY	-0.0847	993E	RZ	4	993V	DZ	7.82	2302.11	15393.
5	100	0.15	6	-105.37	993V	DY	-0.0885	993E	RZ	4	993V	DZ	8.23	2404	1539
6	100	0.2	6	-109.49	993V	DY	-0.0922	993E	RZ	4	993V	DZ	8.65	2505.89	15393.
7	100	0.25	6	-113.6	993V	DY	-0.0959	993E	RZ	4	993V	DZ	9.09	2607.78	15394.
8	100	0.3	6	-117.7	993V	DY	-0.0996		RZ		993V	DZ	9,54	2709.67	15394
. 9	100	0.35	6	121.79		DY	-0.1033		RZ		993V	DZ	10	2811.55	
10	100	0.4	8	-125.86		DY	-0.107		RZ		993V	DZ	10.48	2913.43	
11	100	0.45	6	-129.92	993V	DY	-0.1107	993E	RZ		993V	DZ	10.96	3015.3	1539
12	100	0.5	8	-133.96	993V	DY	-0.1144	993E	RZ	4	993V	DZ	11.46	3117.17	15395
13	100	0.55	8	-137.99	993V	DY	-0.118	993E	RZ.	4	993V	DZ	11.97	3219.04	15396.
14	100	0.6	8	-142	993V	DY	-0.1217	993E	RZ	4	993V	DZ	12.49	3320.9	15396.
15	100	0.65	8	-145.99	993V	DY	-0.1253	993E	RZ	4	993V	DZ	13.02	3422.76	15396.
16	100	0.7	8	-149.96	993V	DY	-0.1289	993E	RZ	4	993V	DZ	13.57	3524.62	15397
17	100	0.75	10	-153.91	993V	DY	-0.1325	993E	RZ	4	993V	DZ	14.12	3626.47	15397.
18	100	0.8	10	157.84	993V	DY	-0.1361	993E	RZ	4	993V	DZ	14.69	3728.32	15397.
19	100	0,85	10	-161.75	993V	DY	-0,1396	993E	RZ	4	993V	DZ	15.26	3830.17	15397
20	100	0.9	10	-165.63		DY	-0.1432		RŽ		993V	DZ	15.85	3932.01	15398
21	100	0.95	12	-169.49		DY	-0.1467		RZ		993V	DZ	16.45	4033,85	1539
22	100	1	12	~173.33	993V	DY	-0.1502		RZ		993V	DZ	17.06	4135.69	
23	100	1.05	14	-177.14	993V	DY	-0.1536	993E	RŻ		993V	DZ	17.67	4237.52	15399
24	100	1.1	14	-180.92	993V	DY	-0.1571		RZ		993V	DZ	18.3	4339.35	15399
25	100	1.15	16	-184.68	993V	DY	-0.1605		RZ		993V	DZ	18.93	4441.17	15399.
26	100	1.2	18	-186.4	993V	DY	-0.1639	993E	RZ	4	993V	DZ	19.58	4542.99	15400.
27	100	1.25	22	-192.1	993V	DY	-0.1673	993E	RZ	4	993V	DZ	20.23	4644.81	15400.
28	100	1.3	26	-195.77	993V	DY	-0.1706		RZ	4	993V	DZ	20.89	4746.63	15400
29	100	1.35	30	-199.42	993V	DY	-0.1739	993E	RZ	4	993V	DZ	21.56	4848.44	15401
30	100	1.4	39	-203.03	993V	DY	-0.1772	993E	RZ	4	993V	DZ	22.23	4950.24	1540
31	100	1,45	39	-206.61	993V	DY	-0.1804	993E	RZ	4	993V	DZ	22.92	5052.04	15401.
32	100	1.5	39	-210.16	993V	DY	-0.1837	993E	RZ	4	993V	DZ	23.61	5153.84	15402.
33	100	1.55	39	-213.67	993V	DY	-0.1869	993E	RZ	4	993V	DZ	24.31	5255.64	15402
34	100	1.6	39	-217.16	993V	DY	-0,19	993E	RZ	4	993V	DZ	25.01	5357.43	15402.
35	100		39	-220.61	993V	DY	-0.1931	993E	RŻ	4	993V	DZ	25.72	5459.22	15402.

							APSE SO					<b>T</b> 4			
IN CO.	LOAD	LOAD	NO.		UM DEFL			UM ROT			UTION DA			TON SUMM	
INCR	CASE	FACTOR	LOOPS	DEFL. CM	JOINT	DOF	ROT.	JOINT	DOF	MAX. DIGITS	JOINT	DOF	FX KN	FY KN	FZ KN
36	100	1.7	39	-224.04	993V	DY	-0.1962	993E	RZ	4	993V	DZ	26.44	5561.01	15403,24
37	100	1.75	39	-227.42		DY	-0.1993		RZ	4		DZ	27.17	5662.79	
38	100	1.8	39	-230.78	993V	DY	-0.2023	993E	RZ	4	993V	DZ	27.9	5764.57	15403.85
39	100	1.85	39	-234.11	993V	DY	-0.2053	993E	RZ	4	993V	ΟZ	28.63	5866.34	15404.15
40	100	1,9	39	-237.4		DY	-0.2083		RZ		993V	DZ.	29.37	5968.12	15404.46
41	100	1.95	39	-240.66		DY	-0.2112		RZ		993V	DZ	30.12	6069.89	
42	100	2	39			DY	-0.2141		RZ		993V	DZ	30.87	6171.65	
43	100	2.05 2.1	39 39	-247.08		DY DY	-0.217		RZ		993V 993V	DZ DZ	31.63 32.39	6273.42 6375.18	
45	100	2.15	39	-253.37		DY	-0 2226		RZ		993V	DZ	33.15	6476.93	
46	100	2.2	39	-256.46		DY	-0.2253		RZ		993V	DZ	33.92	6578.69	
47	100	2.25	39	-259.53		DY	-0.2281		RZ		993V	DZ	34.7	6680.44	
48	100	2.3	39	-262.56	993V	DY	-0.2308	993E	RŽ	4	993V	DZ	35.47	6782.19	15406.89
49	100	2,35	39	-265.57		DY	-0.2334		RZ		993V	DZ	36.25	6883.93	15407.2
50	100	2.4	39	-268.54		DY	-0.236		RZ		993V	DZ	37.04	6985.68	15407.5
51	100	2.45	39	-271.48		DY	-0.2386		RZ		993V	DZ	37.83	7087.42	15407.8
52	100	2.5	39	-274.39		DY	-0.2412		RZ		1993V	DZ	38.62	7189.15	
53 54	100 100	2.55	<u>39</u> 39	-277.27		DY DY	-0.2437		RZ RZ		993V	DZ DZ	39.41 40.21	7290.89	
55	100	2.6 2.65	39	-280.13		DY	-0.2462		RZ		993V 993V	DZ DZ	40.21	7392.62	
56	100	2.05	39			DY	-0.2511		RZ.		993V ·	DZ	41.81	7596.08	
57	100	2.75	39	-288.51		DY	-0.2536		RZ		993V	DZ	42.62		15409.64
58	100	2.8	39			DY	-0.256		RZ		993V	DZ	43.42	7799.53	
59	100	2.85	39	-293.96	993V	DY	-0.2583	993X	RZ	4	993V	ĎΖ	44,23	7901.25	15410.25
60	100	2.9	39	-296.65		DY	-0.2607		RZ			DZ	45.05		15410.56
61	100	2.95	39	-299.3		DY	-0.263		RZ			DZ	45.86	8104.68	
62	100	3	39	-301.94		DY	-0.2652		RZ		993V	DZ	46.68	8206.39	
63 64	100 100	3.05	39 39	-304.54		DY DY	-0.2675		RZ RZ		993V	DZ DZ	47.49 48.31	8308.1	
65	100	3.1 3.15	39	-307.13 -309.68		DY	-0.2697		RZ		993V 993V	DZ	49.14	8409.81 8511.52	15411.79
66	100	3.2	39	-312.22		DY	-0.2741		RZ		993V	DZ	49.96	8613.22	
67	100	3.25	39	-314.73		DY	-0.2762		RZ		993V	DZ	50.79	8714.92	
68	100	3.3	39	-317.22		DY	-0.2783		RZ			DZ	51,61	8816.62	
69	100	3,35	39	-319.68		DY	-0.2804		RZ		993V	DZ	52.44	8918.32	15413.34
70	100	3.4	39	-322.13		DY	-0.2825		RZ			DZ	53.27	9020.01	
71	100	3.45	39	-324.56		DY	-0.2846		RZ		993V	DZ	54.1		15413.97
72	100	3.5	39			DY	-0.2866		RZ		993V	DZ	54.94	9223.38	15414.29
73	100 100	3.55 3.6	39 39	-329.36 -331.74		DY DY	-0.2886		RZ RZ		993V 993V	DZ DŽ	55.77 56.61	9325.05	15414.81 15414.93
75	100	3.65	39	334.1		DY	-0.2905		RZ			DZ	57.44		15415.26
76	100	3.05	39	336.45		DY	-0.2944		RZ		993V	DZ	58.28		15415.59
77	100	3.75	39	-338.78		DY	-0.2963		RZ		993V	DZ	59.12		15415.93
78	100	3.8	39	-341.1		DY	-0.2982		RZ			DZ	59.96	9833.35	
79	100	3.85	39	-343.42		DY	-0.3001	993X	RZ	4	993V	DZ	60.75	9934.98	15416.62
80	100	3.9	39	-345.58		DY	-0.3019		RZ		the second se	DZ		10036.68	15416.9
81	100	3.95	39	-347.99		DY	-0.3037		RZ			0Z	62.41	10138.23	15417.31
82	100	4	39	-350.06		DY	-0.3056		RZ			DZ	63.59	10239.97	
83 84	100	4.05	39 39	-352.41		DY DY	-0.3072		RZ RZ		993V 993V	DZ DZ	63.89 65.5	10341.53	15417.98 15418.38
85	100	4.15	39	-354.73		DY	-0.3082		RZ		993V	DZ	66.14		15418.79
86	100	4.2	39	-359.68		DY	-0.3125		RZ			DZ		10645.83	
87	100	4.25		-363.27		DY	-0.3142		RZ			DZ	67,58	10746.11	15420.71
88	100	4.3	39	-363.43	993V	DY	-0.3157	993X	RZ	4	993V	DZ	67.3	10848.99	15420.14
89	100	4.35		-364.63		DY	-0.3176		RZ			DZ		10951.37	
90	100	4.4	39			DY	-0.3193		RZ			DZ	70.25		15420.87
91	100	4.45	39			DY	-0.3214		RZ			DZ		11155.73	
92	100	4.5	39	-370.37		DY	0.3224		RZ			DZ		11256.28	
93	100	4.55	39	-374.76		DY DY	-0.3245 -0.3256		RZ P7			DZ DZ		11357.45	
94 95	100	4.6	39 39	-374.17		DY	-0.3256		RZ RZ			DZ DZ		11459.68 11560.01	15421.59
96	100	4.05	39	-390.18		DY	-0.3285		RZ			DZ DZ			15430.24
97	100	4.75	39	-449.53		DY	-0.3311		RZ		993V	DZ		11698,89	
98	100	4.8	15			DY	-1.5748					DΖ		11690.11	

#### 317.89 deg

	LOAD CASE	LOAD	NO.	MAXIMI	IM DEEL		APSE SO				UTIONEDA		OF ACT	0	
1	CASE					ECTION		IUM ROT	ATION	301	<u>ÚTION DA</u>	IA	REAU	<u>10N SUMM</u>	ATION
		FACTOR	LOOPS	DEFL. CM	JOINT	DOF	ROT	JOINT	DOF	MAX. DIGITS	JOINT	DOF	FX KN	FY KN	FZ KN
	CLPS	0.5	30	-43.445	993V	DY	-0.0395	993E	RZ	4	993V	DZ	-858.49	733.29	7717.59
2	CLPS	1	22	85.162		DY	-0.0774		RZ		993V	DZ	1712.48	1465.21	15435.79
3	100	0.05	6	-88.837		DY	-0.081		RZ		993V	DZ	-1795.4	1536.92	15438.34
4	100	0.1	6	92,495		DY	-0.0846		RZ		993V	DZ	-1878.3	1608.64	15440.88
5	100	0.15	6	-96.136		DY	-0.0882		RZ		993V	DZ	-1961.2	1680.36	15443.43
6 7	100 100	0.2	6	-99.76 -103.37		DY DY	-0.0917		RZ RZ		993∨ 993∨	DZ DZ	-2044.08	<u>1752.08</u> 1823.8	
8	100	0.23	6	-106.95		DY	-0.0988		RŽ			DZ	-2209.82	1895.53	
9	100	0.35	8	-110.52		DY	-0.1023	993E	RZ		993V	DZ	-2292.67	1967.26	15453.61
10	100	0.4	8	-114.07		DY	-0,1058		RZ		993V	DZ	-2375.52	2038.99	15456.10
11	100	0.45	8	-117.59		DY	-0.1092		RZ		993V	DZ	-2458.35	2110.72	15458.71
12 13	100	0.5	8	121.1		DY DY	-0.1127		RZ RZ		993∨ 993V	DZ DZ	2541.17	2182.45 2254.19	15461.20 15463.81
14	100	0.55	10	124.55		DY	-0.1195		RZ		993V	DZ	2706.79	2325.93	15466.37
15	100	0.65	10	131,49		DY	-0.1229		RZ			DZ	-2789.59	2397.67	15468.92
16	100	0.7	10	-134.91		DY	-0.1262		RZ	· · · · · · · · · · · · · · · · · · ·	993V	DZ	-2672.37	2469.41	
17	100	0.75	12	-138.3	993V	DY	-0.1295	993E	RZ		993V	DZ	-2955.15	2541.15	15474.03
18	100	0.8	14	-141.67		DY	-0.1328		RZ		993V	DZ	-3037.92	2612.89	
19	100	0.85	14	-145.02		DY	-0.1361		RZ		993V	DZ	-3120.68		15479.15
20 21	100 100	0.9	16	-148.34		DY DY	0.1394		RZ RZ		993∨ 993∨	DZ DZ	-3203.43	2756.38	15481.71
21	100	0.95	22	151,64		DY	-0.1426		RZ		993V	DZ	3368.9	2899.87	15486.84
23	100	1.05	26	-158,16		DY	-0.149		RZ		993V	DZ	-3451.63	2971.62	15489.41
24	100	1.1	32	-161.38		DY	-0.1521		RZ		993V	DZ	-3534.34	3043.37	15491.97
25	100	1.15	39	-164.57		DY	-0.1552		RZ		993V	DZ	-3617.05	3115.12	
26	100	1.2	24	-167.74		DY	-0.1583		RZ		993V	DZ	-3699.75		15497.11
27	100	1.25	9	-170.88		DY	-0.1614		RZ.		993V	DZ DZ	-3782.45	3258.62	
28 29	100	1.3	39 39	-173.99 -177.08		DY DY	-0.1644		RZ RZ		993V 993V	DZ DZ	-3865.13 -3947.81	3330.38	15502.26
30	100	1.4	39	180.14		DY	0.1704		RZ		993V	DZ	-4030.49	3473.88	
31	100	1.45	39	-183,17		DY	0.1734		RZ		993V	DZ	-4113.15	3545.64	
32	100	1.5	39	-186.17		ĎΥ	-0.1763		RZ			DZ	-4195.81	3617.39	15512.57
33	100	1.55	39	:189.15		DY	-0.1792		RZ		993V	DZ	-4278.46	3689.15	15515.16
34	100	1.6	39	<u>-192.09</u>		DY	-0.1821		RZ		993V	DZ	-4361.11		15517.74
35 36	100 100	1.65	39	-195.01 -197.9		DY DY	-0.1849		RZ		993∨ 993∨	DZ DZ	-4443.75 -4526.38	3832.66	15520.33
37	100	1.75	39			DY	-0.1905		RZ		993V	DZ	-4609.01	3976.17	15525.5
38	100	1.8	39	-203.6		DY	-0.1932		RZ		993V	DZ	-4691.63	4047.93	15528.1
39	100	1.65	39	-206.41	993V	DΥ	-0.196		RZ		993V	DZ	-4774.25	4119.69	15530.69
40	100	1.9	39	-209.18		DY	-0.1987		RZ		993V	DZ	-4856.86	4191.45	
41	100	1.95	39	-211.93		DY	-0.2013		RZ		993V	DZ	-4939.47	4263.2	15535.88
42 43	100	2	39 39	-214.66		DY DY	-0.204		RZ RZ		993V 993V	DZ DZ	-5022.07 -5104.66	4334.96	15538.48
43	100	2.05	39	-217.35		DY	-0.2092		RZ		993V	DZ DZ	-5187.25	4478.48	15543.69
45	100	2.15	39	-222.66		DY	-0.2117	_	RZ		993V	DZ	-5269.84	4550.24	15546.3
46	100	2.2	39			DY	0.2143		RZ		993V	DZ	-5352.41		15548.91
47	100	2.25	39	-227.89	993V	DY	-0.2168	993X	RZ	4	993V	DZ	-5434.98	4693.73	15551.53
48	100	2.3	39	-230.47		DY	-0.2192		RZ		993V	DZ	-5517.53	4765.45	
49 50	100	2.35	39	-233.22		DY DY	-0.2217		RZ RZ		993V 993V	DZ DZ	-5599.93		15556.84
50 51	100	2.4	39	-234.62		DY DY	-0.2265		RZ		993V	DZ DZ	-5765.31		15561.78
52	100	2.5	39	-239.81		DY	-0.2288		RZ		993V	DZ	-5848.02		15564.45
53	100	2.55		-241.96	993V	DY	-0.2312	993X	RZ	4	993V	DZ	-5930.47	5124.93	15566.9
54	100	2.6	39	-246.22		DY	-0.2332		RZ		993V	DŽ	-6011.58		15570.76
55	100	2.65	39	-246.35		DY	0.2355		RZ		993V	DZ	-6093.65	5266.96	
56	100	2.7	39	-249.42		DY	-0.2365		RŻ		993∨ 993∨	DZ	-6170.87		15578.96
57 58	100	2.75	39	-264.49		DY DY	-0.2387		RZ RZ		993V 993V	DZ DZ	-6246.8 -6329.95	5396.39 5466.43	15587.5
59	100	2.8	<u>39</u> 39	-257.43		DY	-0.2398		RZ		993V	DZ	-6416.6		15588.01
60	100	2.9	39	-279.1		DY	0.2451		RZ			DZ	6489.3		15600.54
61	100	2.95	39	-270.2	993V	DY	-0.2475	993X	RZ	4	993V	DZ	-6570.05	5682.52	15603.02
62	100	3	39	-320.17	993V	DY	0.49126					DZ	-6623.5	5714.52	
63	100	3.05	39	311.19		DY	-0.2485		RŽ		993V	DZ	-6703	5790.47	
64	100	3,1	39	-365.62	993V	DY	1.17749	1257	RX	t <b>4</b>	993V	DZ	-6747.52	5811.37	15670.9

APPENDIX C.2 B) PLATFORM B

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							APSE SO								
LOAD	LOAD	NO.	NO.	the second s	IM DEFL			UM ROTA			UTION D	the second s		ON SUMM	
INCR	CASE	FACTOR		DEFL.	JOINT	DOF	ROT.	JOINT	DOF	MAX.	JOINT	DOF	FX	FY	FZ
CM	DIGITS	KN	KN	KN			i			DIGITS					
	CLPS	0.5	39	12.406		DX	-0.03152		RZ	3	1030		-875.58	-0.41	1183.
2	CLPS	1	39	24.849	18	DX	-0.06328	42	RZ	3	1030	DX	-1751.14	-0.81	2366
3	100	0.05	_39	26.113		DX	-0.06636		RZ	3	1030		-1837.11	-0.85	2367
4	100	0.1	39	27.303		DX	-0.0695		RZ	3	1030	=	-1923.1	-0.93	2368
5	100	0.15	39			DX	-0.07268		RZ	3	1030		-2009.08	-0.94	2369
6	100	0.2	39	29.803	51	DX	-0.07557		RZ	3	1030	DX	-2095.04	-1]	2370
7	100	0.25	39	31.048		DX	0.07869		RZ	3	1030	DX	-2181.02	-1.01	2371
8]	100	0.3	39	32.445	51	DX	-0.08195	42	RZ	3	1030	DX	-2266.95	-1	2372
9	100	0.35	39	33.621	51	DX	-0.08537	42	RZ	3	1030	DX	-2352,94	-1.06	2373
10	100	0.4	39	34.864	51	DX	-0.08819	42	RZ	3	1030	DX	-2438.91	-1.14	2374
11	100	0.45	39	36.084	51	DX I	-0.09118	42	RZ	3	1030	DX	-2524.91	-1.14	2375
12	100	0.5	39	37.29	51	DX	-0.09412	42	RZ	3	1030	DX	-2610.89	-1.19	2376
13	100	0.55	39	38.55	51	DX	-0.09774	42	RZ	3	1030	DX	-2696.86	-1.19	2377
14	100	0.6	39	39.786	51	DX	-0.10037		RZ	3	1030	DX	-2782.86	-1.29	2378
15	100	0.65	39	41.032	51	DX	-0.10384	42	RZ	3	1030	DX	-2868.81	-1.3	2379
16	100	0.7	39	42.243		DX	-0.10685		RZ	3	1030		-2954.8	-1.31	2380
17	100	0.75	39	43,542		DX	-0.11026		RZ	3	1030	DX	-3040.76	-1.34	2381
18	100	0.8	39	44.764	51	DX	-0.11366		RŽ	3	1030	DX	-3128.75	-1.46	2382
19	100	0.85	39	46.125		DX	-0.1162		RZ	4	7010	DY	-3212.72	-1.47	2383
20	100	0.9	39	47.27		DX	-0.11944	42	RZ	3	1030	DX	-3298.68	-1.43	2384
21	100	0.95	39	48.515	51	DX	-0.12242	42	RZ	3	1030	DX	-3384.68	-1.58	2385
22	100	1	39	49.841	51	DX	0.12621	42	RZ	3	1030	DX	3470.62	-1.62	2387
23	100	1.05	39	51.018	51	DX	-0.13002	42	RZ	3	1030	DX	3556.61	-1.65	2387
24	100	1.1	39	52.442	51	DX	-0.13384	42	RŻ	3	1030	DX	-3642.51	-1.45	2388
25	100	1.15	39	53.625	51	DX	-0.13743	42	RZ	3	1030	DX	-3728.51	-1.64	2390
26	100	1.2	39	61.125	12	DX	-0.14101		RZ	3	1030	DX	-3814.45	-1.53	2391
27	100	1.25	39	55.977	51	DX	-0.14303		RZ.	3	1030	DX	-3900.41	-1.53	2391
28	100	1.3	39	58.152		DX	-0.14734		RZ	3	1030		-3986.47	-1.84	2393
29	100	1.35	39	58.728		DX	-0.15125		RZ	3	1030		-4072.44	-1.69	2393
30	100	1.4	39	574.46	251	DX	0.29405	7020	RY	3	1030	DX .	4138.71	-26.4	2439

