

Creep Performance of Various Bituminous Mixtures

by

Liyana bt. Nikmatlah

Dissertation submitted in partial fulfillment of
the requirements for the
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(Civil Engineering)

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Universiti Teknologi PETRONAS
Bandar Seri Iskandar
31750 Tronoh
Perak Darul Ridzuan

CERTIFICATION OF APPROVAL

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Approved by,



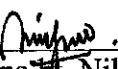
(A.P. Dr. Madzlan b. Napiah)

UNIVERSITI TEKNOLOGI PETRONAS
TRONOH, PERAK

JUNE 2007

CERTIFICATION OF ORIGINALITY

This is to certify that I am responsible for the work submitted in this project, that the original work is my own except as specified in the references and acknowledgements, and that the original work contained herein have not been undertaken or done by unspecified sources or persons.


Liyana bt. Nikmatlah



ABSTRACT

Creep is a failure mode, used to describe the tendency of a material to move or to deform permanently to relieve stresses. An example of permanent deformations is rutting, which is caused by repetitive traffic loading that exceeds the ability of the pavement structure to maintain its original profile. Aggregate's particle size distribution, or gradation, is one of the most influential characteristics in the creep performance analysis. Aggregate gradation plays vital role in the rutting behavior; different gradation yield different strength and durability of pavement structures. This report presents mix design and creep performance data of four gradations; gap graded, continuous graded, open graded and dense graded. The aggregates used are crushed granite (coarse aggregate), river sand (fine aggregate) and ordinary Portland cement (OPC) as filler, while the bitumen used is 80 penetration bitumen. The evaluation of creep performance is done using the Dynamic Creep Test, and the result shows that dense graded mixture yields the best creep resistance.



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CHAPTER 1

INTRODUCTION

1.1 BACKGROUND

Creep is the term used to describe the tendency of a material to move or to deform permanently to relieve stresses. Material deformation occurs as a result of long term exposure to level of stress that are below the yield or ultimate strength of the materials. The rate of this damage is a function of the material properties and the exposure time, exposure temperature, and the applied load (stress). Creep does not happen upon sudden loading, but the accumulation of creep strain in longer times causes failure of the material. An example of a permanent deformation is rutting, which is caused by repetitive traffic loading that exceeds the ability of the pavement structure to maintain its original profile.

Aggregate's gradation is one of the most influential characteristics in the creep performance analysis. Rutting resistance under traffic loads depends on the stability of aggregate structure in bituminous mixture. Aggregate gradation and shape have been recognized among the top factors that influence the stability of aggregate structure.

A bituminous mixture consists of aggregates, sand, binder (bitumen) and filler. As bitumen is a viscoelastic binding material, then, viscoelastic properties can also be found in the bituminous mixtures. One of the reason there is permanent deformation on pavement surface is due to pavement structures with viscoelastic properties. A way to describe viscoelastic properties in bituminous mixtures is creep analysis. Creep involves time dependent deformation under constant compressive stress and temperature level (Tjan and Adrian, 2003).

1.2 PROBLEM STATEMENT

"Rutting is one of the main modes of failure in road pavement. Various bituminous mixtures will result in different creep characteristics."

Rutting problem often requires complete removal and replacement of the rutted layer, which is an expensive undertaking. Rutting is also a serious safety issue for road users. When water accumulates in the ruts, there is a potential for aquaplaning. This phenomenon results in the tire losing contact with the surface with the consequence loss of steering control.

Resistance to permanent deformation such as rutting is an important consideration in the design of bituminous mixtures. It depends on the following factors.

- Magnitude, frequency, pressure, and speed of loading,
- Temperature,
- Aggregate gradation, shape, and texture,
- Binder type, and amount, and
- Construction variables such as compaction, quality control, and segregation.

Aggregate gradation plays vital role in the stability and strength of bituminous structures. Different aggregates gradations will result in different creep characteristics.

1.3 OBJECTIVES AND SCOPE OF STUDY

The objectives of this project are:

- To perform the sieve analyses tests and determine the aggregates proportion for each gradation.
- To assess creep performances of various bituminous mixtures,
 - By preparing various bituminous mixtures with different aggregates gradation; gap graded, continuous graded, open graded, and dense graded mixtures.
 - By performing the dynamic creep test for these different bituminous mixtures.
- To determine the best mixture for creep resistance,
 - By evaluating the result from the dynamic creep test.

CHAPTER 2

LITERATURE REVIEW AND THEORY

Rutting resistance under traffic loads depends on the stability of aggregate structure in bituminous mixture. Lee (1970) has discussed the variation of the aggregate gradation on properties of mixes, while Huang (1970) combines gradation effects and shape effects in his study by using a gradation index and a particle index. He found a large influence of gradation and shape of the aggregate on the properties of the mixes. Huang also suggest that gradation should be further studied in order to get high stability mixes with sufficient voids in the mineral aggregate to allow sufficient asphalt binder. Pan et al. (2006) discussed that fine or coarse graded asphalt mixtures, having either more fine aggregate or more coarse aggregate in the mix respectively often result in different rutting performances under traffic loads.

Pan et al. (2006) also suggested that, in certain asphalt mixes, coarse aggregate are more likely to establish physical contact due to their large sizes, commonly referred as interlock and has been shown to be significant for increased rutting resistances. Previous research studies that realized the important role the coarse aggregate plays in the rutting behavior of Hot Mix Asphalt related stability of aggregate structure to coarse aggregate morphologies.