LOAD	LOAD	NO.	NÓ.	MAXIM	MDEFL		APSE SO	UM ROTA				ATA	REACT	ION SUMM	ATION
INCR CM	CASE DIGITS	FACTOR KN	LOOP\$	DEFL. KN	JOINT	DOF	ROT.	JOINT	DOF	MAX. DIGITS	JOINT	DOF	FX	FY	FZ
	CLPS	0.5	39	9,349	251		-0.04855		RZ	3			-730.93	-431.43	1182.
2	CLPS	1	39	17.566	251		-0.09756		RZ	3			-1461.92	-862.88	2365
3	100	0.05	39	18.847	251		-0.10256		RŽ	3			-1533.27	-904.99	2366
4	100	0.1	39	18.065	251		-0.10716		RŽ	3			-1604.65	-947.14	2367
5	100	0.15	39	18.433	251		-0.11224		RZ	3			-1675.98	-989.26	2368
6	100	0.2	39	20.593	251	DX	-0.11668		RZ	3			-1747.35	-1031.41	2369
7	100	0.25	39	20.376	251	DX	-0.12185	50	RZ	3	1030	DX	-1818.7	-1073.51	23
8	100	0.3	39	28,194	251	DX	-0.12661	50	RZ	3	1030	DX	1890.02	-1115.85	2371
9	100	0.35	39	22.143	251	DX	0.13145	50	RZ	3	1030	DX	-1961.4	-1157.75	2371
10]	100	0.4	39	24.814	251	DX	-0.13601	50	RZ	3	1030	DX	-2032.74	-1199.88	2373
11	100	0.45	39	26,223	251	DX	-0.14127	50	RZ	3	1030	DX	-2104.11	-1242	2374
12	100	0.5	39	23.142	18	DX	-0.14605		RZ	3	1030	DX	-2175.49	-1284.14	2374
13	100	0.55	39	23.797		DX	-0.15055		RŻ	3	1030	DX	-2248.84	-1326.32	2375
14	100	0.6	39			DX	-0.15669		RZ	3	1030		-2318.13	-1368.27	2376
15	100	0.65	39	25.449		DX	-0.18111		RZ	3			-2389.48	-1410.42	2377
16	100	0.7	39	32.235	12	DX	0.16574	50	RZ	3	1030	DX	-2460.86		2379
17	100	0.75	39	33.569	251	DX	-0.17054	50	RZ	3	1030	DX	-2532.29	-1494.77	2380
18	100	0.8	39	27.683	18	DX	-0.17438	50	RZ	3			-2603.78	-1537.06	2380
19	100	0.85	39	28.412	18	DX	-0.18012		RZ	3	1030	DX	-2675.05	-1579.15	2381
20	100	0.9	39	29.321	18	DX	-0.18444		RZ	3	1030	DX	-2746.52	-1621.27	2382
21	100	0.95	39	31.172	-251	DX	-0.19002	50	RZ	3	1030	DX	-2817.72	-1863.07	2383
22	100	1	39	30.629	28	DX	-0.19263	50	RZ	3	1030	DX	-2888.91	-1705.38	2384
23	100	1.05	39	31.635	18	DX	-0.19905		RZ	3		DX	-2960.6	-1747.63	2385
24	100	1.1	39	34.472		DX	-0.20573		RZ	3	1030	DX	-3031,91	-1789.32	2386.
25	100	1.15	39	35.752		DX	0.21294		RZ	3	1030			-1831.24	2387
26	100	1.2	39	34,822	251		-0.21532		RZ	3	1030	DX	-3174.47	-1873.68	2388
27	100	1,25	39	45.677		DX	-0.21977		RZ	3	1030	DX	-3245.94	-1915.84	2390
28	100	1.3	39	35.248		DX	-0.22203		RZ	3	1030		-3317.29	-1958.42	2390
29	100	1.35	39	38.583	9010		-0.23153		RZ	3	1030		-3389.05	-1999.61	2390
30	100	1.4	39		9010		-0.23805		RZ	3			-3460.19	-2041.05	2391
31	100	1.45	39	39,726	251	DX	-0.23845	50	RŽ	3	1030	DX	-3531.27	-2084.42	23
32	100	1.5	39		7505		-0.23942		RZ	3	1030	DX	-3601.85	-2126.63	2394
33	100	1.55	38	144,99	7110		-0.22153		RZ	3	1030	DX	-3728.11	-2205.03	238

	*****					COLL	APSE SO	LUTION S	UMMAF	<del>ΥΥ</del>					
LÓAD	LOAD	NO.	NO.	MAXIMU	IM DEFL	ECTION	MAXIM	UM ROT/	ATION	SOL	UTION D	ATA	REACT	10N SUMM	ATION
INCR	CASE	FACTOR	LOOPS	DEFL.	JOINT	DOF	ROT.	JOINT	DOF	MAX.	JOINT	DOF	FX	FY	FZ
CM	DIGITS	KN	KN	KN						DIGITS				' {	
	CLPS	0.5	39			DY	-0.06281		RZ	3	1030		-443.07	-783.46	1180.25
	CLPS	1	39	21.572		DY	-0.12537		RZ	3	1030		-886.18	-1566.91	2360.55
3		0.05	39			DY	-0.13137		RZ	3	1030		-929.42	-1643.4	2361.44
4	100	0.1	39			DY	-0.13725		RZ	3	1030		-972.68	-1719.93	2362.01
5	100	0.15	39	24.823		DY	-0.14403		RZ	3	1030		-1016.01	-1798.53	2382.78
6		0.2	39			DY	-0.14973		RZ	3			-1059.22	-1873.04	2363.52
7	100	0.25		26.875		DY	-0.15588		RZ	3	1030		-1102.52	-1949.63	2364.37
8	100	0.3	39	28,145		DY	-0.1627		RZ	3	1030		-1145.81	-2026.14	2365.3
9	100	0.35	39	29.067	and the second se	DY	-0.16842		ŔŻ	3			-1189.13	-2102.73	2365.64
10	100	0,4	39	30.094	28		-0.17448		RZ	3	1030		-1232.38	-2179.3	2366.63
11	100	0.45	39	31.152	28		-0.18031		RZ	3	1030		-1275.58	-2255.78	2367.19
12	100	0.5	39	32.312	51		-0.18758		RZ	3	1030		1318.96	-2332.27	2367.95
13	100	0.55	39	33.389	51		-0.19349		RZ	3	1030		-1362.33	-2408.95	2368.55
14	100	0.6	39	34.433		DY	-0.19896		RZ	3	1030		-1405.48	-2485.42	2369.57
15	100	0.65	39	35.455	28		-0.20499		RZ	3	1030		-1448.69	-2581.93	2370.19
18	100	0.7	39	36.842	51		-0.21234		RZ	3	1030		-1492.11	-2638.43	2370.96
17	100	0.75	39	38.075	51		-0.21982		RZ	3.	1030		-1535.4	-2714.77	2371.7
18	100	0.8	39	39.25	51		-0.22679		RZ	3	1030		-1579.01	-2791.54	2372
19	100	0.85	39	39.664	28		-0.22821		RZ	3			-1621.77	-2868.32	2373.71
20	100	0,9	39	41.71	51		-0.24134		RZ	3	1030		-1665.81	-2944.5	2373.33
21	100	0.95	39	42.752	51		-0.24757		RZ	3	1030		-1709.14	-3021.07	2374.07
22	100	1	39	47.289	46		-0.25861		RZ	3	1030		-1752.86	-3097.2	2374.51
23	100	1.05	39	44.152	51		-0.25717		RZ	3	1030		-1795.45	-3174.36	2375.99
24	100	1.1	39	46.299	44		-0.26501		RZ	3	1030		-1838.95	-3250.85	2376.83
25	100	1.15	39	172.366	40	the second se	-0.31399		RZ	3	1030		-1885.04	-3323.66	2382.74
26	100	1.2	16	1040.05	7660	DY	-1.35723	7660	RZ	3	1030	DX	-2127.37	-3411.08	2418.4

## 90 deg

						COLL	APSE SO	UTION S	UMMAF	RY					
LOAD	LOAD	NÖ.	NO.	MAXIMU	IM DEFL	ECTION	MAXIM	UM ROTA	TION	SO	UTION	ATA	REACT	TON SUMM	ATION
INCR	CASE	FACTOR	LOOPS	DEFL.	JOINT	DOF	ROT.	JOINT	DOF	MAX.	JOINT	DOF	FX	FY	FZ
CM	DIGITS	KN	KN	KN		1				DIGITS		1 1		1	
										1				1	
1	CLPS	0.5	39	12.689	44	DY	-0.04878	50	RZ	3	1030	DX	0.18	-870.23	1186.25
2	CLPS	1	39	22.555	44	DY	-0.09746	50	RZ	3	1030	DX	0.31	-1740.46	2372.68
3	100	0.05	39	25.999	44	DY	-0.10216	50	RZ	3	1030	DX	0.35	-1825.77	2374.12
4	100	0.1	39	24.093	44	DY	-0.10652		RZ	3	1030	DX	0.43	-1911	2375.41
5	100	0.15	39	27.167	44	DY	-0.11211	50	RZ	3	1030	DX	0.36	-1998.29	2376.79
6	100	0.2	39	26.291	- 44	DY	-0.11682	50	rz.	3	1030	DX	0.35	-2091.8	2378.11
7	100	0.25	39	27.818	44	DY	-0.1215	50	RZ	3	1030	DX	0.42	-2166.86	2379.5
8	100	0.3	39			DY	-0.1266		RZ	3	1030	DX	0.41	-2252.16	2380,98
9	100	0.35	39	29 325		DY	-0.13123		RZ	3	1030		0.41	-2337.47	2382.21
10	100	0.4	39	30.615	44	DY	-0.13598		RZ	3	1030	DX	0.47	-2422.73	2383.58
11	100	0.45	39	32.05		DY	-0.14079		RZ	3	1030		0.48	-2508.02	2384.97
12	100	0.5	39	32.058		DΥ	-0.14576		RZ	3	1030	DX	0.45	-2593.32	2386.3
13	100	0.55	39	36.736	- 44	DY	-0.1507	50	RZ	3	1030	DX	0.46	-2678.52	2387.79
14	100	0.6	39	36.122	44	DY	-0.15553		RZ	3	1030	DX	0.52	-2763.81	2389.08
15	100	0.65	39	39.947	44	DY	-0.16104		RZ	3	1030	DX	0.43	-2849.15	2390.5
16	100	0.7	39	42.802	44	DY	-0.16704	50	RZ	3	1030	DX	0.3	-2934.38	2391.81
17	100	0.75	39	46.09	44	DY	-0.17143	50	RZ	3	1030	DX	0.36	-3019.68	2393.26
18	100	0.8	39	49.914	44	DY	-0.17752	50	RZ	3	1030	DX	0.28	-3104.92	2394.65
19	100	0.85	39	51.199	44		-0.18261		RZ.	3	1030	DX	0.25	-3190.19	2395.97
20	100	0.9	39	55.462	44	DY	-0.18831	50	RZ	3	1030	DX	0.18	-3275.44	2397.4
21	100	0.95	39	57.492	4		-0.19367		RZ	3	1030	DX	0.14	-3360.7	2398.72
22	100	1	39	57.686	44		-0.19892		RZ	3	1030	DX	-0.03	-3446.06	2399.92
23	100	1.05	39	63.898	44		-0.20416		RZ	3	1030	DX	-0.07	-3531,3	2401.46
24	100	1.1	39	64.067	44		-0.20891		RZ	3	1030		-0.02	-3616.5	2402.72
25	100	1.15	39	80.575	44		-0.22646		RZ	3	1030		-1.62	-3701.3	2403.73
26	100	1.2	39	81.295	44		-0.21882		RZ	3	1030		-0.13	-3787.14	2406.05
27	100	1.25	39	640.491	251		-0.34747	7030		3	1030	DX [	1.5	-3871.03	2452.82
28	100	1.3	3	1178.67	251	DY	-0.82751	7030	RZ	3	1030	DX	2.68	-3956.14	2459.69

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						COLL	APSE SO	LUTION S	UMMAF	RY		····	· · · ·	•	
LOAD	LOAD	NO.	NO.	MAXIMU	JM DEFL	ECTION	MAXIM	UM ROTA	TION	SOL	UTION D	ATA	REACT	ION SUMM	ATION
INCR	CASE	FACTOR	LOOPS	DEFL.	JOINT	DOF	ROT.	JOINT	DOF	MAX.	JOINT	DOF	FX	FY	FZ
СМ	DIGITS	KN	KN	KN						DIGITS				. [	
1	CLPS	0.5	39	10.769	42	DY	-0.03068	50	RZ	3	1030	DX	443.41	-783.69	1181.67
2	CLPS	1	39	21.57	42	DY	-0.0612		RZ	3	1030	ÔX.	886.87	-1587.36	2363.4
3	100	0.05	39	22.661	42	DY	-0.0642		RZ	3	1030	DX	930.18	-1843.92	2364.3
4	100	0.1	39	23.72		DY	-0.06719	50	RZ	3	1030		973.46	-1720.49	2365.1
5	100	0.15	39	24.833	42	DY	-0.07047		RZ	3	1030	DX	1016.78	-1797.08	2366.0
6	100	0.2	39	25,945	42	DY	-0.07383	50	RZ	3	1030	DX	1060.04	-1873.65	2367.1
7	100	0.25	39	26,982	42	DY	-0.07663	50	RZ	3	1030	DX	1103.4	-1950.21	2367.8
8	100	0.3	39	28.094	42	DY	-0.07973	50	RZ	3	1030	DX	1146.72	-2026.79	2368.60
8	100	0.35	39	29.192	42	DY	-0.08306	50	RZ	3	1030	DX	1189.98	-2103.39	2369.78
10	100	0.4	39	30,244	42	DY	-0.08575		RZ	3	1030	DX	1233.33	-2179.92	2370.4
11	100	0.45	39	31.36		DY	-0.08922		RZ	3	1030		1276.62	-2256.51	2371.4
12	100	0.5	39	32.468		DY	-0.09218		RŻ	3	1030	DX	1319.91	-2333.08	2372.2
13	100	0.55	39	33.556	42	DY	-0.0955		RZ	3	1030	DX	1363.2	-2409.64	2373.3
14	100	0.6	39	34.618	42		-0.09838		RZ	3	1030		1406.51	-2486.21	2374.1
15	100	0.65	39	35.685		DY	-0.10077		RZ	3	1030		1449.93	-2562.76	2375.2
16	100	0.7	39	36,791		DY	-0.10432		RZ	3	1030		1493.23	-2639.35	2375.7
17	100	0.75	39	37.851		DY	-0.10873		RZ	3	1030		1536.59	-2715.9	2376.6
18	100	0.8	39	38.968	42		-0.11019		RZ	3	1030		1579.77	-2792.49	2377.5
19	100	0.85	39	40.069	42	DY	-0.11354		RZ	3	1030	DX	1623.2	-2869.07	2378.2
20	100	0.9	39	41 151	42	DY	-0.11718	50	RZ	3	1030	DX	1668.49	-2945.65	2379.2
21	100	0.95	39	42.271	42	DY	-0.12079	50	RZ	3	1030	DX	1709.59	-3022.2	2380.10
22	100	1	39	43.331	42	DY	-0.12371	50	RZ	3	1030	DX	1752.96	-3098.79	2381.1
23	100	1.05	39	44,599	42	DY	-0.12922	50	RZ	3	1030	DX	1795.83	-3175.4	2381.9
24	100	1.1	39	45.566	42	DY	-0.13155	50	RZ	3	1030	DX	1839.45	-3251.9	2382.74
25	100	1.15	39	46.545		DY	-0.13379		RZ	3	1030	DX	1882.6	-3328.45	2383.8
26	100	1.2	39	50.877	23	DY	-0.13478	50	RZ	3	1030	DX	1926.67	-3404.97	2385.1
27	100	1.25	39	50.565		DY	-0.13919		RZ	3	1030		1969.27	-3481.57	2385.5
28	100	1.3	39	62 139	44		-0.14425		RZ	3	1030		2012.44	-3558.26	2387.2
29	100	1.35	39	55.937	44		-0.1459		RZ	3	1030		2055.93	-3634.71	2387.2
30	100	1.4	.39	59.907	44		-0.15099		RZ	3	1030		2099.14	-3711.28	2388.3
31	100	1.45	39	64.276		DY	-0.15338		RZ	3	1030		2142.73	-3787.85	2389.2
32	100	1.5	39	72.673	32		-0.15837		RZ	3	1030		2185.58	-3864.42	2390.6
33	100	1.55	39	72.984	32		-0.16222		rz.	3	1030		2228.88	-3940.94	2390.9
34	100	1.8	39	88,595	32		-0.16642		RZ.	3	1030		2272.06	-4017.5	2392.5
35	100	1.65	39	91.697	32		-0.17001		RZ	3	1030		2315.35	-4094.06	2392.9
36	100	1.7	39	95.587	32		-0.17394		RZ	3	1030		2358.58	-4170.64	2393.7
37	100	1.75	39		32		-0.17774		RZ	3	1030		2401.59	-4247.24	2395.4
38	100	1.8	39			ĎΥ	-0.1798		RZ	3	1030		2445.35	-4323.57	2396.0
39	100	1.85	39			DY	-0.18391		RZ	3	1030		2488.43	-4400.06	2397.0
40	100	1,9	25	474.532	251	DY	-2.54401	217	RX	3	1030	DX	2531.4	-4478.84	2435.9

						COLL	APSE SO	LUTION S	SUMMAF	RÝ					
LOAD	LOAD	NQ.	NO.	MAXIM	IM DEFL	ECTION	MAXIM	UM ROTA	TION	SOL	UTION D	ATA	REACT	10N SUMM	ATION
INCR	CASE	FACTOR	LOOPS	DEFL.	JOINT	DOF	ROT.	JOINT	DOF	MAX.	JOINT	DQF	FX	FY	FZ
CM	DIGITS	KN	KN	KN			:			DIGITS					
1	CLPS	0.5	39	-10.309	251	DX	0.00263	4013	RZ	3	1030	DX	731.54	-431.4	1182.46
2	CLPS	1	39	-16.828	251	DX	0.00496	20	RY	3	1030	DX	1463.09	-862.8	2364.97
3	100	0.05	39	-23.32	251	OX	-0.00691	6020	RY	3	1030	DX	1534.46	-904.94	2368.14
4	100	0.1	39	-20.381	251	DX	0.00536	4013		3	1030	DX	1605.89	-947.07	2366.97
5	100	0.15	39	-19.022		DX	0.00542	4013		3	1030	DX	1677.26	-989.19	2367.87
6	100	0.2	39	-26.205	251		-0.00621	7615		3	1030		1748.66	-1031.34	2369.0
7	100	0.25	39	-21.043	251		0.00656	4013		3	1030		1820.16	-1073.45	2369.8
8	100	0.3	39	-19.998	51	DX	0.0062	4013	RZ	3	1030		1891.57	-1115.57	2370.6
9	100	0.35	39	-22.094	251	DX	0.00747	4013	RZ	3	1030	DX	1963.01	-1157.69	2371.81
10	100	0.4	39	-23.802	251	DX	0.00764	4013	RZ	3	1030	DX	2034.43	-1199.82	2372.79
11	100	0.45	39	-22.826	251	DX	-0.00698	5130	RZ	3	1030	DX	2105.62	-1241.98	2373.5
12	100	0.5	39	-23.142	51	DX	-0.00663	5130	RZ	3	1030	DX	2177.27	-1284.12	2374.4
13	100	0.55	39	-23,92		DX	-0.00678	5130	RZ	3	1030	DX	2248.5	-1326.24	2375.5
14	100	0.6	39	-25.075		DX	0.01056	8021		3	1030		2319.66	-1368.41	2376.33
15	100	0.65	39	-32.437	251		-0.00866	7615		3	1030		2391.28	-1410.52	2377.80
18	100	0.7	39	-26.294	51	DX	-0.00804	5130	RZ	3	1030		2462.58	-1452.65	2378.47
17	100	0.75	39	-26.919	51	DX	0.00806	4013	RZ	3	1030	DX	2534.4	-1494.77	2379.2
18	100	0.8	39	-36.656	44	DX	0.0184	8041	RZ	3	1030	DX	2605.08	-1536.98	2380.0
19	100	0.85	39	-32.07	251	DX	-0.01507	8120	RX	3	1030	DX	2677.45	-1578.69	2381.
20	100	0.9	39	-30.13	51	DX	0.01723	9020	ŘX I	3	1030	DX	2747.68	-1621.27	2381.87
21	100	0.95	39	-30.774	51	DX	0.01632	9020	RX	3	1030	DX	2819.39	-1663.43	2382.69
22	100	1	39	-31.583	51	DX	0.01731	9020	RX	3	1030	DX	2890.73	-1705.57	2383.65
23	100	1.05	39	-32.211	51	DX	-0.0138	8225	RY	3	1030	DX	2982.29	-1747.66	2384.62
24	100	1.1	39	-46.69	44	DX	-0.01769	8225	RY	3	1030	DX	3033.48	-1789.85	2386.68
25	100	1.15	39	-34.207	51	DX	0.02208	9020	RX	3	1030	DX	3104.57	-1631.98	2386.44
26	100	1.2	39	-33.921	51	DΧ	0.00975	4013	RZ	3	1030	DX	3176.79	-1873.87	2388.2
27	100	1.25	39	-34.35		DX	0.01287	4013	RZ	3	1030	DX	3248.19	-1915.93	2389.1
28	100	1.3	39	-39.264	44	DX	-0.01663	7625	RY	3	1030	DX	3319.08	-1958.29	2390.0
29	100	1.35	39	45.038	9020		0.03256	9020		3	1030		3389.68	-2000.59	2389.9
30	100	1.4	39			DX	-0.01315	8225		3	1030		3462.66	-2042.45	2391.0
31	100	1.45	39		44	DX	-0.01557	7625	RY	3	1030	DX	3533.14	-2084.59	2393.0
32	100	1.5	39	-42.372	4013	DX	0.02823	8520	RX	3	1030	DX	3604.13	-2127.01	2393.03
33	100	1.55	37	295.575	8041	DY	-0.37213	8041	RZ	3	1030	ĎΧ	3667.73	-2168.57	2394.20

#### 180 deg

						COLL	APSE SO	UTION S	UMMAF	ŧΥ	-				
LOAD	LOAD	NO.	NO.	MAXIMU	JM DEFL	ECTION	MAXIM	UM ROTA	TION	SO	UTION D	ATA	REACT	ION SUMM	ATION
INCR	CASE	FACTOR	LOOPS	DEFL.	JOINT	DOF	ROT.	JOINT	DOF	MAX.	JOINT	DOF	FX	FY	FZ
CM	DIGITS	KN	KN	KN						DIGITS			1 1	1	(
												i			
	CLPS	0.5	39	-12.422		DX	0.03178	50		3	1030	DX	875.83	-0.68	1181.68
2	CLPS	1	39	-24.867		DX	0.06329		RZ	3	1030		1751.63	-1.36	2363.36
3	100	0.05	39	-26,115		DX	0.06641		RZ	3	1030		1837.62	-1.42	2384.22
4	100	0.1	39	-27.425		DX	0.06884		RZ	3	1030		1923.61	-1.49	2365.12
5	100	0.15	39	-28.67		DX	0.07216		RZ	3	1030	DX	2009.59	-1.57	2366.17
6	100	0.2	39	-29.883	18	DX	0.07521		RZ	3	1030	DX	2095.61	-1.63	2367.01
7	100	0.25	39	-31.15		DX	0.07823		RZ	3	1030	DX	2181.63	-1.68	2367.68
8	100	0.3	39	-32.331	18	DX	0.08175	50	RZ	3	1030	DX	2267.64	-1.75	2368.46
9	100	0.35	39	-33.802	18	DX	0.08411	50	RZ	3	1030	DX	2353.57	-1.82	2369.47
10	100	0.4	39	-34.833	18	DX	0.08805	50	RZ	3	1030	DX	2439.55	-1.87	2370.33
11	100	0.45	39	-36.053	18	DX	0.09121	50	RZ	3	1030	DX	2525.64	-1.95	2371.24
12	100	0.5	39	-37.368	18	DX	0.09347	50	RZ	3	1030	DX	2611.58	-2.02	2372.01
13	100	0.55	39	-38.726	18	DX	0.09662	50	RZ	3	1030	DX	2697.63	-2.07	2372.8
14	100	0.6	39	-39.713	18	DX	0.10091	50	RZ	3	1030	DX	2783.81	-2.12	2373.81
15	100	0.65	39	-40.899		DX	0.1054	50		3	1030	DX	2869.68	-2.17	2374.96
16	100	0.7	39	-42.512	18		0.1049		RZ	3	1030		2955.4	-2.28	2375.57
17	100	0.75	38	-43.421	18		0.11145		RZ	3	1030		3041.69	-2.3	2378.84
18	100	0.8	39	-44.708	18		0.11427		RZ	3	1030		3127.29	-2.35	2377.23
19	100	0.85	39	-46.065	18		0.11499	50		3	1030		3213.66	-2.47	2378.01
20	100	0.9	39	-47.645		DX	0.11438	50		3	1030		3299.57	-2.6	2378.54
21	100	0.95	39	-48,808	18		0,11844		RZ	3	1030		3385.68	-2.65	2379.43
22	100	1	39	-49.813		DX	0.12371		RZ	3	1030		3471.59	-2.65	2380.6
23	100	1.05				DX	0.11251		RZ	3	1030		3557.17	-2.95	2380.68
24	100	1.1	39			DX	0.11676		RŻ	3	1030		3643.4	-2.98	2381.65
25	100	1.15	39	-56.397	12		0.13426		RZ	3	1030		3729.62	-2.82	2384.35
26	100	1,2	20	-390.73	9020	DX	0.58143	9020	RY	3	1030	DX	3880.84	-3.27	2383.51

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						COLL	APSE SO	LUTION S	UMMAF	RY .					
LOAD	LOAD	NO.	NO.	MAXIMU	JM DEFL	ECTION	MAXIM	UM ROTA	TION	SO	UTION D	ATA	REACT	ION SUMM	ATION
INCR	CASE	FACTOR	LOOPS	DEFL.	JOINT	DOF	RÓT.	JÖINT	DOF	MAX.	JOINT	DÖF	FX	FY	FZ
СМ	DIGITS	KN	KN	KN		1			i i	DIGITS					
										1					
1	CLPS	0.5	39	11.605	251	DX	0.04899	50	RZ.	3	1030	DX	731.36	431.3	1186.97
2	CLPS	1	39	23.439	251	DX	0.09784	50	RZ.	3	1030	DX	1462.63	862.59	2374.13
3		0.05	39	22.068	251	DX	0.10296	50	RZ	3	1030	DX	1534.03	904.74	2375.53
4	100	0.1	39	23 756	251	DX	0.10797	50	RZ	3	1030	DX	1605.30	946.86	2376.98
5	100	0.15	39	-24.548	251	DX	0.11261	50	RZ.	3	1030	DX	1676.79	988.99	2378.38
6	100	0,2	39	-22.646	251		0.11733		RZ.	3	1030		1748.21	1031.13	2379.74
7	100	0.25	39	-27.388	251		0.12141		RZ	3	1030		1819.55	1073.23	2381.31
8	100	0.3	39	25.124	251		0.12654		RZ	3	1030		1890.97	1115.39	2382.61
9	100	0.35	39	28.213	251		0.13099		RZ	3			1962.32	1157.5	2384.07
10	100	0.4	39	-25.84	251		0.13691		RZ	3			2033.72	1199,64	2385.46
11	100	0.45	39		251		0.14117		RZ	3			2105.06	1241,75	2388.93
12	100	0.5	39	-29.186	251	DX	0.14484		RZ	3	1030		2178.5	1283.66	2388.43
13	100	0.55	39	-30.402	22	DX	0.14996	50	RZ	` 3	1030	DX	2247.87	1326.01	2389.84
14	100	0.6	39	• 34.137	12	DX	0.15505	50	RZ	3	1030	DX	2319.21	1368.15	2391.33
15	100	0.65	39	-30.481	12	DX	0.16051	50	RZ	3	1030	DX	2390.57	1410.27	2392.72
16	100	0.7	39	-33.137	22	DX	0.16505	50	RZ	3	1030	DX	2461.96	1452.43	2394.08
17	100	0.75	39	-36,442	22	DX	0.16876	50	RZ	3	1030	DX	2533.39	1494.56	2395.38
18	100	0.8	39	-38.085	22	DX	0.17344	50	RZ	3	1030	DX	2604.74	1538.7	2396.88
19	100	0.85	39	40.298		DX	0.17753	50	RZ	3	1030	DX	2676.1	1578.81	2398.28
20	100	0.9	39	-43.38	22	DX	0.1819	50	RZ	3	1030	DX	2747.48	1620.97	2399.79
21	100	0.95	39	-44.402		DX	0.18487		RZ	3	1030		2818.97	1863.06	2401.01
22	100	1	39	-47.203	13	DX	0.19014	50	RZ	3	1030	DX	2890.37	1705.16	2402.42
23	100	1.05	39	-51.624	13	DX	0.19541		RZ	3	1030		2961.63	1747.34	2403.94
24	100	1.1	39	-55.478	13	DX	0.19912	50	RZ	3	1030	DX	3033.05	1789.46	2405.44
25	100	1.15	39	-55.32	13	DX	0.20141	50	RZ	3	1030	DX	3104.61	1831.52	2406.74
26	100	1.2	39	-60.265	13	DX	0.20712	50	RŻ	3	1030	DX	3175.78	1873.7	2408.12
27	100	1.25	39	63.267	13	DX	0.21222	50	RZ	3	1030	DX	3247.21	1915.85	2409,48
28	100	1.3	39	-69.165	13	DX	0.21635	50	RZ	3	1030	DX	3318,6	1957,96	2410.99
29	100	1.35	39	71.856	13	DX	0.22109		RZ	3	1030		3389.92	2000.14	2412.36
30	100	1.4	39	-81.221	13	DX	0.22586		RZ	3	1030	DX	3461.21	2042.28	2414.17
31	100	1.45	39	-94.975	13	DX	0.2297	50	RZ	3	1030	DX	3532.6	2084.36	2415.85
32	100	1.5	39	-93.377		DX	0.23533		RZ	3	1030		3603.87	2126.52	2417.19
33	100	1.55	37	-488.23	251		0.2704		RZ	3	1030		3670.15	2168.66	2461.35

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### 240 deg

						COLL	APSE SO	UTION S	UMMAF	RY				· · · · · ·	
LOAD	LOAD	NO.	NO.	MAXIM	IM DEFLI	CTION	MAXIM	UM ROTA	TION	SO	UTION D	ATA	REACT	TON SUMM	ATION
INCR	CASE	FACTOR	LOOPS	DEFL.	JOINT	DOF	ROT.	JOINT	DOF	MAX.	JOINT	DOF	FX	FY	FZ
CM	DIGITS	KN	KN	<b>KN</b>						DIGITS			1 1	· · · · · ·	
															1
1	CLPS	0.5	39	-10.846	42	DY	0.06304	50	RZ	3	1030	DX	442.31	784.5	1180.86
2	CLPS	1	39	21.812	42	DY	0.12619	50	RZ	3	1030	DX	884.54	1569.02	2361.75
3	100	0.05	39	-22.837	42	DY	0.13253	50	RZ	3	1030	DX	927.72	1645.68	2362.54
4	100	0.1	39	-23.951	42	DY	0.13828	50	RZ	3	1030	DX	970.95	1722.33	2363.55
5	100	0.15	39	-25.13	42	DY	0.14484	50	RZ	3	1030	DX	1014.1	1799.03	2364.11
6	100	0.2	39	-26 133	42	DY	0.1513	50	RZ	3	1030	DX	1057.28	1875.61	2364.89
7	100	0.25	39	-27.338	42	DY	0.15662	50	RZ	3	1030	DX	1100.5	1952.33	2365.71
8	100	0.3	39	-28.376	42	ĎΥ	0.16363	50	rz	3	1030	DX	1143.59	2029	2366.79
9	100	0.35	39	-29.466	42	DY	0.17014	50	RZ.	3	1030	DX	1186.77	2105.7	2367.38
10	100	0.4	39	-30.657	42	DY	0.17589	50	RŽ	3	1030	DX	1230.12	2182.24	2368.2
11	100	0.45	39	-31.671	42	DY	0.18227	50	RZ.	3	1030	DX	1273.26	2258.86	2368.92
12	100	0.5	39	-32.787	42	DY	0.18854	50	RZ	3	1030	DX	1316.35	2335.65	2369.92
13	100	0.55	39	-34.064	42	DY	0.1944	50	RZ	3	1030	DX	1359.61	2412.32	2370.45
14	100	0.6	39	-34.961	- 42	DY	0.20054	50	RZ	3	1030	DX	1402.72	2488.98	2371.83
15	100	0.65	39	-36.027	42	DY	0.20698	50	rz	3	1030	DX	1445.84	2565.68	2372.64
16	100	0.7	39	-37,285	42	DY	0.21308	50	RZ	3	1030	DX	1489.01	2642.32	2373.32
17	100	0.75	39		42		0.21827		r,Z	3	1030		1532.4	2718.94	2373.69
18	100	0.8	39	39.259	42		0.2261		RZ	3			1575.26	2795.7	2374.64
19	100	0.85	39		42		0.22979		RZ	3			1618.86	2872.11	2375.38
20		0.9	39		42		0.23578		RZ	3	1030		1661,83	2949.05	2378.02
21	100	0.95	39	-43.213	42		0.2418		RZ	3	1030		1705.41	3025.45	2376.73
22	100	1	17	-97.237	7110	DY	0.25742	50	RZ	3	1030	DX	1762.63	3159.82	2376.69

						COLL	APSE SO	<b>LUTION S</b>	UMMAP	RY					
LOAD	LOAD	NO.	NO.	MAXIMU	JM DEFL	ECTION	MAXIM	UM ROTA	TION	SO	UTION E	ATA	REACT	ION SUMM	ATION
INCR	CASE	FACTOR	LOOPS	DEFL.	JOINT	DOF	ROT.	JOINT	DOF	MAX.	JOINT	DOF	FX	FY	FZ
CM I	DIGITS	KN	KN	KN		[	[ [		Í	( DIGITS (		1	1 1	ł	
1	CLPS	0.5	39	-9.783	44	DY	0.04918	50	RZ.	3	1030	DX	-0.6	871.56	1179.27
2	CLPS	1	39	-17.901	42	DY	0.09912	50	RZ	3	1030	DX	-1.23	1743,1	2358.64
3	100	0.05	39	-18.79	42	DY	0.1038	50	RŻ	3	1030	DX	-1.31	1828.52	2359.34
4	100	0,1	39	-20.299	251	DY	0.10894	50	RZ	3	1030	ÐX	-1.4	1913.97	2360.04
5	100	0.15	39	-20.633	42	DY	0.11356	50	RZ.	3	1030	DX	-1.43	1999.4	2360.62
6	100	0.2	39	-21.474	42	DY	0.11865	50	RZ	3	1030	DX .	-1.49	2084.8	2361.28
7	100	0.25	39	-22.381		DY	0.12363		RZ	3	1030	DX	-1,57	2170.26	2361.9
8	100	0.3	39	-23.297		DY	0.12819		RZ	3	1030		1.61	2255.68	2382.59
9	100	0.35	39	-24.155		DY	0.13368		RZ	3	1030		-1.74	2341,1	2383.25
10	100	0,4	39	-25.423		DΥ	0.13784		RZ	3	1030		-1.71	2426.48	2363.99
11	100	0.45	39	-26.884	44	DY	0.14353		RZ	3	1030		-1.94	2511.99	2364.7
12	100	0.5	39	-26.924	42	DY	0.14761		RZ	3	1030		-1.84	2597.4	2365.17
13	100	0.55	39	-36.255	44	DY	0.15299	50	RZ_	3	1030	DX	-2.04	2682.85	2366.35
14	100	0.6	39	-28.589	42	DY	0.15834	50	RZ	3	1030	DX	-1.98	2767.99	2366.53
15	100	0.65	39	-31.248	251	DY	0.16286	50	RZ	3	1030	DX	-2.09	2853.63	2367.37
16	100	0.7	39	-38.646	44	DY	0.16956	50	RZ	3	1030	DX	-2.59	2939.24	2368.5
17	100	0.75	39	-33.011	251	DY	0.17146	50	RZ	3	1030	DX	1.91	3024.38	2368.63
18	100	0.8	39	-32.454	42	DY	0.17715	50	RZ	3	1030	DX	-2.18	3110.09	2368.94
19	100	0.85	39	-34.167		DY	0.17685	50	RZ	3	1030	DX	-1.44	3195.11	2369.49
20	100	0,9	39	-34.128	42	DY	0.18818	50	RZ.	3	1030	DX	-2.53	3281.01	2370.39
21	100	0.95	39	-35.603	28	DY	0.18851		RZ	3	1030	DX	-1.6	3366.06	2370.74
22	100	1	39	-35.797	42	DY	0.19704	50	RZ	3	1030	DX	-2.68	3451.88	2371.75
23	100	1.05	39	-37.39	28	DY	0.19864		RZ	3	1030	DX	-1.73	3536.97	2371.98
24	100	1.1	39	-37,574		DY	0.20832		RZ	3	1030		-2.67	3622.18	2373.21
25	100	1.15	39	-41.025		DY	0.20837		RZ	3	1030		-1.59	3707.36	2373.76
26	100	1.2	39	-52,324	23	DY	0.2077	50	RZ	3	1030	DX	-0.46	3792.63	2373.92
27	100	1.25	39	-40.192	42	DY	0.22253	50	RZ	3	1030	DX	-2.88	3878.26	2375.33
28	100	1.3	39	-45,198	41	DY	0.22949	50	RZ	3	1030	DX	-3,76	3964 49	2376.78
29	100	1.35	39	-42.061	42	DY	0.23109	50	RZ	3	1030	DX	-3.19	4049.63	2376.57
30	100	1,4	39	-54.671	22	DY	0.22621	50	RZ	3	1030	DX	-0.92	4134.51	2377.03
31	100	1.45	39	-59.19	8040	DY	0.23037	50	RZ	3	1030	DX	0.18	4219.55	2378.62
32	100	1.5	39	-71.726	8040	DY	0.23122	50	RZ	3	1030	DX	0.9	4304.67	2377.64
33	100	1.55	39	-92.868	7515	DY	0.24624	50	RZ	3	1030	DX	-1.15	4389.32	2376.82
34	100	1.6	2	107.409	7515	DX	0.21788	42	RZ	3	1030	DX	28.41	4491.73	2382.4

## 300 deg

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LOAD	LOAD	NO.	NÖ.	MAXIMU	JM DEFL	ECTION	MAXIM	UM ROTA	TION	SO	LUTION D	ATA	REACT	ION SUMM	ATION
INCR	CASE	FACTOR	LOOPS	DEFL.	JOINT	DOF	ROT.	JOINT	DOF	MAX.	JOINT	DOF	FX	FY	FZ
CM	DIGITS	KN	KN	KN			• ·			DIGITS			1		
												1.			
1	CLPS	0.5	39	-10.839	42	DY	0.03163	50	RZ	3	1030	DX	-442.94	785.13	1179.37
2	CLPS	1	39	-21.666	42	DY	0.06312	50	RZ	3	1030	DX	-885.86	1570.31	2358.8
3	100	0.05	39	-22.891	44	DY	0.06603	50	RZ	3	1030	DX	-929.1	1646.98	2359.67
4	100	0.1	39	-23.844	51	DY	0.06951	50	RZ	3	1030	DX	-972.37	1723.73	2360.14
5	100	0.15	39	-24.919	51	DY	0.0729	50	RZ	3	1030	DX	-1015.65	1800.5	2360.69
6	100	0.2	39	-28.007	51	DY	0.07584	50	RZ	3	1030	DX	-1058.87	1877,16	2361.43
7	100	0.25	39	-27.028	51	DY	0.07875	50	RZ	3	1030	DX	-1102.11	1953.9	2362.12
8	100	0.3	39	-28.208	51	DY	0.08238	50	RZ.	3	1030	DX	-1145.38	2030.63	2382.77
9	100	0.35	39	-29.285	51	DY	0.08522		RZ	3	1030	DX	-1188.61	2107.38	2383.37
10	100	0.4	39			DY	0.08926		RZ	3			-1231.93	2184.05	2364,03
11	100	0.45	39	-31.347	51	DY	0.09279	50	RZ	3	1030	DX	-1275,21	2260.76	2364.62
12	100	0.5	39	-32.608	51	ĎΥ	0.0941	50	RZ	3	1030	DX	-1318.27	2337.49	2365.25
13	100	0.55	39	-33.433	51	DY	0.10043	50	RZ	3	1030	DX	-1361.88	2414.3	2366.28
14	100	0.6	39	-34.675		DY	0.1012		RZ	3	1030	DX	-1404,93	2491.13	2366.49
15	100	0.65	39			DY	0.09986		RZ	3			-1447.48	2567.58	2366.84
16	100	0.7	39			DY	0.11059		RŽ	3			-1491.76	2844,54	2368.38
17	100	0.75	39			DY	0.11325		RZ	3			-1535.03	2721.27	2369.18
18	100	0.8	39			DY	0.1027		RZ	3	1030		-1576.44	2797.48	2368.75
19	100	0.85				DY	0.1196		RZ	3	1030		-1621.57	2874,7	2370.44
20	100	0,9	39		4120		0.10801		RZ	3	1030		-1662.4	2950.79	2370.17
21	100	0.95	39			DY	0.11318		RZ	3	1030		-1705,98	3027.97	2370.23
22	100	1	39	-44.2		DY	0.12545		RZ	3	1030		-1750.98	3104.88	2372.92
23	100	1.05	39		8040		0.10786		RZ	3			-1790.43	3180.77	2371.16
24	100	1.1	39		251		0.13188	8038		3			-1830.65	3261.84	2377.3
25	100	1,15	1	4397.64	8041	DZ	36.6716	8021	RZ	3	1030	DX	-1833.56	3255.22	2383.04

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LOAD	LOAD	NO.	NO.			ECTION		UM ROTA			UTION D			ION SUMM	ATION
INCR CM	CASE DIGITS	FACTOR KN	LOOPS KN	DEFL. KN	JOINT	DOF	ROT.	JOINT	DOF	MAX, DIGITS	JOINT	DOF	FX	FY	FZ
1	CLPS	0.5	39	11.443	251	DX	-0.00266	40	RY	3	1030	DX	-731.53	431.94	118
2	CLPS	1	39		251	DX	-0.00528	40	RY	3	1030	OX	-1463.06	863.89	237
3	100	0.05	39	21.069	251	DX	-0.00533	40	RY	3	1030	DX	-1534.46	906.07	237
4	100	0.1	39	21.975	251		-0.00534	40	RY	3	1030	DX	-1605.87	948.29	237
5	100	0.15	39	24.481	251	DX	0.00573	7615	RY	3	1030	DX	-1677.25	990.47	237
6	100	0.2	39		251		0.00565	7615	RY	3	1030	DX	-1748.66	1032.65	237
7	100	0.25	39	25.63		DX	-0.00641	4013	RZ	3	1030	DX	-1820.08	1074.65	23
8	100	0.3	39		251		0.0063	7615		3	1030		-1891.51	1117.05	238
9	100	0.35	39	27.325	251		0.00662	7615		3	1030	DX	-1962.83	1159.23	238
10	100	0.4	39	27.079	251		-0.00658	4013		3	1030		-2034.31	1201.41	236
11	100	0.45	39	30.942		DX	0.00754	7615		3	1030		-2105.71	1243.62	238
12	100	0.5	39	27.824	251		0.00684	7615	RY	3	1030		-2177.12	1285.79	235
13	100	0.55	39	29.137	251		0.00792	7010	RÝ	3	1030	DX	-2248.53	1327.95	238
14	100	0.6	39	31.603	32	DX	0.00781	7615	RY	3	1030	DX	-2319.89	1370.19	236
15	100	0.65	39	32.468	32	DX	0.00805	7615	RY	3	1030	DX	-2391.28	1412.4	230
16	100	0.7	39	33.039	32	DX	-0.0089	4013	RZ	3	1030	DX	-2482.58	1454.58	239
17	100	0.75	39	37,57	32	DX	0.00943	7615	RY	3	1030	DX	-2534.09	1496.88	239
18	100	0.8	39	38.63	32	DX	0.00968	7615	ŔY	3	1030	DX	-2605.38	1539.02	23
19	100	0.85	39	39.092	32	DX	0.00946	7615	RY	3	1030	DX	-2676.77	1581.10	239
20	100	0.9	39	44.938	32	DX	0.01093	7615	RY	3	1030	DX	-2748.16	1623.58	239
21	100	0.95	39	44.433		DX	0.01035	7615	RY	3	1030	DX	-2819.58	1865.57	240
22	100	1	39	46.89	32	DX	0.01114	7615	RY	3	1030	DX	-2891.02	1707.87	240
23	100	1.05	39	52.59	33	DX	0.01226	7615		3	1030	DX	-2962.44	1750.05	240
24	100	1.1	39	55.319		DX	0.01251	7615		3	1030		-3033.68	1792.41	240
25	100	1.15	39	58.664		DX	-0.01511	2121	RZ	3	1030		-3104.75	1834.51	240
26	100	1.2	39	67.436		DX	-0.01795	2121		3	1030		-3176.06	1876.78	240
27	100	1.25	39			DX	-0.01923	<u>2121</u>		3	1030		-3247.82	1918.69	
28	100	1.3	39			DX	-0.03141	2121		3	1030		-3318.77	1961.1	241
29	100	1.35	39	84.705		DX	-0.06802	2121		3	1030		-3389.98	2003.7	241
30	100	1.4	39	96.124		DX	-0.08888	2121		3	1030		-3460.72	2045.95	241
31	100	1.45	39	98.942		DX	-0.09702	2121		3	1030		-3531.81	2088.2	241
32	100	1.5	39	123.9		DX	-0.11118	2121		3	1030		-3602.87	2130.08	241
33	100	1.55	39	462.379	251		0.28271	7020		3	1030		-3677.46	2171.07	245
34	100	1.6	2	694.704		DX	0.58018	7021	RX	3	1030	DX	-3545.99	2409.5	247

APPENDIX C.2 C) PLATFORM C

							APSE SC				11714		1		
INCO	LOAD	LOAD	NO.		JM DEFLE			NUM ROT			UTION			TION SUM	
INCR	CASE	FACTOR	LOOPS	DEFL. CM	JOINT	DOF	ROT.	JOINT	DOF	MAX. DIGITS	JOINT	DOF	FX KN	FY KN	FZ KN
1	CLPS	0.5	29	-418.73	7004		0.07811	7020		2			-4511.72	138.61	48855.73
_	CLPS	1	13	-696.78	7004		0.12751	7020		2	7005		-9013.13	273.39	
3	100	0.5	2	-697.02	7004		0.12753	7020		2	7005		-10642.4	340.86	
4 5	100	0.59	1	-697.05	7004		0.12754	7020		2			-10951.9	353.68 366.5	
5 6	100	0.89		-697.1 -697.15	7004		0.12755	7020		2			-11571	379.32	
7	100	0.88	2	-697.2	7004		0.12755	7020		2			-11880.6	392.14	
8	100	0.97	2	-697.25	7004		0.12756	7020		2	7005		-12190,1	404.96	
9	100	1.07	4	-697.29	7004		0.12756	7020		2	7005		-12499.7	417.78	97656.20
10	100	1.17	1	4697.33	7004		0.12757	7020		2	7005		-12809.2	430.6	
11 12	100	1.26	2	-697,38	7004		0.12757	7020		2	7005		-13118.8	443.42	
13	100	1.35 1.45	2	-697.42	7004		0.12758	7020		2	7005		-13428.3	456.24	
14	100	1.54	1	-697.51	7004		0.12758	7020		2	7005		-14047.4	481.88	
15	100	1.64	2	-697.56	7004	DZ	0.12759	7020		2	7005		-14357	494.69	
16	100	1.74	1	-697.6	7004		0.12759	7020		2			-14666.6	507.51	
17	100	1.83	2	-697.65	7004		0.1276	7020		2			-14976.1	the second s	97616.07
18	100	1.93	1	-697,69	7004		0.1276	7020		2	7005		-15285.7	533.15	
<u>19</u> 20	100	2.02	<u>2</u> 1	-697.74 -697.79	7004		0 12761	7020		2	7005		-15595.2	545.97 558.79	97608. 97601.12
20	100	2.21	2	-697.84	7004		0.12762	7020		2	7005		-16214.3	571.61	
22	100	2.3	2	697.69	7004		0.12762	7020		2	7005		-16523.9	584.43	97591.2
23	100	2.4	2	-697.93	7004	DZ	0.12763	7020	RY	2	7005	DY	-16833.4	597.25	97586.2
24	100	2.5	4	-697.97	7004		0.12763	7020		2	7005		-17143	610.07	97581.3
25	100	2,59	1	-698.02	7004		0.12763	7020		2	7005		-17452.5	622.89	
26 27	100 100	2.69 2.78	2	-698.07	7004		0 12764	7020		2	7005		-17/62.1	635.7 648.52	
28	100	2.68	2	698.16	7004		0.12765	7020		2			-18381.2	661.34	
29	100	2.97	1	-698.2	7004		0.12765	7020		2			-18690.8	674.16	the second s
30	100	3.06	2,	-698.25	7004	DZ	0.12766	7020	RY	2			-19000.3	686.98	
31	100	3.16	1	-698.29	7004		0.12766	7020		2			-19309.9	699.8	97546.92
32	100	3.25	2	-698.34	7004		0.12767	7020		2	7005		-19619.4	712.62	97542.05
33 34	100 100	3.35	1	-698.38	7004		0.12767	7020	and the second	2			-19929	725.44	
34	100	3.44 3.54	2	-698.43 -698.48	7004		0.12768	7020		2	7005		-20238.5	738.25	
36	100	3.63	2	698.53	7004		0 12769	7020		2			-20857.6	763.89	
37	100	3.73	2	-698.58	7004		0.12769	7020	RY	2	7005		-21167.2	778.71	
38	100	3.83	2	-698,63	7004		0.1277	7020		2	7005		-21476.7	789.53	
39	100	3.92	4	-698.67	7004		0.1277	7020		2	7005		-21786,3	802.35	97508.
40	100 100	4.02	1	-698.71 -698.76	7004		0.1277	7020		2	7005		-22095.8	815,17 827.98	
42	100	4.21		-698.8	7004		0.12771	7020		2	7005		-22715		97493.66
43	100	4.3	2	698.85	7004		0.12772	7020		2			-23024.5	853.62	
44	100	4.39	1	-698.89	7004	DZ	0,12772	7020	RY	2	7005	DY	-23334.1	866.44	97484.08
45	100	4.49	2	698.95	7004		0.12773	7020		2	7005	COLUMN STREET, STRE	-23643.6	879.26	
48	100	4.58		-698.99	7004		0.12773	7020		2	7005		-23953.2	892.07	97474.53
47	100	4.68	2	-699.04	7004		0.12774	7020		2	7005		-24262.7	904.89	
48	100 100	4.78 4.87	2	-699.08 -699.13	7004	_	0.12774	7020		2	7005		-24881.8	917.71	97465.0
50	100	4.96		699.18	7004		0.12775	7020		2			-25191.4		97455.52
51	100		2		7004		0.12776	7020	RY	2			-25500.9		97450.
52	100	5.16	1	-699.26	7004		0.12776	7020	RY	2			-25810.5	968.98	97446.07
53	100	5.25	2	-699.31	7004		0.12776	7020		2			-26120		97441.3
<u>54</u> 55	<u>100</u> 100	5.34	2 2	-699.35	7004	and the second se	0.12777 0.12777	7020		2			-26429.6 -26739.1		97438.66 97431.96
56	100	5.54	4	-699.44	7004		0.12778	7020		2			-20730.1		97427.2
57	100	5.63	2	-699.49	7004		0.12778	7020		2			-27358.2	1033.07	
58	100	5.72	2	-699.53	7004	DZ	0.12779	7020	RY	2	7005	DY	-27667.7		97417.94
59	100	5.82	2	-699.57	7004		0.12779	7020		2	7005		-27977.2		97413.2
60	100	5.92	3	-699.6	7004		0.12779	7020		2	7005		-28286.7		97408.67
61 62	100	<u>6.01</u> 6.11	3	-699.64	7004		0.1278	7020		2	7005	-	-28596.2		97404.00 97399.40
63	100	6.2		-699.69	7004		0.1278	7020		2	7005		-29215.1		97394.9
64	100	6.29	6	699.7	7004		0.1278	7020		2			-29524.5		97390.4
65	100	6.39	5	-699.71	7004	DZ	0.1278	7020	RY	2	7005	DY	-29833.9	1135.59	97385.9
66	100	6,49	7	-699.7	7004		0.1278	7020		2	7005		-30143.2		97381.57
67 68	100	6.58	8 39	-699.68 -698.97	7004		0.12779	7020		2	7005		-30452.4		97377 23 97374 84
69	100	6.67	39	-698.97	7004		0.1277	7020		2	7005		-30/59.1		97383.61
70	100	6.87	39	-694.45	7004		0.12714	7020		2	7005		-31344.1		97393.87
71	100	6.96	39	-679.89	7004		0.12497	7020		2	7005		-31474.4		97539.24
72	100	7.05	39	-641.4	7004	DZ	-0.1769	1638 2342		2	7005		-31486.2		97661.23
73	100	7.15	4	-633.34	7004		-0.5892			2	7005		-31704.8		97728.7

						COL	APSE SC	LUTION	SUMMAR	Y					
	LOAD	LOAD	NO.	MAXIMU	JM DEFLE			UM ROT		SOL	UTION	DATA	REACT	TON SUMM	ATION
INCR	CASE	FACTOR	LOOPS	DEFL. CM	JOINT	DOF	ROT.	JOINT	DOF	MAX. DIGITS	JOINT	DOF	FX KN	FY KN	FZ KN
	CLPS	0.5	29	418.47	7004		0.07808	7026		2			-2247.8	-3991.34	
_	CLPS	1	.13	-696.27	7004		0.12752	7026		2			-4487.68		97390.6
3	100	0.05	1	-696.28 -696.3	7004		0.12753	7026		2			-4587.44 -4687.2	-8302.2 -8827.97	
5	100	0.15	1	-696.32	7004		0.12753	7026		2			-4786.96	-8953.77	
6	100	0.2	2	-696.34	7004		0.12753	7026		2			-4886.71	-9279.54	
7	100	0.25	1	-696.36	7004		0.12753	7026		2			-4986.47	9605.33	97475
8	100	0.3	2		7004		0.12754	7026		2			-5088.23	-9931.11	
9	100 100	0.35	2	-696.42	7004		0.12754	7026		2	7005		-5185.99 -5285.75	10258.9	
11	100	0.45	1	-696.44	7004		0.12754	7026		2	7005		-5385,51	-10908.5	
12	100	0.5	2	-696.46	7004	DZ	0.12754	7026		2	7005		-5485.26	-11234.2	
13	100	0.55	1	-696.48	7004		0.12754	7026		2	7005		-5585.03	-11560	
14 15	100 100	0.6	2		7004		0.12755	7026		2			-5684.78 -5784.54	-11885.8 -12211.6	
18	100	0.00	2	-696.52	7004		0.12755	7028		2			-5884.3	12537.4	
17	100	0.75	1	-696.56	7004		0.12755	7026		2	7005		-5984.08	12883.2	97645
18	100	0.8	2	-696.59	7004		0.12755	7026		2	· · · · · · · · · · · · · · · · · · ·		-6083.82	-13188.9	
19	100	0.85	2	-696.61	7004		0.12756	7026		2			-6183.57	-13514.7	
20 21	100	0.9	4	-696.63	7004		0.12756	7026		2	7005		-6283.33 -6383.1	-13840.5	97696.8 97713.9
22	100	0.85	2	-696.67	7004		0.12758	7026		2	7005		-6482.85	-14492.1	
23	100	1.05	1	-696.68	7004		0.12756	7026		2			-6582.61	-14817.9	
24	100	1.1	2	-696.71	7004	DZ	0.12757	7026	RY	2	7005	DY	-6682.37	-15143.6	97765.2
25	100	1.15	1	-696.73	7004		0.12757	7026		2	7005		-6782.13	15469.4	
26 27	100	1.2 1.25	2	-696.75 -696.77	7004		0.12757	7026		2	7005		-6881.89 -6981.85	-15795.2	97799.5
28	100	1.3	2	-696.79			0.12757	7026		2			-7081.41	-16446.8	
29	100	1.35	1		7004		0.12757	7026		2	7005		-71B1.17	-16772.6	
30	100	1.4	2	-696.83	7004		0.12758	7026		2	_		-7280.93	-17098.3	
31 32	100	1.45 1.5	1	-696.86 -696.88	7004		0.12758	7026		2	7005		-7380,69	-17424.1 -17749.9	97885 97902
33	100	1.55		-696.9	7004		0.12758	7026		2	7005		-7580.21	18075.7	97902
34	100	1.6	2	698.92	7004		0.12758	7026		2			7679.96	-18401.5	
35	100	1.65	1	-696.94	7004	DZ	0.12759	7026		2		DY	-7779.73	-18727.3	97954.2
36	100	17	2	-696.97	7004		0.12759	7026		2	7005		-7879.48	-19053	
37	100	1.75	2	-696.99	7004		0.12759	7026		2			-7979.24	-19378.8	
38 39	100	1.8 1.85	4	-697.01 -697.03	7004		0.12759	7026		2	7005		-8079	-19704.6	
40	100	1.9	2	-697.05	7004		0.1276	7026		2			-8278.52	-20356.2	
41	100	1.95	1	-697.07	7004		0.1276	7026		2			-8378.28	-20682	96057.7
42	100	2	2	-697.1	7004		0.1276	7026		2	7005		-8478.04	7	98075.0
43	100	2.05	- 1	-697.12 -697.14	7004		0.1276	7026		2			-8577.8	21333.5	
45	100	2.15	1	-697.16	7004		0.12761	7026		2	7005		-8777.32	-21985.1	
46	100	2.2	2	-697.19	7004		0.12761	7026		2			-8877.08	-22310.9	98144.2
47	100	2.25	1	-697.21	7004		0.12761	7026		2	7005		-8976.84	-22636.7	98161.
48 49	100 100	2.3 2.35	2	-697.23	7004		0.12761	7026		2	7005		-9076.6	-22962.4	
50	100	2.35	2	-697.26 -697.28	7004		0.12762	7026		2			-9276.12		98213.6
51	100	2.45	2	-697.3	7004		0.12762	7026		2			-9375.88		
52	100	2.5	4	-697.33	7004		0.12762	7028		2			-9475.64		
53	100	2.55	6	697.35	7004		0.12762	7026		2			-9575.4	-24591.3	
54 55	100	2.6	6	-697.38 -697.4	7004		0.12762	7028		2			-9774.92	-24917.1 -25242.9	
56	100	2.03	6	-697.42	7004		0.12763	7026		2	7005		-9874.68		
57	100	2.75	10	-697.45	7004	DZ	0.12763	7028	RY	2	7005	DY	-9974.44	-25894.5	98335.4
58	100	2.8	10	-697.47	7004		0.12763	7026		2			-10074.2	-26220.2	
59 80	100	2.85	14 26	-697.5 -697.52	7004		0.12764	7026 7026		2	7005		10174	-26546 -26871.8	98370.2
61	100	2.95	39	-697.54	7004		0.12764	7026		2			-10373.5		
62	100	3	39	-697.57	7004	DZ	0.12784	7026	RY	2	7005	DY	-10473.2	-27523.3	96422.6
63	100	3.05	39	-697.59	7004		0.12764	7028		2			-10573		
64 65	100	3.1 3.15	<u>39</u> 39	-697.62 -697.62	7004		0.12765	7026 7026		2				-28174.9	
66	100	3.15	39		7004		0.12765	7026		2				-28500.7	
67	100	3.25	39	697.35	7004	DZ	0.12762	7026	RY	2	7005	DY	-10972.6	-29152.6	98509.7
68	100	3.3	39	697.45	7004		0.12763	7026		2	7005		-11072.4		
69 70	100	3.35	39 39	-697.32 -697.53	7004		0.12761	7026		2	7005		-11172.3	-29804.3 -30129.9	
71	100	3.45	39	-697.25	7004		0.1276	7028		2	7005		-11372	-30455.9	
72	100	3.5	39	-697.46	7004	DŻ	0.12763	7026	RY	2	7005	DY	-11471.6	-30781.5	96597.4
73	100	3.55	30	-697.27	7004		0.12761	7026		2			-11571.6		
74	100	3.6	39	-697.48	7004		0.12763	7026		2			-11671.1		
75 78	100 100	3.65 3.7	39 39		7004		0.1278	7026		2				-31747.5 -32065.1	
Q							0.12798	7026		2			-11964.5		98714.7
77	100	3.75	39	-701.13	7004	02	0.121801	10201	<b>NI I</b>	4	7005	זט	1 - 1100-4.01	-923/0.01	00/14./

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	LOAD	LOAD	NQ.		M DEFLE			<b>JUM ROT</b>			UTION		REACT	10N SUMM	IATION
INCR	CASE	FACTOR	LOOPS	DEFL. CM	JOINT	DOF	ROT.	JOINT	DOF	MAX. DIGITS	JOINT	DOF	FX KN	FY KN	FZ KN
1	CLPS	0.5	28	-394.29	7004	DZ	0.07364	7026	RY	2	7005	DY	204.1	-6936.95	49207.
	CLPS	1	11	-658.69	7004	DZ	0.1208	7028	RY	2	7005	DY	404.88	-13861	98416.
3	100	0.05	2	-658.7	7004	DZ	0.1208	7026	RY	2	7005	DY	427.3	-14481	98449.
4	100	0.1	4	-658.7	7004	DZ	0.1208	7026	RY	2	7005	DY	449.72	-15101	98480.
5	100	0.15	2	-858.71	7004	DZ	0.1208	7026	RY	2	7005	DY	472.14	-15721	98510
6	100	0.2	21	-858.72	7004	DZ	0.1208	7028	RY	2	7005	DY	494.57	-16341	98541.
7	100	0.25	2	-658,73	7004	DZ	0.1208	7026	RY	2	7005	DY	516.99	-16961	9857
8	100	0.3	2	-658.74	7004	DZ	0.1208	7026	RY	2	7005	DΥ	539.41	-17580.9	98603.
9		0.35	2	-658.75	7004		0.1208	7026		2	7005		561.83	-18200.9	
10		0.4	2	-658.76	7004		0.1208	7028		2	7005		584,25	-18820.9	
11	100	0.45	2	-658.77	7004		0.1208	7026		2	7005		606.67	-19440.9	9869
12	100	0.5	2	-658.79	7004		0.1208	7028		2	7005		629.09	-20060.9	
13	100	0.55	- 2	-658.8	7004		0.1208	7026		2	7005		651.51	20680.9	
14	100	0.8	4	-658.81	7004		0.1208	7026		2	7005		873.92	21300.8	
15	100	0.65	2	-858.82	7004		0.1208	7026		2	7005		696.34	21920.8	
18	100	0.7	2	-658.83	7004		0.12081	7026		2	7005		718,76		
17	100	0.75	2	-858.84	7004		0.12081	7026		2	7005		741.18	-23160.8	
18	100	0.8	2	-658.86	7004		0.12081	7026		2	7005		763.6		
19	100	0.85	2	-658.87	7004		0.12081	7026		2	7005		786.02	-24400.8	
20	100	0.00	2	-658.88	7004		0.12081	7026		2	7005		808.44	25020.7	98977
21	100	0.95	2	658.9	7004		0.12081	7026		2	7005		830,86	25640.7	99008
22	100	1	2	658.91	7004		0.12081	7026		2	7005		853,27	26260.7	99040
23	100	1.05	- 4	658.92	7004		0.12081	7026		2	7005		875.69	-26680.6	
24	100	1.1	3	-658.94	7004		0.12081	7026		2	7005		898.11	-27500.6	
25	100	1.15	3	-658.96	7004		0.12081	7026		2	7005		920.53	-28120.6	
26		1.2	3	-658.97	7004		0.12081	7026		2	7005		942.96	-28740.5	
27	100	1.25	4	-658.99	7004		0.12082	7028	and the second second	2	7005		965.38	-29360.4	99197
28	100	1.3	7	-659.01	7004		0.12082	7026		2	7005		987.8	-29980.4	99229
29	100	1.35		-659.03	7004		0.12082	7028		2	7005			-29900.4	
30	100	1.4	- 9	-659.05	7004		0.12082	7026		2	7005		1032.66	31220 1	99292
31	100	1.45	10	-659.05	7004		0.12082	7028		2	7005		1052.00	-31839.9	99324.
31	100	1.45	39	-660.12	7004		0.12092	7026		2	7005		1079.52	-32447	99380.
32			39		7004			7026		2	7005				
33	100	1.55	39	-660.86	7004		0.12099			2	7005		1103.02	-33055.1	99402
		1.6		-661.66				7026					1126.67	-33661.6	99446
35	100	1.65	39	-662.2	7004		-0.1226	437		2	7005		1149.49	-34277.8	99481
36	100	1.7	39	-662.32	7004		0.12114	7026		2	7005		1171.88	-34897.3	99512
37	100	1.75	39	-662.71	7004		-0.1234	437		2	7005		1194.69	-35512.6	9954
38	100	1.8	39	-663.57	7004		-0.1219	437		2	7005		1217.62	-36120.4	99589
30	100	1.85	39	-666.41	7004		-0.1577	437		2	7005		1243.12	-36713	99641
40	100	1.9	23	-674.73	7004	DZ	-0.1636	433	RZ	2	7005	DY ,	1272.41	-37295.8	99694.

							APSE SC								
	LOAD	LOAD	NO.		M DEFLE			<b>IUM ROT</b>			UTION			FION SUMM	
INCR	CASE	FACTOR	LOOPS	DEFL. CM	JOINT	DOF	ROT.	JOINT	DQF	MAX. DIGITS	JOINT	DOF	FX KN	FY KN	FZ KN
1	CLPS	0.5	27	-368.89	7004	DZ	0.06897	7026	RY	2	7005	DY	2765.83	-4466.48	48740,75
2	CLPS	1	12	-617.89	7004	DZ	0.11348	7026		2		-	5527.07	-8924.96	97483
3	100	0.05	1	-617.87	7004		0.11347	7026		2			5682.17	-9298.13	
4	100	0.1	1	-617.85	7004		0.11347	7028		2			5837.27		
5 6	100 100	0.15	1	-617.84 -617.82	7004		0.11347	7026		2			5992.37 6147.47	10044.4	
7	100	0.25	1	-617.8	7004		0.11347	7026		2			6302.57	-10790.7	
8	100	0.3	1	-617.79	7004		0.11347	7028		2			6457.67		97617.3
9	100	0.35	1	-617.77	7004		0.11346	7026		2	7005		6612.76		97639.72
10	100	0.4	1	617.76	7004		0.11346	7026		2	7005		6767.86	-11910.2	97682.1
11	100	0.45	1	-617.74	7004		0.11346	7026		2	7005		6922.96	-12283.4	97684.6
12	100	0.5 0.55	1	-617.73 -617.71	7004		0.11346	7026		2	7005		7078.06	-12656.5 -13029.7	97707.0
14	100	0.55		617.7	7004		0.11346	7026		2	7005		7388.25		
15	100	0.65	1	-617.68	7004		0.11345	7026		2	7005		7543.35	-13776	97774.50
16	100	0.7	1	617.67	7004		0.11345	7026		2			7698.44		97797.0
17	100	0.75	1	-617.65	7004	DZ	0.11345	7026	RY	2	7005	DY	7853.54	-14522.3	97819.6
18	100	0.8	1	-617.64	7004		0.11345	7026		2			8008.64		97842.1
19	100	0.85	1	-617.62	7004		0.11345	7026		2			8163.73	Statement of the local division of the local	97864.7
20 21	100	0.9 0,95	2	-617.61 -617.6	7004		0.11344	7026		2	7005		8318.82 8473.92	-15641.7	97887.28 97909.86
21	100	0,95	2	-617.59	7004		0.11344	7026		2			8629.02	-16388	97932.40
23	100	1.05	4	617.57	7004		0.11344	7026		2	7005		8764.11	16761.2	97955.07
24	100	1.1	1	617.56	7004		0.11344	7026		2			8939.21	17134 4	
25	100	1.15	2	-617.54	7004		0.11344	7026	RY	2	7005	DY	9094.3	-17507.5	98000.3
26	100	1.2	1	-017.53	7004		0.11344	7026		2	7005		9249.4		98022.9
27	100	1.25	2	-617.52	7004		0.11343	7026		2			9404.49		98045.6
28	100	1.3	2	-617.5	7004		0.11343	7026		2			9559.59	-18627	98068.2
29 30	100	1.35 1.4	2	-617.49 -617.48	7004		0.11343	7026		2			9714.68		98090.9 98113.6
31	100	1.45	4	-617.46	7004		0.11343	7026		2	7005		10024.88	-19746.4	98138.38
32	100	1.5	2	-617.45	7004		0.11343	7026		2	7005		10179.97	20119.6	98159.11
33	100	1.55	2	617.44	7004		0.11342	7028		2	7005		10335.06	-20492.7	98181.8
34	100	1.6	2	-617.43	7004		0.11342	7026		2	7005		10490.16		
35	100	1.65	2	-617.41	7004		0.11342	7026		2	7005		10645.25	-21239	98227.3
<u>36</u> 37	100	1.7	2	<u>-617.4</u> -617.39	7004		0.11342	7026		2	7005		10800.34		98250.12
38	100	1.8	2	-617.38	7004		0.11342	7028		2	7005		11110.53	-21985.3 -22358.5	98272.9
39	100	1.85	2	-817.36	7004		0.11342	7026		2			11265.63	-22731.6	
40	100	1.9	3	-617.35	7004		0.11341	7026		2			11420.72	23104.8	96341.3
41	100	1.95	2	-617.34	7004		0.11341	7026		2	7005		11575.81	-23477.9	98364.11
42	100	2	3	617.33	7004		0.11341	7026		2			11730.91	-23851.1	98387.0
43	100	2.05	4	-617.32	7004		0.11341	7026		2	7005		11886	-24224.2	98409.5
44	100	2.1	4	-617.31 -617.3	7004		0.11341	7026		2	7005		12041.09	-24597.4 -24970.5	98432.78 98455.67
46	100	2.13		-617.29	7004		0.11341	7026		2	7005		12351.28	-25343.7	98478.58
47	100	2.25	6	-617.28	7004		0.1134	7026		2	7005		12508.37	-25716.8	98501.
48	100	2.3	6	-617.27	7004	DZ	0.1134	7026		2	7005	DY	12661.47	-26089.9	98524.4
49	100	2.35	00	617.26	7004		0.1134	7026		2	7005		12816.58	26463.1	98547.3
50	100	2.4	- 7	617.25	7004		0.1134	7026		2			12971.66		98570.3
<u>51</u> 52	100	2.45 2.5	14 30	-617.24 -617.23	7004		0.1134	7026		2			13126.75		98593.3 98616.3
52	100	2.55	39	-617.23	7004		0.1134	7026		2	7005		13436.94		
54	100	2.6	39	-617.21	7004		0.11339	7026		2			13592.04		
55	100	2.65	39	-617.2	7004		0.11339	7026		2			13747.13	-28701.7	
56	100	2.7	39	-617.2	7004		0.11339	7026	RY	2	7005	DY	13902.27	-29074.8	98708.3
57	100	2.75	39	-617.2	7004		0.11339	7026		2			14057.38		98731.4
58	100	2.8	39 39	-617.27	7004		0.1134	7026		2	7005		14212.93		
59 60	100 100	2.85 2.9	39	-617.3 -617.3	7004		0.11341	7026		2			14368.05	-30194.2 -30567.1	98777.32
61	100	2.95	39	-617.33	7004		0.11341	7026		2			14678.45		98823.5
62	100	3	39	-618.39	7004		0.11351	7026		2			14835.23		
63	100	3.05	39	619.46	7004	DZ	0.11361	7026	RY	2	7005	DY	14991.19	-31645.6	98891.9
64	100	3.1	39	-619.78	7004		0.11365	7028		2			15146.75		
65	100	3.15	39		7004		0.11386	7026		2			15301.9		
66 67	100	3.2 3.25	39 39	-620.29	7004		0.11371	7026		2	7005		15458.07	-32756.8 -33127.8	
68	100	3.25	39	-620.16 -620.53	7004		0.11368	7026		2			15765.93		99991.4
69	100	3.35	39	-620.53	7004		0.11378	7026		2			15918.45		99049.96
70	100	3.4	39	-622.72	7004		0.11393	7026		2	7005		16071.69		99085.17
71	100	3.45	39	-628.3	7004		0.12548	437		2			16216.11		99125.05
72	100	3.5	20	-654,77	7004	DZ	0.12629	138	RZ	2	7005	DY	16352.7	-34788	99168.1

								LUTION							
	LOAD	LOAD	NO.	And the owner of the owner	M DEFL		_	AUM ROT			UTION			TION SUM	
INCR	CASE	FACTOR	LOOPS	DEFL. CM	JOINT	DOF	ROT.	JOINT	DOF	MAX. DIGITS	JOINT	DOF	FX KN	FY KN	FZ KN
	CLPS	0.5	27	-369.05	7004	DZ	0.06899	7020	RY	2	7005	DY	3059.61	-256,49	48384,7
	CLPS	1	12	-618.21	7004		0.11347	7026		2	7005		6104.01	-511.79	96771
3		1.08	3	-617.73 -617.69	7004		0.11342	7026		2			9802.57	-1022.56	96721.6 96717.6
5		1.16		-617.66	7004		0.11341	7026		2			10394.35	-1104.29	98713.7
6	100	1.24	1	617.62	7004		0.11341	7026	RY	2	7005		10690.23	-1145.15	
7	100	1.32	1	-617.58	7004		0.11341	7026		2			10986.12	-1188.01	96705
8	100	1.4	1	-617.54 -617.51	7004		0.1134	7026		2			11282	-1228.87	96701.
10	100	1.56		-617.47	7004		0.11339	7026		2	7005		11873.77	-1308.6	96693.
11	100	1.64	1	-617.43	7004	DZ	0.11339	7026		2	7005	DY	12189.65	-1349.46	96690.
12	100	1.72	1	-617.39	7004		0.11339	7026		2	7005		12465.54	-1390.32	96686.
13	100	1.8 1.88	2	-617.35 -617.32	7004		0.11338	7026		2	7005		12761.41	-1431.18 -1472.04	96682. 96678.
15	100	1.96	1		7004		0.11338	7026		2	7005		13353.19	-1512.9	96674.
16	100	2.04	1	-617.24	7004		0.11337	7026		2	7005		13649.07	-1553.77	96670.
17 18	100	2.12	1	-617.2 -617.17	7004	and the second second	0.11337	7026		2	7005		13944.95 14240.84	-1594.63	96666. 96662.
19	100	2.28	1		7004		0.11336	7028		2			14536.72	-1676.35	96658.
20	100	2.36	1	-617.09	7004	DZ	0.11336	7026	RY	2	7005	DY	14832.6	-1717.21	96655.
21	100	2.44	1	-617.05	7004		0.11335	7026		2	7005		15128.49	-1758.07	96651.
22 23	100	2,52	1	-617.02	7004		0.11335	7026		2	7005		15424.37	-1798.93	96647.
23	100	2.6 2.68	1	-616.98 -616.94	7004		0.11335	7026		2	7005		16016.13	1880.66	96639.
25	100	2.76	1	-616.9	7004		0.11334	7026	RY	2	7005	DY	16312.02	1921.52	96635.
26	100	2.84	1	-616.87	7004		0.11333	7026		2	7005		16607.9	-1962.38	96632.
27 28	100	2.92	1	-616.83 -616.79	7004		0.11333	7026		2	7005		16903.78	-2003.24	96628 96624.
28	100	3.08	1	-616.79 -616.76	7004		0.11333	7026		2	7005		17199.66	-2044.1	96624. 96620.
30	100	3.16	1	-018.72	7004		0.11332	7026		2	7005		17791.43	-2125.82	96616
31	100	3.24	1	-616.68	7004		0.11332	7026		2			18087.31	-2168.68	96613.
<u>32</u> 33	100	3.32	1	-616.64 -616.61	7004		0.11331	7026		2	7005		18383.19	2207.54	96609. 96605.
33	100	3.48	1	-616.57	7004		0.1133	7026		2			18974.96	-2289.27	96601.
35	100	3.56	1	-816.53	7004	DZ	0.1133	7028		2	7005		19270.84	-2330.13	96597.
36	100	3.64	1	-616.49	7004		0.1133	7026	·	2	7005		19566.72	-2370.99	96594.
37 38	100	3.72 3.8	1	-616.46 -616.42	7004		0.11329	7026		2	7005		19862.6 20158.48	-2411.85 -2452.71	96590. 96586.
39	100	3.88	1	-616.38	7004		0.11329	7026		2	7005		20454.36	-2493.57	96582.
40	100	3.96	1	616.34	7004	DZ	0,11328	7020	RY	2	7005	DY	20750.24	2534.43	96579.
41	100	4.04	1	-616.31	7004		0.11328	7020		2	7005		21048,13	-2575.29	96575.
42 43	100	4.12	1	-616.27 -616.23	7004		0.11327	7020		2	7005		21342.01 21637.88	-2616.15	96568
44	100	4.28	4	-816.19	7004		0.11327	7020		2		DY	21933.76	-2697.87	96564
45	100	4.36	1	-616,16	7004	DZ	0.11326	7020		2	7005		22229.65	-2738.74	96560.
46 47	100	4.44	1	-616.12 -616.09	7004 7004		0.11326	7020 7020		2	7005	_	22525.53 22821.41	-2779.6	96557. 96553.
48	100	4.6		-018.05	7004		0.11325	7020		2	7005	the second s	23117.29	2861.32	96549.
49	100	4.68	1	-816.01	7004		0.11325	7020		2	7005		23413.17	-2902.18	96546.
50	100	4.78	1	-615.97	7004		0.11324	7020		2		DY	23709.05	-2943.04	96542.
51 52	100	4.84 4.92	1		7004		0.11324	7020		2			24004.93 24300.81		
53	100	4.92	1		7004		0.11323	7020		2			24596.69	-3024.76	96531.
54	100	5.08	1	-615.83	7004	DZ	0.11323	7020	RY	2	7005	DY	24892.57	-3106.48	96527
55	100	5.16	1		7004		0.11323	7020		2				-3147,34	
<u>56</u> 57	100	5.24 5.32	1	-615.75 -615.72	7004		0.11322	7020		2	7005		25484.33	-3188.2 -3229.06	
58	100	5.4	1		7004		0.11321	7020		2			26076.09		
59	100	5.48	1	-615.65	7004	DZ	0.11321	7020	RY	2	7005	DY	26371.97	-3310.7B	96509.
60 61	100	5.58 5.64	1	-615.61 -615.57	7004		0.11321	7020		2			26963.72	-3351.64	
62	100	5.72	2	-615.54	7004		0.1132	7020		2			27259.58		
63	100	5.8	2	-615.5	7004	DZ	0.1132	7020	RY	2	7005	DY	27555.45	-3474.22	96495
64	100	5.88	2		7004		0.11319	7020		2			27851.32	-3515.07	
65 66	100	5.96 6.04	2	-615.43 -615.4	7004		0.11319	7020 7020		2	7005		28147.19 28443.06	-3555.93 -3596.78	
67	100	8.12	2	-615.37	7004		0.11318	7020		2	7005	DY	28738.92	-3637.64	9648
68	100	6.2	4	-615.33	7004	DZ	0.11318	7020	RY	2	7005	DY	29034.78	-3678.49	96477.
69	100	6.28	4	615.3	7004		0.11318	7020		2			29330.63		
70 71	<u>100</u>	6.36 6.44	2	-615.27 -615.24	7004		0.11317 0.11317	7020		2	7005		29628.49 29922.33	-3760.19 -3801.04	
72	100	6.52	4	-815.21	7004		0.11317	7020	RY	2	7005	DY	30218.16	-3841.89	96463
73	100	6.6	4	-615.19	7004	DZ	0.11316	7020		2	7005		30513.98	-3882.73	
74	100	6.68	6	-615.16	7004		0.11316	7020		2	7005		30809.8	-3923.56 -3964.4	96456
75 76	<u>100</u> 100	6.76 6.84	4	-615.13 -615.11	7004		0.11316	7020		2			31105.6 31401.38		
- 10	100	6.92	6		7004		0.11315	7020		2			31697.14	-4046.05	

						COLL	APSE SC	LUTION	SUMMAR	Y					
	LOAD	LOAD	NO.	MAXIMU	IM DEFLE	CTION	MAXIN	IUM ROT	ATION	SOL	UTION	DATA	REACT	ION SUMM	IATION
INCR	CASE	FACTOR	LOOPS	DEFL	JOINT	DOF	ROT.	JOINT	DOF	MAX.	JOINT	DOF	FX	FY	FZ
				СМ						DIGITS			KN	KN	KN
78	100	7	6	-615.07	7004	DZ	0.11315	7020	RY	2	7005	ĎΥ	31992.89	-4086.86	96443.3
79	100	7.08	6	-615.06	7004	DZ	0.11315	7020	RY	2	7005	DY	32288.61	-4127.68	96439.96
80	100	7.16	8	-615.05	7004	DZ	0.11315	7020	RY	2	7005		32584.3	-4168.46	96436.64
81	100	7.24	13	-615.05	7004	DZ	0.11315	7020	RY	2	7005	DY	32879.94	-4209.24	96433.37
82	100	7.32	13	-615.05	7004		0.11315	7020	RY	2	7005	DY	33175.52	-4249.99	96430.15
63	100	7.4	15	-615.07	7004	DZ	0.11315	7020	RY	2	7005	DY	33471.01	-4290.69	98428.98
84	100	1.1.4	17	-615.09	7004		0.11316	7020		2	7005		33766.44	-4331.4	96423.87
85	100	7.56	19	615.12	7004		0.11316	7020		2	7005		34061.79		96420.82
66	100	7.64	21	-615.19	7004		0.11317	7020		2	7005		34358.89		96417.92
87	100	7.72	21	-615.3			0.11318			2			34651.59	-4453.15	96415.26
88	100		28	-615.57	7004		0.11321	7020		2	7005		34945.13		96413.22
89	100	7.88	39	-616.95			0.11335	7020		2	7005		35229.13	-4531.78	96416.06
90	100	7.96	39	-639.02	7004	DZ	0.11565	7026	RY	2	7005	DY	35404.15	-4587.45	96463.98
91	100	8.04	39	-723.34	7005	DZ	-0.1434	202	RY	2	7005	DŶ	35237.67	-4589.9	96669.78
92	100	8.12	4	-753.19	7004	DZ	-0.1682	293	RY	2	7005	DY	35333.69	-4696.02	95110.97

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[						COLL	APSE SC	LUTION	SUMMAR	Y					
	LOAD	LQAD	NO,	MAXIMUM DEFLECTION			MAXIMUM ROTATION			SOLUTION DATA			REACTION SUMMATION		
INCR	CASE	FACTOR	LOOPS	DEFL.	JOINT	DOF	ROT.	JOINT	DOF	MAX.	JOINT	DOF	FX	FY	FZ
				CM						DIGITS			KN	KN	KN
													i		
1	CLPS	0.5	27	-369.16	7004	DZ	0.06904	7020	RY	2	7005	DY	1969.56	2762.61	48563.78
2	CLPS	1	32	-611.63	7004	DZ	0.49623	5833	RZ	2	7005	DY	3918.12	5569.23	97127.23

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		1045	10		IN DOCT				SUMMAR		117101	DATA	REACTION SUMMATION			
INCR	LOAD	LOAD	NO.	DEFL.	JM DEFLI JOINT	DOF	ROT.	JOINT	ATION DOF	MAX.	JOINT		REAC	TION SUMM	ATION FZ	
	UNUL		20013	CM			NOI:	50111		DIGITS	30111	001	KN .	KN	KN	
	CLPS	0.5	28	-394.24	7004		0.07367	7020		2	7005		-96.96		49037.0	
	CLPS 100	0.07	11	-658.52 -658.52	7004		0.12078	7020		2	7005		-198.01	8225.14	98074.7	
	100	0.07	2	-658.52	7004		0.12078	7020		2	7005		-208.63	6558.66		
5	100	0.21	2	-858.52	7004		0.12078	7020		2	7005	The second second second	-229.87	7225.72	98131.3	
6	100	0.28	2	-858.52	7004		0.12078	7020		2	7005	the second s	-240,5	7559.24	98150.2	
7	100	0,35	2	-658.52	7004		0.12078	7020		2	7005		-251.12	7892.77		
8	100	0.42	2	-658.52 -658.52	7004		0.12078	7020		2	7005		-261.74	8226.3		
10	100	0.49	2	-658.52	7004		0.12078	7020		2	7005		-272.36	8893.35	98207.0	
11	100	0.83	2	-658.53	7004		0.12078	7020		2	7005		-293.61	9226.88	98244.9	
12	100	0.7	2	-658.53	7004		0.12078	7020		2	7005		-304.23	9560.41	98263.9	
13	100	0.77	2	-658.53	7004		0.12078	7020		2			-314.88	9893.94		
14 15	100	0.84	2	-658.53 -658.53	7004		0.12078	7020	and the second	2	7005		-325.48	10227.46	98301.8	
16	100	0.98	2	-858.53	7004		0.12078	7020		2	7005	-	-346.72	10894.52	98339.8	
17	100	1.05	2	-658.54	7004	The second s	0.12078	7020		2	7005		-357.35	11228.05	98358,8	
18	100	1.12	2	658.54	7004		0.12078	7020		2	7005	·	-367.97	11561.57	98377.8	
19	100	1.19	2	-658.54	7004		0.12078	7020		2	7005		-378.59	11895.1	98396.8	
20	100	1.26	2	-658.54 -658.54	7004		0.12078	7020 7020		2	7005		-389.22	12228.63	98415.8 98434.8	
21	100	1.33	2	-658.54	7004		0.12078	7020		2	7005		-399.84	12562.16		
23	100	1.47	2	-658.55	7004		0.12078	7020		2	7005		-421.09	13229.21	98472.9	
24	100	1.54	2	-658.55	7004	DZ	0.12078	7020	RY	2	7005	DY	-431.71	13502.74	98492.0	
25	100	1.61	2	-658.55	7004		0.12078	7020			7005		-442.33	13898.27		
26 27	100 100	1.68	2	-658.55 -658.56	7004		0.12078	7020		2	7005	_	-452.95	14229.8 14563.33	98530.1	
27	100	1.82	2	-658.56	7004		0.12078	7020		2	7005		-474.2	14896.85	98568.	
29	100	1.89	2	-658.56	7004		0.12078	7020		2	7005		-484.82	15230.38	98587.3	
30	100	1.96	2	-658.57	7004	DZ	0.12078	7020	RY	2	7005	DY <sup>2</sup>	-495.45	15563.91	98606.4	
31	100	2.03	2	-658.57	7004		0.12078	7020		2	7005		-506.07	15897.44	98625.	
32	100	2.1	2	-858.57	7004		0.12078	7020	the second s	2	7005		-516.69	16230.96	98644.7	
33	100	2.17 2.24	2	-658.57 -658.58	7004		0.12078	7020		2	7005		-527.32	16564,49	98663.8	
35	100	2.31	2	-658.58	7004		0.12078	7020		2	7005		-548.56	17231.55	96702.1	
36	100	2.38	2	-658.58	7004	DZ	0.12078	7020	RY	2	7005	DY	-559.19	17565.08	98721.2	
37	100	2.45	2	-658.59	7004		0.12078	7020		2	7005		-569.81	17898.61		
<u>38</u> 39	100	2.52	- 2	-658.59 -658.6	7004	State of the local division of the local div	0.12078	7020		2	7005		-580.43	18232.13	98759.5 98778.7	
40	100	2.86	2	-658.6	7004		0.12078	7020		2	7005	and the second se	-601.68	18899.19		
41	100	2.73	2	-658.6	7004		0.12078	7020		2	7005		-612.31	19232.72	98817.1	
42	100	2.8	2	-658.61	7004		0.12078	7020		2	7005		-622.93	19568.25	98836.3	
43	100	2.87	2	-658.61	7004		0.12078	7020	The second s	2	7005		-633.55	19899.77		
44	100	2.94	2	-658.61 -658.62	7004		0.12078	7020		2	_ <u>7005</u> 7005		-644.18 -654.8	20233.3		
40	100	3.08	2	-658.62	7004		0.12078	7020		2	7005		-865.42	20566.83	98893.9 98913.1	
47	100	3.15	2	-658.62	7004		0.12078	7020	the second s	2	7005	the second s	-676.05	21233.89	98932.4	
48	100	3.22	- 2	-658.63	7004	DZ	0.12078	7020		2	7005		-688.67	21567.41	98951.8	
49	100	3.29	2	-658.63	7004		0.12078	7020		2	7005		-697.29	21900.94	98970.	
50	100	3.36	2	-658.64	7004		0.12078	7020	The state	2	7005		-707.92 719 64		98990.1	
51 52	100	3,43		-658.64	7004		0.12078	7020		2	7005			22567.99 22901.52		
53	100	3,57	2	-858.65	7004	DZ	0.12078	7020		2	7005	DY		23235.04	99047.9	
54	100	3.64	2	-858.66	7004		0.12078	7020		2	7005		-750.41			
55	100	3.71	2		7004		0.12078	7020		2	7005			23902.09		
56 57	100	3.78 3.85	2	-658.67 -658.67	7004		0.12078	7020		2	7005			24235.61 24569.13		
58	100	3.92	2	-658.68	7004		0.12078	7020		2	7005			24902.65		
59	100	3.99	2	-658.68	7004	DZ	0.12078	7020		2	7005			25236.17		
60	100	4.06	2	-658.69	7004		0.12078	7020		2	7005			25569.68		
61	100	4.13	2	658.69	7004		0.12078	7020		2	7005			25903.19		
62 63	100	4.2	4	-658.7 -658.71	7004	DZ	0.12078	7020		2	7005			26236.7 26570.21		
64	100	4,34	3	-658.71	7004		0.12078	7020	RY	2	7005	DY	-858.63	26903.71	99260.6	
65	100	4.41	3		7004	DZ	0.12078	7020		2	7005		-867.25	27237.22	99280.0	
66	100	4.48	3		7004		0.12078	7020			7005			27570.71		
67 68	100	4.55	3	-658.73 -658.74	7004		0.12078	7020		2	7005			27904.21 28237.69		
69	100	4.62		-658.74			0.12078	7020		2	7005			28571.17		
70	100	4.76	4	-658.75	7004	DZ	0.12078	7020	RY	2	7005	DY	-920.34	28904.63	99377.1	
71	100	4.83	6	-658.76	7004	DZ	0.12078	7020		2	7005			29238.08		
72	100	4.9	6	-658.77	7004		0.12078	7020		2	7005			29571.52		
73 74	100	4.97 5.04	7	-658.78 -658.79	7004		0.12078	7020		2	7005			29904.93 30238.31		
75	100	5.11		-658.8	7004		0.12078	7020		2	7005			30230.3		
76	100	5.18	39	-659.34	7004		0.12084	7020		2	7005			30894.9		
77	100	5.25	39	-859.93	7005	DZ	0.1209	7020	RY	2	7005	DY	-991.73	31221.18	99519.	
78	100	5.32	39		7005		0.12098	7020		2	7005			31542.29		
79	100	5.39	32	-701.59	7005	DZ	1.60073	5748	RX	2	7005	DY	-1014.27	31952.65	99510.	

COLLAPSE SOLUTION SUMMARY													REACTION SUMMATION				
	LOAD	LOAD	NO.					MUM ROTATION									
INCR	CASE	FACTOR	LOOPS	DEFL. CM	JOINT	DOF	ROT.	JOINT	DOF	MAX, DIGITS	JOINT	DOF	FX KN	FY KN	FZ KN		
	CLPS	0.5	27	-369.16	7004	DZ	0.06902	7020	RY	2	7005	DY	301.35	2762.39	48590.1		
2	CLPS	1	12	-618,4	7004		0.11352	7020		2			601.05		97180.8		
3		0.07	2	-618.42	7004		0.11353	7020		2			473.55		97191.1		
4		0.14	2	-618.44	7004	_	0.11353	7020		2			348.05		97201.		
5		0.21	2	-618.45 -618.47	7004		0.11353	7020		2	7005		218.54		97211 97221		
7		0.28	2	-618.49	7004		0.11353	7020		2			-36,46	6943.32			
8		0.42	2	-618.51	7004		0.11353	7020		2			-163.96	7227.87	97242.		
9	100	0.49	2	-618.53	7004	122	0.11354	7020		2			-291.47	7512.01			
10	100	0.56	2	-618.54	7004		0.11354	7020		2			-418.97	7796.38			
11	100	0.63	2	-618.56	7004		0.11354	7020		2			-546.47	8080.71			
12	100	0.7	4	-618.58 -618.59	7004		0.11354	7020		2			-673.97	8365.06			
13	100	0.77	2	-618.61	7004		0.11354	7020		2			-928.98	8933.75			
15	100	0.91	2	-618.63	7004		0.11354	7020		2	7005		-1056.48	9218.1			
16	100	0.98	2	-618.65	7004		0.11355	7020	the second s	2	7005	and the second se	-1183.98	9502.45			
17	100	1.05	2	-618.66	7004	DZ	0.11355	7020	RY	2	7005	DY	-1311.49	9786.8	9733-		
18	100	1.12	2	-618.68	7004		0.11355	7020		2			-1438.99	10071.15			
19	100	1.19	2	-618.7	7004		0.11355	7020		2			-1566.49	10355.5			
20	100	1.26	2	-618.72	7004		0.11355	7020		2			-1693.99	10639.84			
21	100	1.33	2	-618.74	7004		0.11355	7020		2			-1821.5	10924.19			
22 23	100	<u> </u>	2	-618.76 -618.77	7004		0.11356	7020		2	7005		-1949	11208.54			
23	100	1.47	2	-618.79	7004		0.11356	7020		2			-20/0.5	11777.24			
25	100	1.61	2	-618.81	7004		0.11356	7020		2	7005		-2331.51	12061.59			
26	100	1.68	2	-618.83	7004		0.11356	7020		2			-2459.01	12345.94			
27	100	1.75	4	-618.85	7004		0,11356	7020		2			-2586.51				
28	100	1.82	2	-618.86	7004	The second s	0.11356	7020		2			-2714.02	12914.63	97447		
29	100	1.89	2	-618.88	7004		0.11357	7020		2			-2841.52	13198.98			
30	100	1.96	2	-618.9	7004		0.11357	7020		2			-2969.02	13483.33			
31 32	100	2.03	2	-618.92 -618.94	7004		0.11357	7020		2	7005		-3096.53	13767.68			
33	100	2.1	2	-618.96	7004		0.11357	7020		2			-3224.03	14032.03			
34	100	2.24	2	-618,98	7004		0.11357	7020		2	7005		-3479.03	14620.73			
35	100	2.31	2	-619	7004		0.11358	7020		2			-3606.54	14905.08			
36	100	2.38	2	-619.02	7004		0.11358	7020		2			-3734.04	15189,42			
37	100	2.45	2	-619.04	7004	DZ	0.11358	7020	RY	2	7005	DY	-3861.54	15473.77	97541.		
38	100	2.52	2	-619.06	7004		0.11358	7020		2	7005		-3989.05	15758,12			
39	100	2.59	2	-619.08	7004		0.11358	7020		2			-4116.55	16042.47			
40 41	100 100	2.66 2.73	2	-619.1 -619.11	7004		0.11359	7020		2	7005		-4244.05 -4371.55	16326.82 16611.17	97572 97582.		
42	100	2.73	2	-619.13	7004		0.11359	7020		2			-4499.06	16895.52			
43	100	2.87	2	-619.15	7004		0.11359	7020		2	7005		-4626.56	17179.87			
- 44	100	2.94	2	619.17	7004		0.11359	7020		2			-4754.06	17464.22	9761		
45	100	3.01	2	-819.19	7004	DZ	0.11359	7020	RY	2	7005	DY	-4881.57	17748.57			
46	100	3.08	2	-619,21	7004		0.11359	7020		2			-5009.07	18032,92	97635.		
47	100	3.15	2	-619.23	7004		0.1136	7020		2	7005		-5136.57	18317.27	97645.		
48	100	3.22	2	-619.25	7004		0.1136	7020		2			-5264.08	18601.61	97656.		
49	100	3.29	2	-619.27	7004		0.1138	7020		2	7005	199 A 4	-5391.58	18885.98			
<u>50</u> 51	100	3.36	2	-619.29 -619.31	7004		0.1136			2	7005			19170.31 19454.66			
52	100	3.5		-619.33			0.11361	7020		2				19739.01			
53	100	3.57	2	-619.35	7004	DZ	0.11361	7020	RY	2	7005	DY	-5901.59	20023.36	97708.		
54	100	3.64	2	-619.37	7004		0.11361	7020		2				20307.71			
55	100	3.71	2	-619.39			0.11361	7020		2				20592.06			
56	100	3.78	2	-619.41	7004		0.11361	7020		2				20876,41			
57	100	3.85	4	-819.43 -619.45	7004		0.11361	7020		2				21160.76			
<u>58</u> 59	100	3.92 3.99	2	-619.45	7004		0.11362	7020		2			6666 61	21445.1 21729.45	97771		
60	100	4.06	2	-619.49	7004		0.11362	7020		2				22013.8			
61	100	4.13	2	-619.51	7004	DZ	0.11382	7020	RY	2	7005	DY	-6921.62	22298.15	97793.		
62	100	4.2	3	-819.53	7004		0.11362	7020		2				22582.49			
63	100	4.27	3		7004		0.11362	7020		2				22866.83			
64 65	100 100	4.34	3		7004		0.11363	7020		2	7005 7005			23151.18 23435.52			
60 68	100	4.41	5		7004		0.11363	7020		2				23135.52			
67	100	4.55	10	-619.64			0.11363	7020		2				24004.2			
68	100	4.62	39	-619.66	7004	OZ	0.11363	7020	RY	2	7005	DY	-7814.13	24288.54	97867		
69	100	4.69	39	-619.68	7004	DZ	0.11363	7020		2				24572.88			
70	100	4.76	39	-619.7	7004		0.11364	7020		2				24857.21			
71	100	4.83	39	-619.72	7004		0.11364	7020		2				25141.54			
72	100	4,9	2	-619.74			0.11364	7020		2			8324.14	25425.88	97909.		
73		4.97	2	-619.77	7004		0.11384	7020		2				25710.21			
74		5.04	2	-619.79			0.11364	7020		2				25994.54			
75		5.11 5.18	2	-619.81 -619.83	7004		0.11365	7020		2				26278.87 26563.19			
77			2	-619.85			0.11365				7005			26847.52			

				•		COLL	APSE SC	LUTION	SUMMAR	(Y					
	LOAD	LOAD	NO.	MAXIMU	JM DEFLE	CTION	MAXIN	IUM ROT	ATION	SOL	UTION	DATA	REACT	TION SUMM	ATION
INCR	CASE	FACTOR	LOOPS	DEFL.	JOINT	DOF	ROT.	JOINT	DOF	MAX.	JOINT	DOF	FX	FY	FZ
				СМ						DIGITS			KN	KN	KN
78	100	5.32	3	-619.88	7004	DZ	0.11365	7020	RY	2	7005	DY	-9089.14	27131.83	97974.14
79	100	5.39	14	-619.9	7004	DZ	0.11365	7020	RY	2	7005	DY	-9216.64	27418.14	97984.87
80	100	5.46	35	-619.92	7004	DZ	0.11365	7020	RY	2	7005	DY	-9344.14	27700.45	97995.6
81	100	5.53	27	-619.94	7004	DZ	-0.1173	3336	RX	2	7005	DY	-9471.64	27984.76	98006.34
82	100	5.6	10	-619.97	7004	DZ	-0.12	3336	RX	2	7005	DY	-9599.13	28269.06	98017.09
83	100	5.67	8	-619.99	7004	DZ	-0.1209	3336	RX	2	7005	DY	-9726.63	28553.35	98027.88
84	100	5.74	24	-620.01	7004	DZ	-0.1243	3336	RX	2	7005	DY	-9854.12	26837.58	98038.65
65	100	5.81	35	-620.04	7004	DŻ	-0.1286	3336	RX	2	7005	DY	-9981.6	29121.78	98049.46
86	100	5.88	13	-620.07	7004	DZ	-0.1306	3336	RX	2	7005	DY	-10109.1	29405.9	98060.31
87	100	5.95	39	-620.21	7004	DZ	-0.1345	3336	RX	2	7005	DY	~10236	29685.2	98073.1
88	100	6.02	39	-621.13	7005	DZ	-0.1411	3336	RX	2	7005	DY	-10361.2	29953.26	98087.32
89	100	6.09	20	-621.25	7005	DZ	0.86487	3336	RX	2	7005	DY	-10488.7	30236.42	98098.06

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TABULATE DATA OF BOTH METHOD

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A) PLATFORM A,B & C COLLAPSE DATA COMPARISON

Wave Direction (deg)	Lateral load for 100-year storm condition, kN	First member failure load Pmf, kN	Factor for first member failure	Coilapse Ioad, Pu, kN	Factor for collapse load	Deformation corresponding to Pmf, mm	Deformation corresponding to Pu, mm	Reserve strength ratio
0.00	2343.89	9230.77	3.94	9488.49	4.05	865.70	2343.67	1.03
42.11	2737.78	7844.51	2.87	9174.21	3.35	1241.46	1594.76	1.17
90.00	5958.80	12226.07	2.05	12974.80	2.18	3395.18	6037.65	1.06
137.89	3444.07	8140.04	2.36	9489.52	2.76	1962.68	2548.18	1.17
180.00	3616.53	10582.18	2.93	11465.45	3.17	1156.08	1428.93	1.08
222.11	3975.24	8396.79	2.11	10078.17	2.54	-1832.43	-2964.16	1.20
270.00	2043.84	10746.32	5.26	11690.44	5.72	-3632.69	-6066.90	1.09
317.89	2200.71	7944.94	3.61	9018.19	4.10	-2462.19	-3680.00	1.14

Table C.3.A.1: Platform A Collapse Solution Data

Table C.3.A.2: Platform C Collapse Solution Data

Wave Direction (deg)	Lateral load for 100-year storm condition, kN	First member failure load Pmf, kN	Factor for first member failure	Collapse Ioad, Pu, kN	Factor for collapse load	Deformation corresponding to Pmf, mm	Deformation corresponding to Pu, mm	Reserve strength ratio
0.00	3275.85	30781.52	9.40	31730.06	9.69	-6989.71	-6333.36	1.03
65.00	6837.27	33858.24	4.95	34839.44	5.10	-6992.43	8331.41	1.03
90.00	12459.22	32464.96	2.61	37317.46	3.00	-6601.16	-6747.33	1.15
115.00	8111.45	34632.22	4.27	38437.97	4.74	-6183.94	-6547.74	1.11
180.00	3744.34	32551.37	8.69	35644.39	9.52	-6150.58	<u>-7</u> 531.91	1.10
245.00	4352.87	6809.40	1.56	6809.40	1.56	-6116.25	-6116.25	1.00
270.00	4784.71	30910.52	6.46	31968.74	6.68	-6593.40	-7015.91	1.03
295.00	4470.12	26443.94	5.92	32003.96	7.16	-6197.23	-6212.48	1.21

Wave Direction (deg)	Lateral load for 100-year storm condition, kN	First member failure load Pmf, kN	Factor for first member failure	Collapse Ioad, Pu, kN	Factor for collapse ioad	Deformation corresponding to Pmf, mm	Deformation corresponding to Pu, mm	Reserve strength ratio
0.00	1729.45	3298.68	1.91	4136.79	2.39	472.70	5744.60	1.25
30.00	1664.24	4017.31	2.41	4331.39	2.60	464.57	1449.90	1.08
60.00	1768.69	3207.17	1.81	4020.10	2.27	392.50	10400.47	1.25
90.00	1712.93	2678.52	1.56	3956.14	2.31	367.36	11786.73	1.48
120.00	1769.47	3296.41	1.86	5142.96	2.91	400.69	4745.32	1.56
150.00	1665.23	3522.15	2.12	4260.86	2.56	-466.90	2955.75	1.21
180.00	1729.94	3299.57	1.91	3880.84	2.24	-476.45	-3907.27	1.18
210.00	1664.90	2775.55	1.67	4262.99	2.56	-304.81	-4882.29	1.54
240.00	1769.86	3208.95	1.81	3618.19	2.04	-392.59	-972.37	1.13
270.00	1715.55	4134.51	2.41	4491.81	2.62	-546.71	1074.09	1.09
300.00	1771.57	3300.51	1.86	3736.09	2.11	-398.44	43976.37	1.13
330.00	1665.84	2694.31	1.62	4287.16	2.57	316.03	6947.04	1.59

Table C.3.A.3: Platform B Collapse Solution Data

Note:

Factor for first member failure =  $\frac{First member failure load, Pmf}{Lateral load for 100 - year storm condition}$ 

Factor for collapse load =  $\frac{Collapse load, Pu}{Lateral load for 100 - year storm condition}$ 

Reservestrength ratio =  $\frac{Factor for collapse load}{Factor for first member failure}$ 

**B) BRACING CONFIGURATION** 

Wave Direction (deg)	Lateral load for 100-year storm condition, kN	First membe <del>r</del> failure load Pmf, kN	Factor for first member failure	Collapse load, Pu, kN	Factor for collapse load	Deformation corresponding to Pmf, mm	Deformation corresponding to Pu, mm	Reserve strength ratio
0.00	2343.89	9230.77	3.94	9488.49	4.05	865.70	2343.67	1.03
42.11	2737.78	7844.51	2.87	9174.21	3.35	1241.46	1594.76	1.17
90.00	5958.80	12226.07	2.05	12974.80	2.18	3395.18	6037.65	1.06
137.89	3444.07	8140.04	2.36	9489.52	2.76	1962.68	2548.18	1.17
180.00	3616.53	10582.18	2.93	11465.45	3.17	1156.08	1428.93	1.08
222.11	3975.24	8396.79	2.11	10078.17	2.54	-1832.43	-2964.16	1.20
270.00	2043.84	10746.32	5.26	11690.44	5.72	-3632.69	-6066.90	1.09
317.89	2200.71	7944.94	3.61	9018.19	4.10	-2462.19	-3680.00	1.14

Table C.3.B.1: Design Basis Configuration

Table C.3.B.2: X-bracing Collapse Solution Data

Wave Direction (deg)	Lateral load for 100-year storm condition, kN	First member failure load Pmf, kN	Factor for first member failure	Collapse load, Pu, kN	Factor for collapse load	Deformation corresponding to Pmf, mm	Deformation corresponding to Pu, mm	Reserve strength ratio
0.00	2488.41	10410.32	4.18	10630.12	4.27	915.02	1289.61	1.02
42.11	2902.09	8311.26	2.86	9144.72	3.15	1173.81	1598.01	1.10
90.00	6261.25	11604.26	1.85	15280.47	2.44	3100.37	4417.47	1.32
137.89	3654.16	8449.42	2.31	10042.01	2.75	1801.86	2542.70	1.19
180.00	3832.05	11397.21	2.97	11584.19	3.02	-1159.44	-1234.57	1.02
222.11	4210.38	8049.95	1.91	10862.32	2.58	-1651.55	-3275.69	1.35
270.00	2159.37	10816.69	5.01	12445.34	5.76	-3490.34	-5267.47	1.15
317.89	2334.09	7960.63	3.41	9885.51	4.24	-2356.80	-12339.85	1.24

Wave Direction (deg)	Lateral load for 100-year storm condition, kN	First member failure load Pmf, kN	Factor for first member failure	Collapse load, Pu, kN	Factor for collapse load	Deformation corresponding to Pmf, mm	Deformation corresponding to Pu, mm	Reserve strength ratio
0.00	2339.53	9215.76	3.94	9493.51	4.06	755.15	2260.15	1.03
42.11	2731.50	7826.34	2.87	9029.29	3.31	1280.09	1658.62	1.15
90.00	5943.88	10424.99	1.75	11221.38	1.89	3094.95	4881.69	1.08
137.89	3436.15	8296.00	2.41	9125.70	2.66	2000.4	2520.5	1.10
180.00	3607.46	10557.71	2.93	11097.71	3.08	-1186.26	-1249.66	1.05
222.11	3962.67	8568.37	2.16	10600.58	2.68	-1910.94	-4258.11	1.24
270.00	2037.12	9683.77	4.75	9751.93	4.79	-4116.2	-4866.89	1.01
317.89	2195.89	7929.77	3.61	9527.65	4.34	-2481.84	-4467.35	1.20

Table C.3.B.3: Single Diagonal Bracing Data

Note:

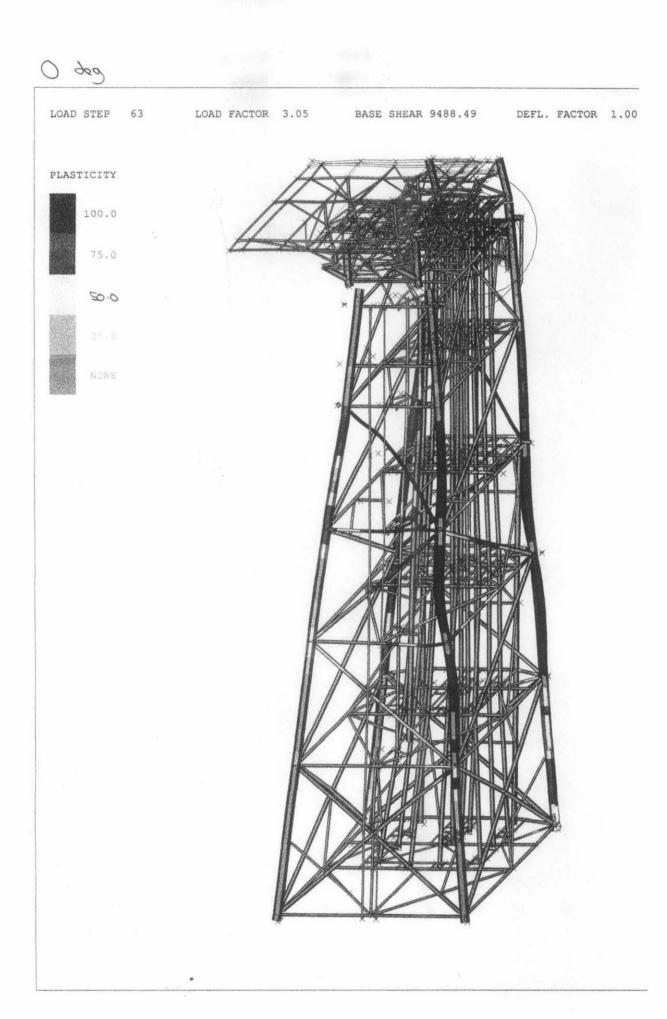
Factor for first member failure =  $\frac{First member failure load, Pmf}{Lateral load for 100 - year storm condition}$ 

Factor for collapse load =  $\frac{Collapse load, Pu}{Lateral load for 100 - year storm condition}$ 

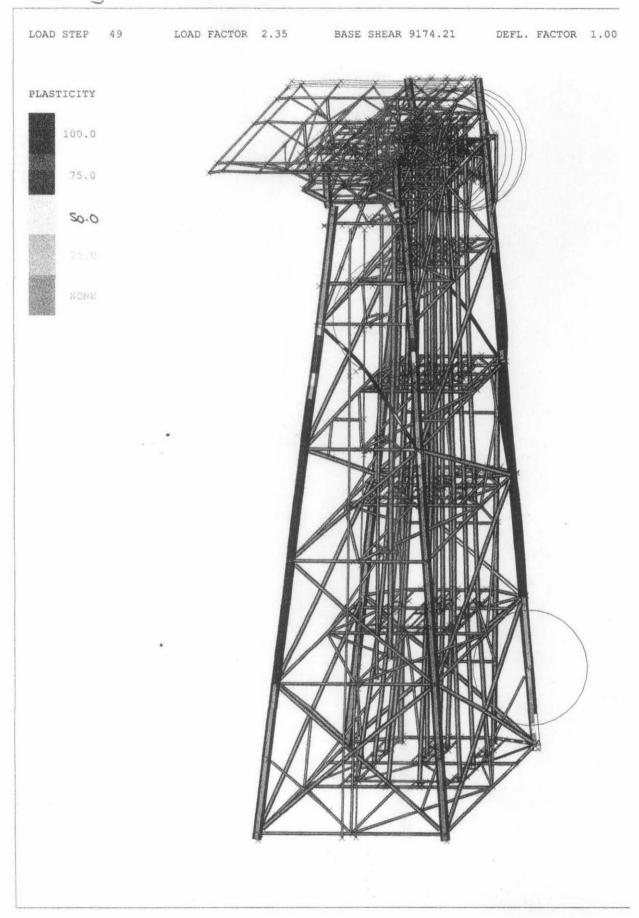
Reserve strength ratio =  $\frac{Factor for collapse load}{Factor for first member failure}$ 

COLLAPSE RESTART FILE

## APPENDIX C.4 A) PLATFORM A

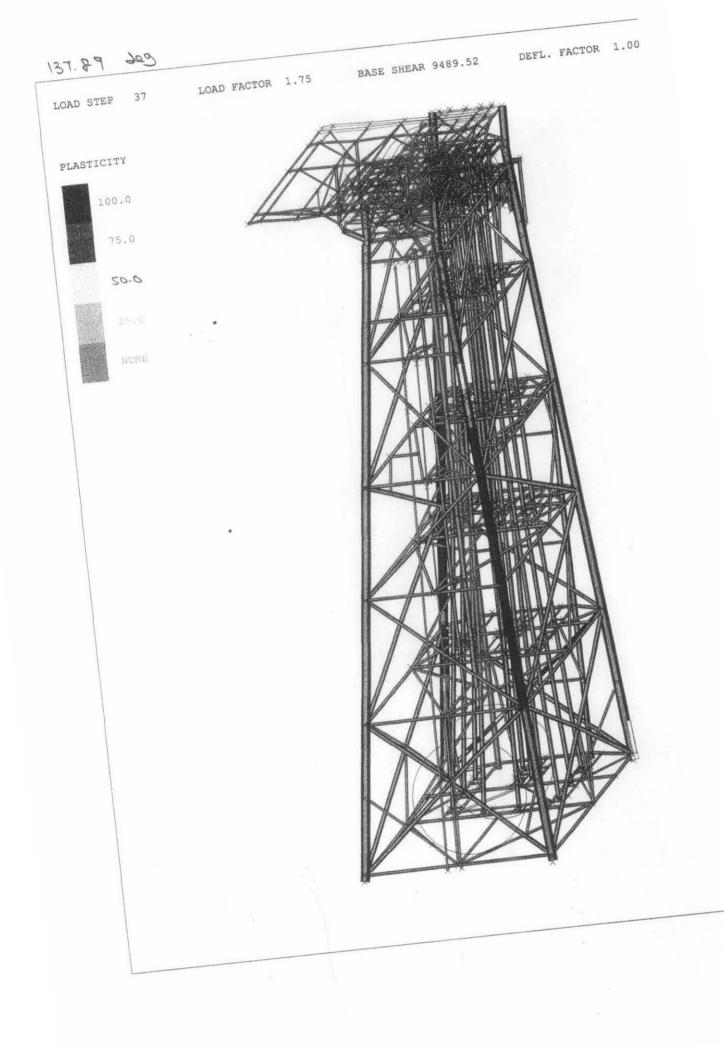


42.11 deg

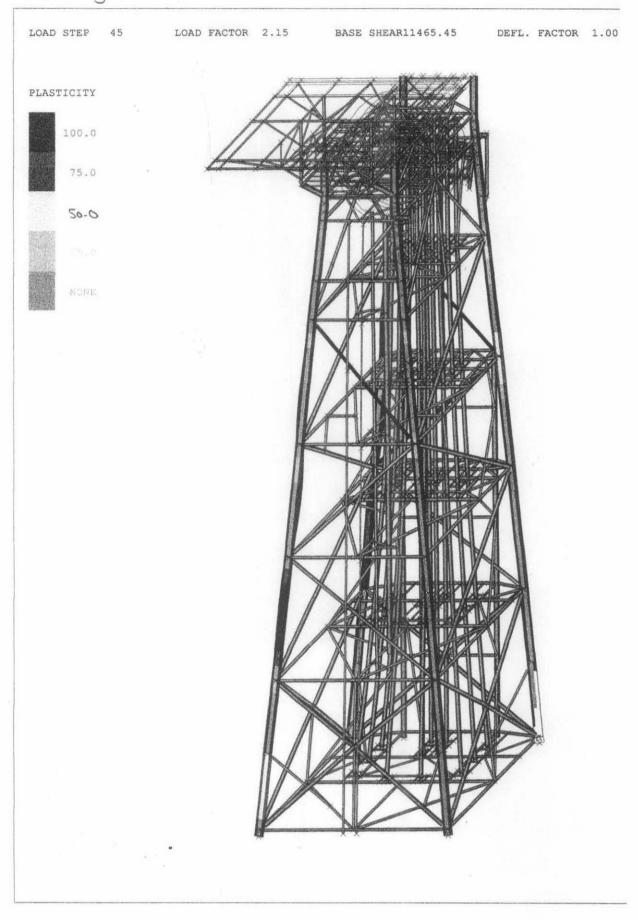


90 deg





180 deg





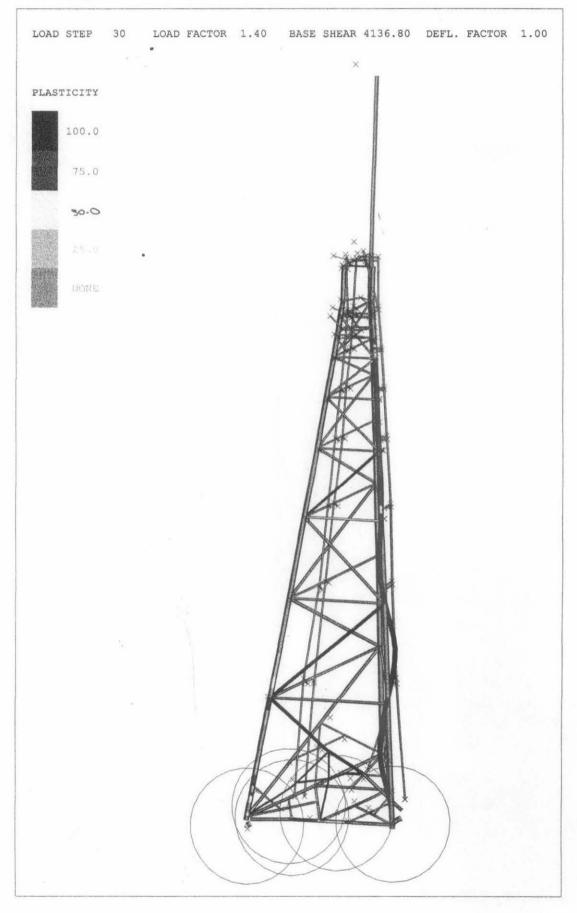
270 209



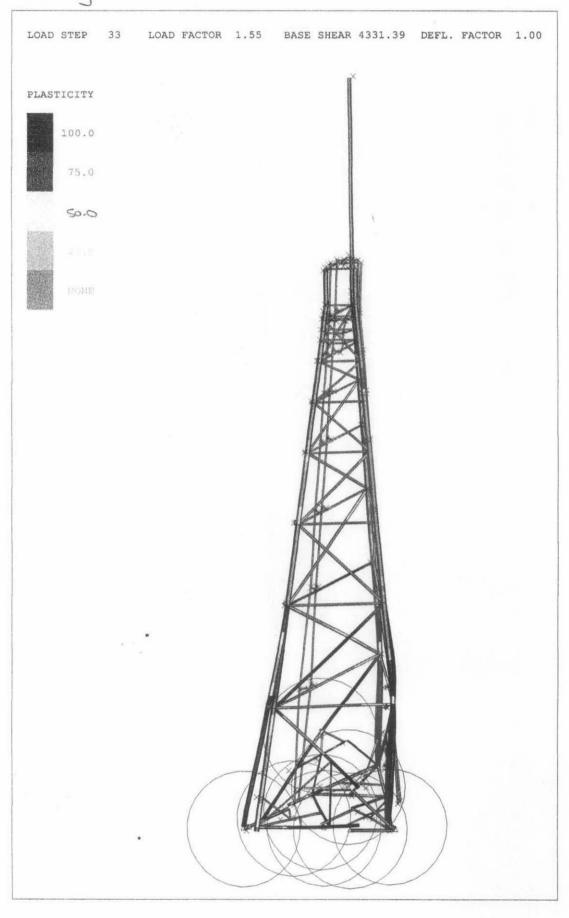


### B) PLATFORM B

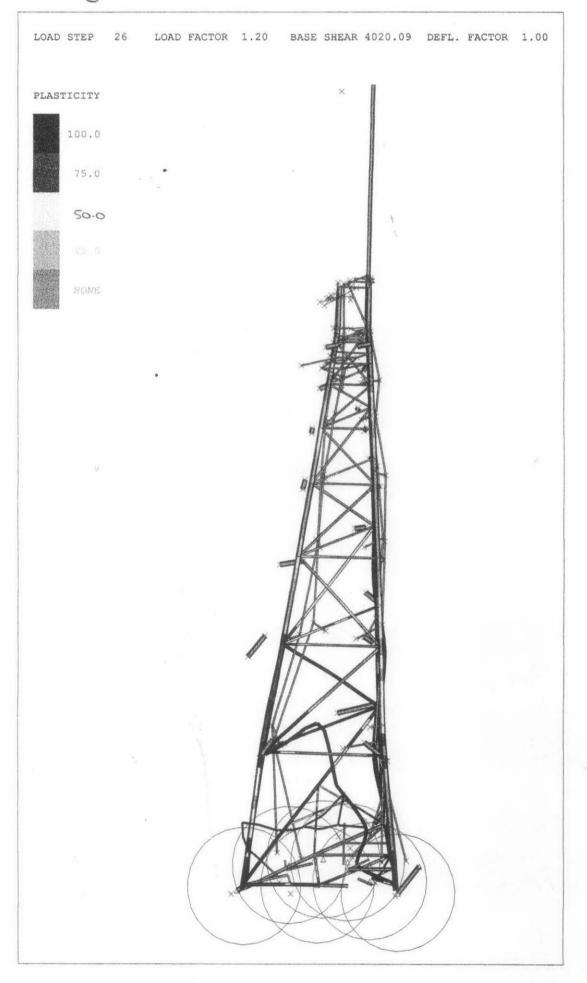
O deg



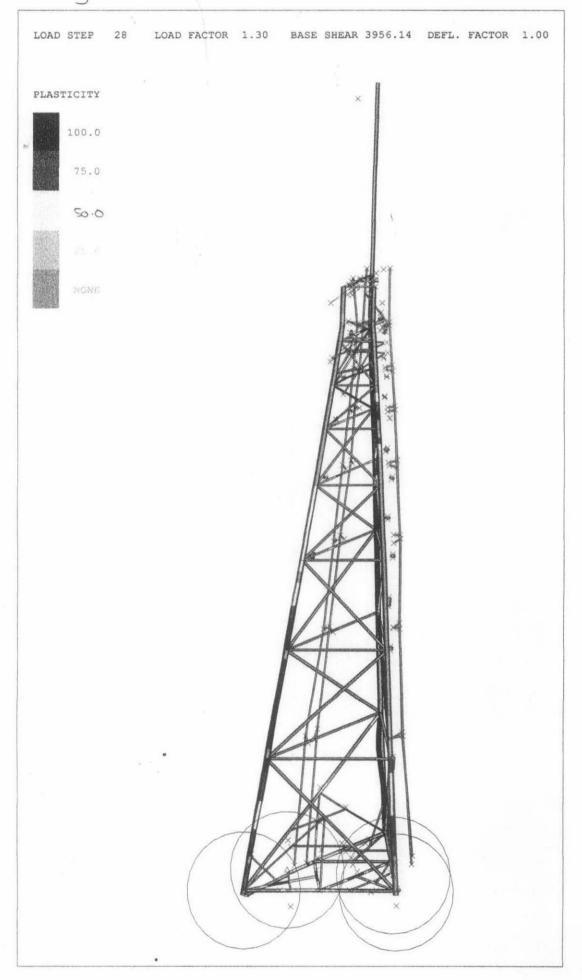
30 deg



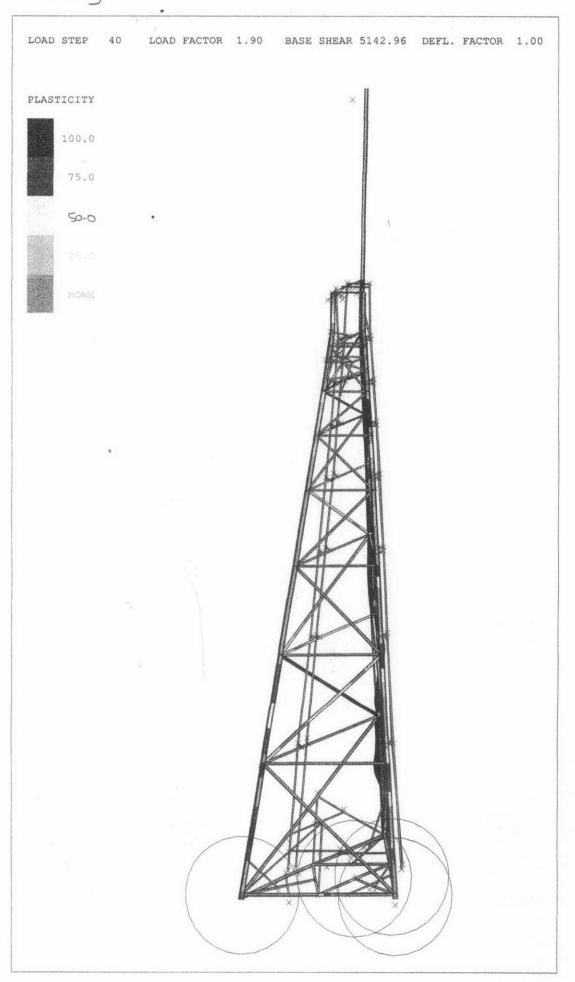
60 200

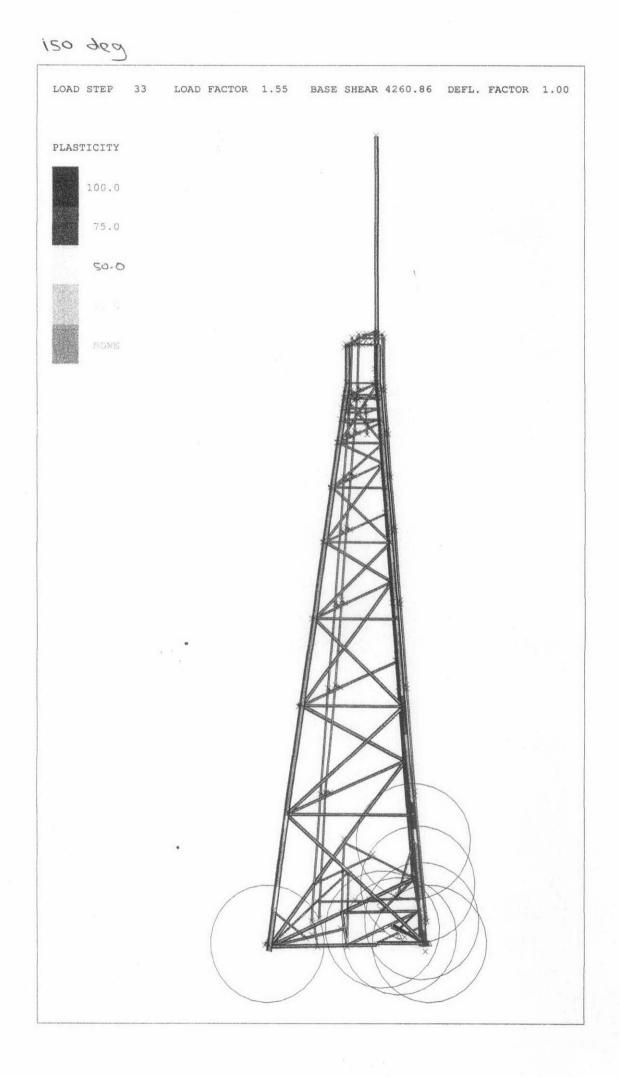


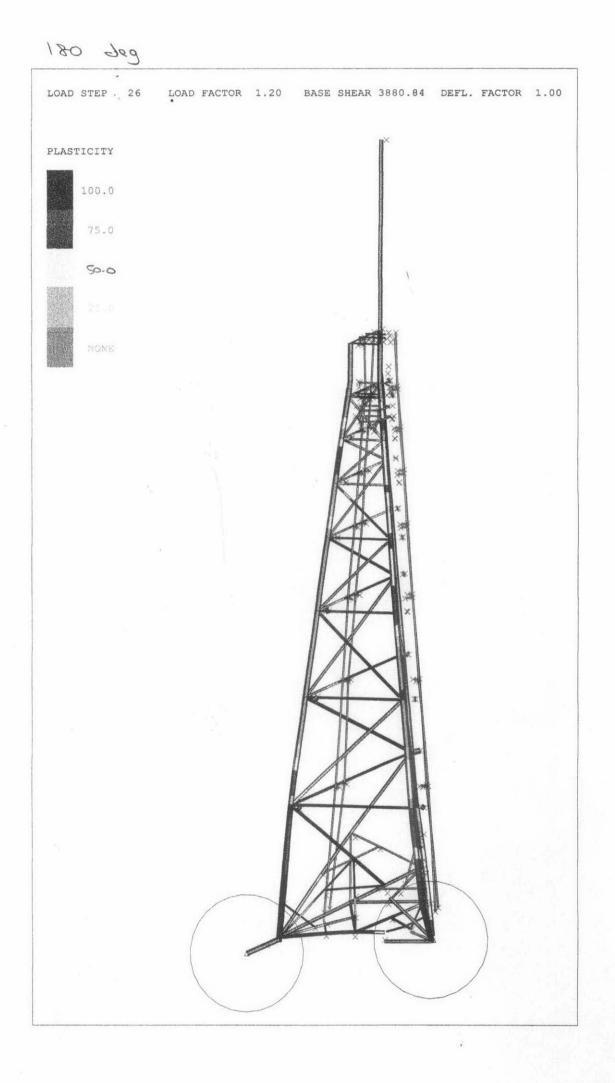
90 200



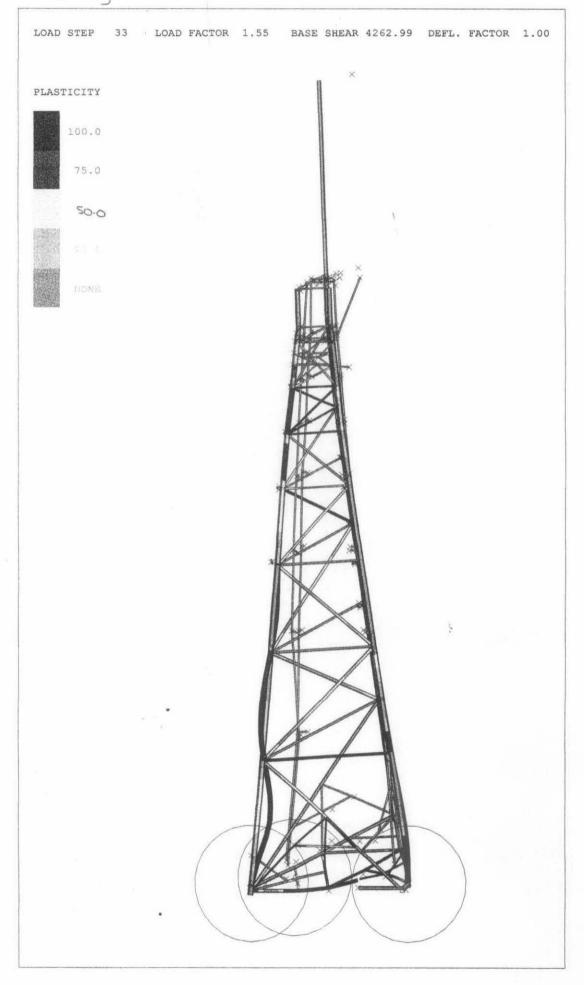
120 200



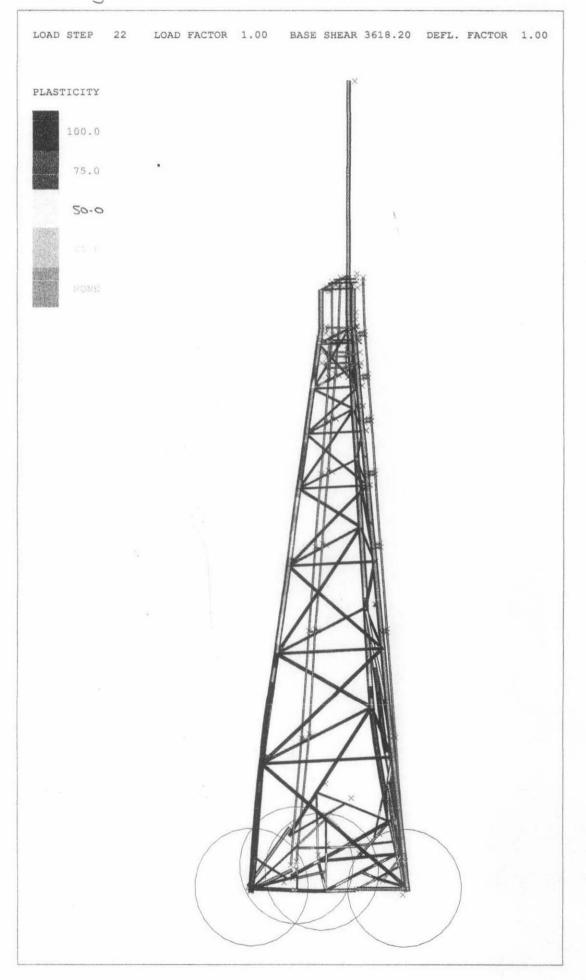




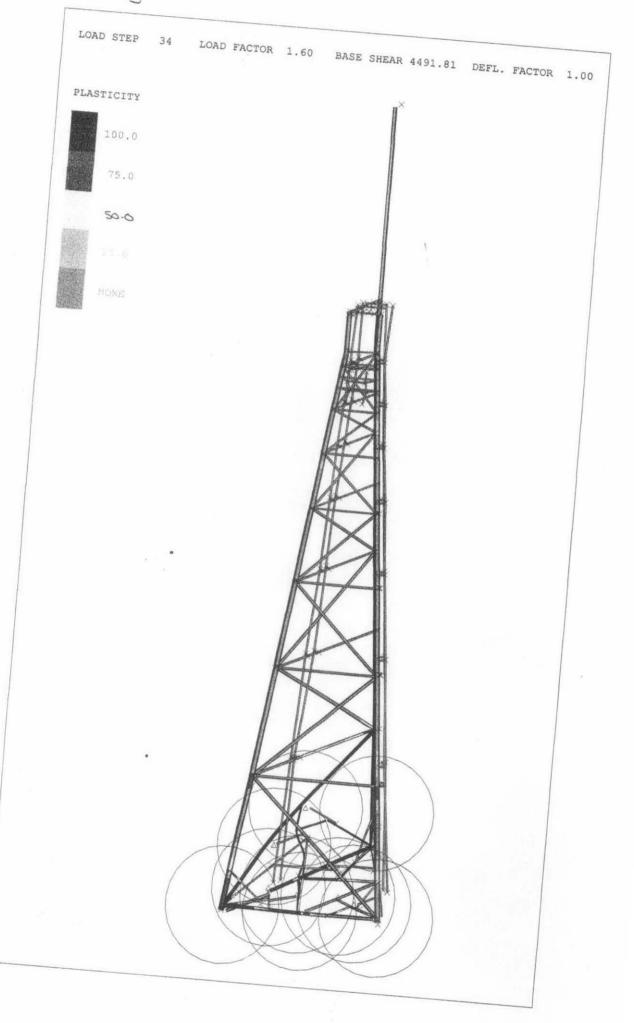
210 deg



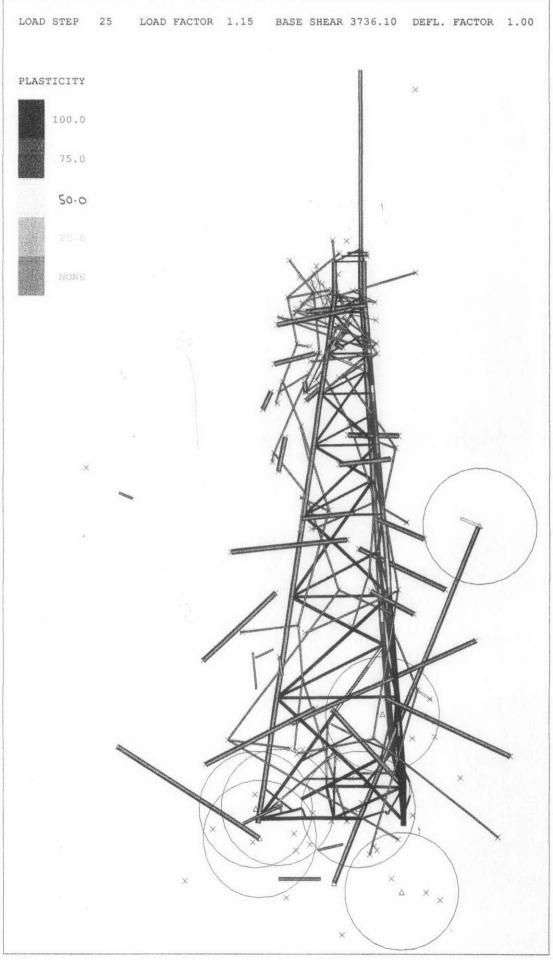
240 200

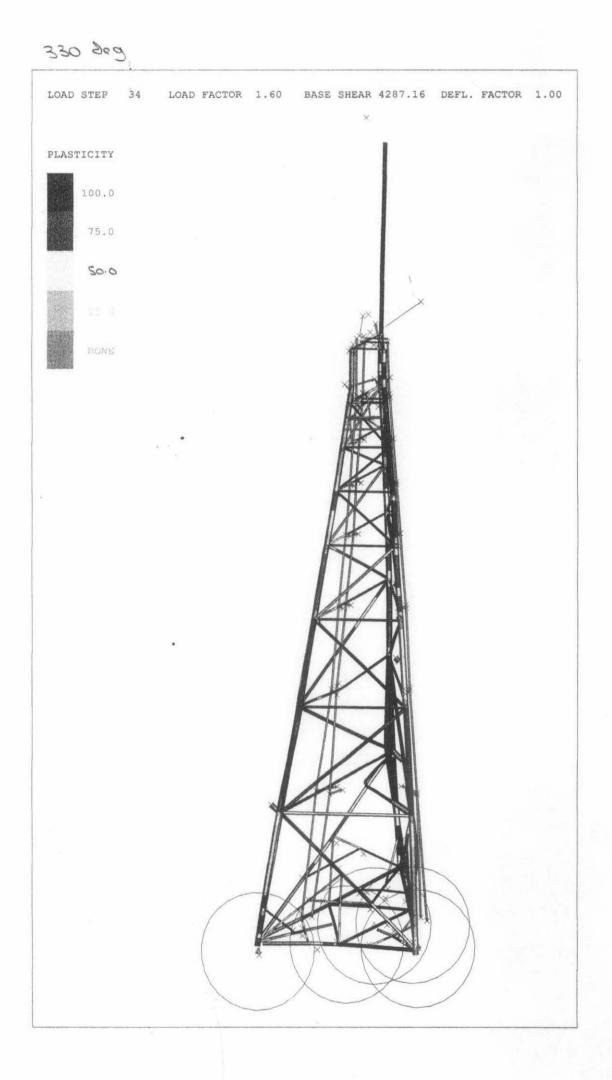


270 209



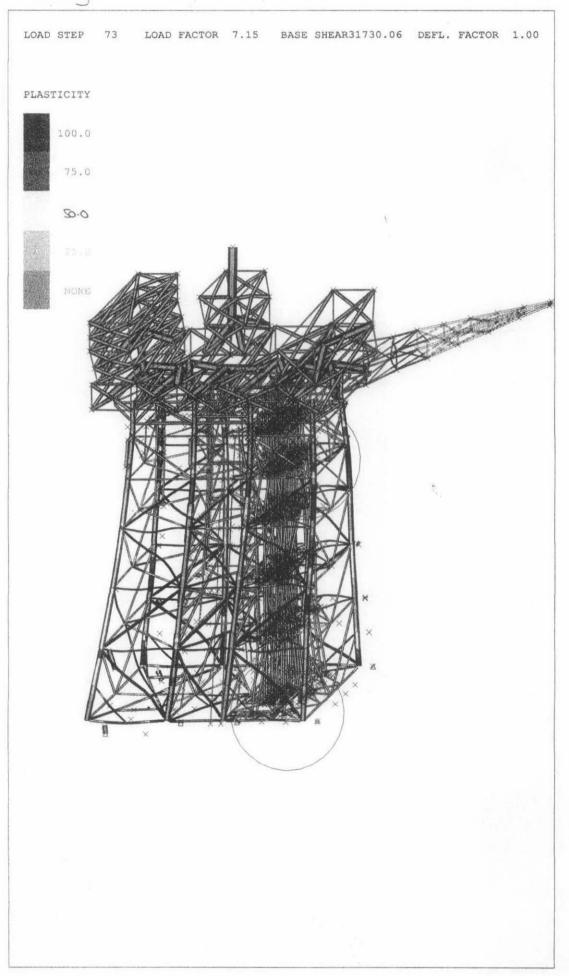
.



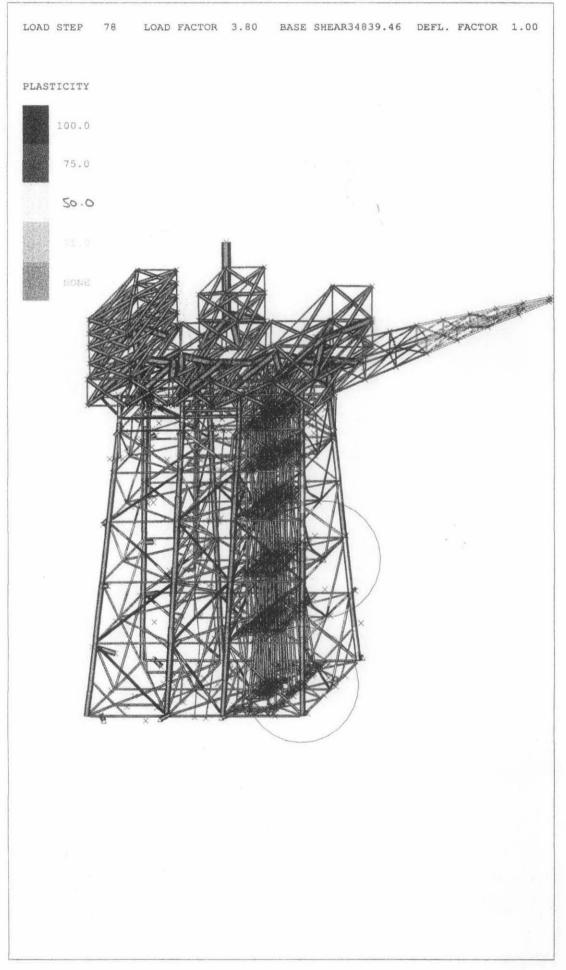


# APPENDIX C.4 C) PLATFORM C

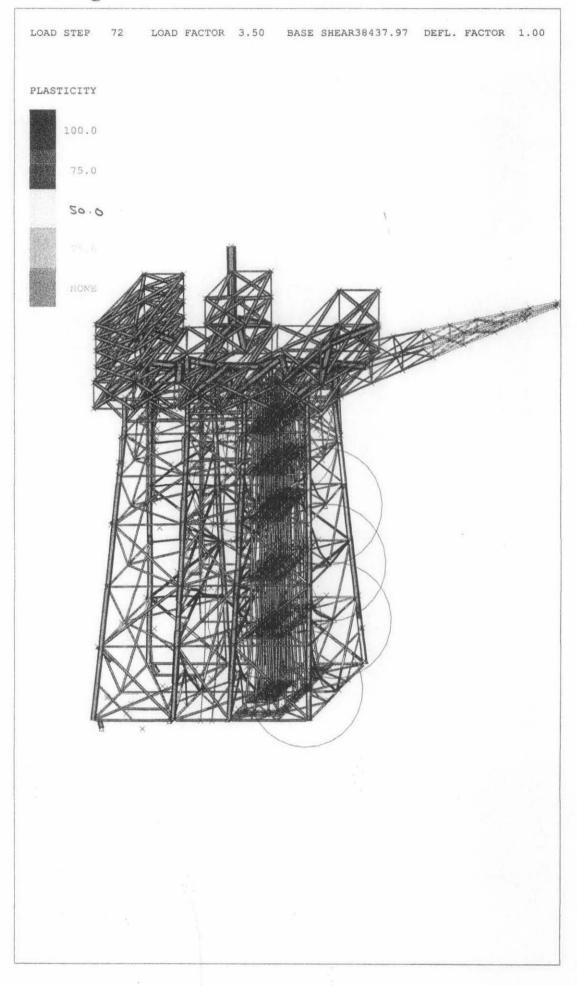
O dea



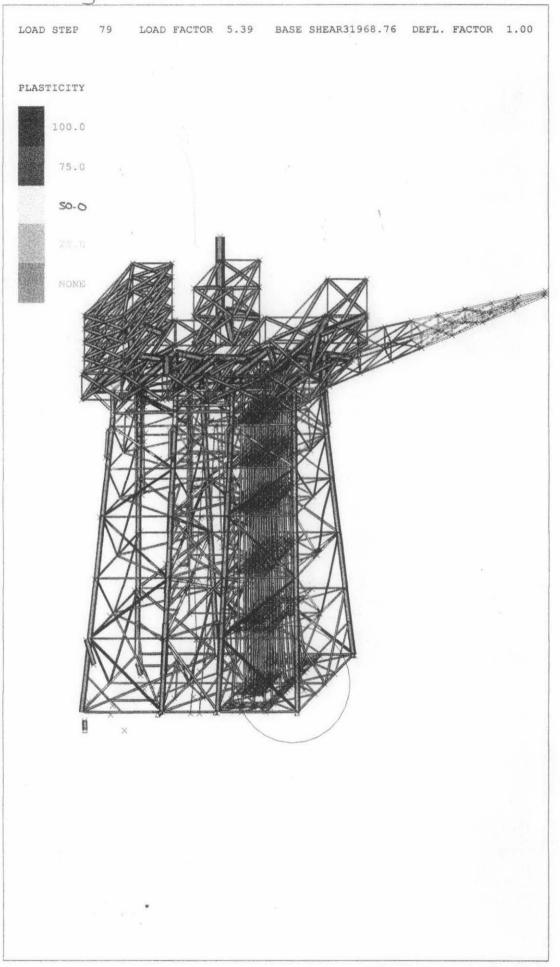
65 209



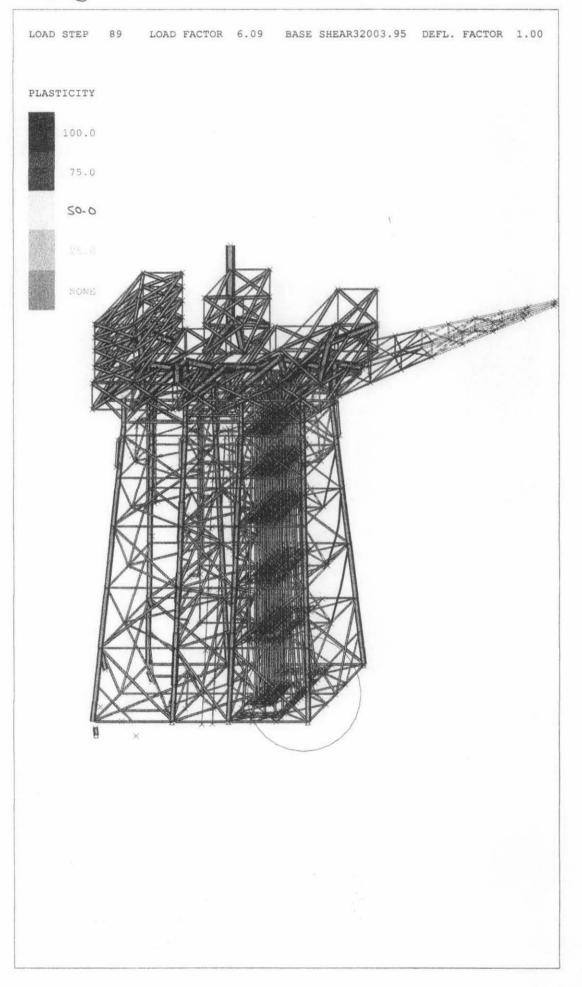
115 deg



270 209







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## STATISTICAL RESULTS ANALYSIS OF BOTH DATA

A) PLATFORM A, B & C COLLAPSE DATA COMPARISON

#### t-Test: Paired Two Sample for Means for Platform B

	Variable 1	Variable 2
Mean	3286.139	4177.111
Variance	209368.6	159270.6
Observations	12	12
Pearson Correlation	0.2387	
Hypothesized Mean Difference	0	
df	11	
t Stat	-5.81762	
P(T<=t) one-tail	5.81E-05	
t Critical one-tail	1.795885	
P(T<=t) two-tail	0.000116	
t Critical two-tail	2.200985	

#### t-Test: Paired Two Sample for Means for Platform A

	Variable	. <u> </u>
	1	Variable 2
Mean	9388.953	10422.41
Variance	2622163	2085908
Observations	8	8
Pearson Correlation	0.966162	
Hypothesized Mean Difference	0	
df	7	
t Stat	-6.72521	
P(T<=t) one-tail	0.000136	
t Critical one-tail	1.894579	
P(T<=t) two-tail	0.000271	
t Critical two-tail	2.364624	

### t-Test: Paired Two Sample for Means for Platform C

	Variable 1	Variable 2
Mean	28556.52	31093.93
Variance	83449873	1.03E+08
Observations	8	8
Pearson Correlation	0.982275	
Hypothesized Mean Difference	0	
df	7	
t Stat	-3.46757	
P(T<=t) one-tail	0.00522	
t Critical one-tail	1.894579	
P(T<=t) two-tail	0.01044	
t Critical two-tail	2.364624	

**B) BRACING CONFIGURATION** 

t-Test: Paired Two Sample for Means for Design basis and X-bracing in RSR

	Variable 1	Variable 2
Mean	1.116387	1 173102
Variance	0.00363	0.015734
Observations	8	8
Pearson Correlation	0.458968	
Hypothesized Mean Difference	0	
df	7	
t Stat	-1.439	
P(T<=t) one-tail	0.096662	
t Critical one-tail	1.894579	
P(T< <del>≖t</del> ) two-tail	0.193325	
t Critical two-tail	2.364624	

t-Test: Paired Two Sample for Means for Design
basis and single diagonal bracing in RSR

	Variable 1	Variable 2
Mean	1.116387	1.107139
Variance	0.00363	0.006866
Observations	8	8
Pearson Correlation	0.801158	
Hypothesized Mean Difference	0	
df	7	
t Stat	0.523498	
P(T<=t) one-tail	0.308393	
t Critical one-tail	1.894579	
P(T<=t) two-tail	0.616786	
t Critical two-tail	2.364624	[ 

t-Test: Paired Two Sample for Means for X-bracing
and single diagonal bracing in RSR

	Variable 1	Variable 2
Mean	1.173102	1.107139
Variance	0.015734	0.006866
Observations	8	8
Pearson Correlation	0.607737	
Hypothesized Mean Difference	0	
df	7	
t Stat	1.868822	
P(T<=t) one-tail	0.051929	
t Critical one-tail	1.894579	
P(T<=t) two-tail	0.103857	
t Critical two-tail	2.364624	