Lee and Al-Dhalaan (1989) discuss the successful elimination of rutting in Saudi Arabia using coarse graded aggregates. Sahu and Rao (1978) suggested that asphaltic pavements constructed using gap graded aggregates perform well and have better Marshall Design properties than those constructed using continuous graded aggregates. Krutz and Sebaaly (1993) developed the Mohr failure envelope for four mixes with aggregate gradations ranging from fine to coarse and several binder contents. They found that the coarser the aggregate gradation, the less is the impact

of the binder on the Mohr failure properties. Stephens and Sinha (1978) presented data on the effect of shape of the aggregate particles, and recommended blends of regular particles, flat particles, and rod-like particles to achieve optimum strength.

2.1 TESTS

The analysis of the work of Pilat et al. (2000) concerning the influence of mastics on the properties of bituminous mixtures, and specifically its resistance to rutting indicates that the assessment of bituminous mixtures resistance to rutting can be conducted on the basis of tests on creep under static or dynamic pressure.

Triggered by traffic loading, rutting is then a manifestation of two mechanisms: densification and shear deformation or plastic flow. Dawley et al. (1989) stated that rutting is characterized primarily by shear deformation and secondarily by post construction densification. According to Perl et al. (1983), asphalt-concrete creep deformation consists of recoverable and inrecoverable elements, some of which are time-dependent and some are time-independent. Sousa et al. (1991) had done an extensive review prediction of the rut depth as a function of wheel load, time, and temperature.

2.2 AGGREGATE GRADATION

Read and Whiteoak (2003) discussed that a continuously graded mixture was one that contained fractions of various (but not necessarily all) sizes throughout the range, while a gap graded mixture is one where sizes are discontinuous. The traditional British Standards that specify asphalts emerged from the description of asphalt as being gap graded or continuously graded. BS 594 deals with asphalts (gap

graded materials) whilst BS 4987 applies to coated macadam (continuously graded materials).

Hunter (2007) discussed that the wearing course of a road pavement may also be bituminous macadam. Dense bituminous macadam provide a surface texture as a result of the stone content of the material, and when compacted they can have a low permeability. Coated stone is an open-graded material resulting from a limited mass proportion of fine aggregate. Continuously graded materials include dense bitumen macadam commonly used for roadbases and basecourses and for wearing courses on minor roads. Gap graded materials include hot-rolled asphalt wearing course. Table 1 shows typical composition of bituminous mixtures while Table 2 shows typical grading ranges.

Table 1: Typical compositions of bituminous mixtures

Sieve size (mm)	Percentage by weight passing each sieve			
	Bitumen macadam		Asphalt	
	20 mm open-graded crushed rock basecourse	20 mm dense crushed rock basecourse	Column 2/2 roadbase, basecourse & regulating course	Column 3/2, 30/14 type F wearing course
28.0	100	100	100	100
20.0	95-100	95-100	100	100
14.0	50-80	65-85	90-100	85-100
10.0	-	52-72	65-100	60-90
6.30	15-35	39-55	-	-
3.35	10-25	32-46	-	-
2.36	-	-	35-55	60-72
0.600	-	-	15-55	45-72
0.300	-	7-21	-	-
0.212	-	-	5-30	15-50
0.075	0-9	2-9	2-9	8-12

Table 2: Grading ranges for common mixtures

	Coated Stone	Continuously-graded	Gap-graded	Mastic
Coarse aggregate %wt	86.0	52.0	30.0	30.0
Fine aggregate %wt	7.0	38.0	53.0	26.0
Filler %wt	3.0	5.0	9.0	32.0
Bitumen %wt	4.0	5.0	8.0	12.0
Coarse aggregate %wt	64.5	44.1	25.7	27.5
Fine aggregate %wt	5.1	32.2	46.0	18.9
Filler %wt	2.1	4.2	7.8	27.0
Bitumen %wt	8.3	11.5	17.5	26.6
Void content %vol	20.0	8.0	3.0	<1.0
Grade of bitumen pen	100-300	100-200	35-100	15-25

Open graded friction course (OGFC) is a type of asphalt wearing course with a higher amount of air voids than regular asphalt dense graded surface course (DGSC). OGFC can be placed on either asphalt or concrete pavements, and it consists of roughly 93 % crushed stone, 7 % modified asphalt binder, and a small amount of stabilizing fibers.

DGSC is a layer of hot mix asphalt (HMA) that is the standard wearing course for most asphalt pavements. Although there are several different types of layers in asphalt pavements, what most people associate with the term “asphalt” is actually DGSC because it is so frequently used as the top layer of asphalt pavements. DGSC is composed of approximately 95 % aggregate (rocks and sand) and about 5 % asphalt cement binder.

The most common type of flexible pavement surfacing in the U.S. is HMA. HMA is known by many different name such as hot mix, asphaltic concrete (AC or ACP), asphalt, blacktop or bitumen.

2.3 AGGREGATE PROPERTIES

Atkins (2003) mentioned that relative density (specific gravity) for Portland cement is usually 3.15.

Mannan and Ganapathy (2005) discussed the properties of crushed granite and river sand. The physical and mechanical properties of crushed granite aggregate and oil palm shell (OPS) are shown in Table 3. Physical properties such as specific gravity, water absorption and fineness modulus of sand are noted as 2.6, 0.95 % and 2.56, respectively. The maximum bulking of sand due to moisture content has been found to be 13 % increase of volume at the moisture content of 10 %.

Table 3: Physical and mechanical properties of crushed granite and OPS aggregates

Physical and mechanical properties	Granite	OPS
Maximum size (mm)	12.50	12.5
Specific gravity (saturated surface dry)	2.61	1.17
Water absorption for 24 h (%)	0.76	23.30
Aggregate abrasion value (%)	24.00	4.80
Bulk density (compacted), kg/m ³	1470	590
Fineness modulus (FM)	6.33	6.24
Flakiness index (%)	24.94	65.17
Elongation index (%)	33.38	12.36
Aggregate impact value (%)	17.29	7.86

CHAPTER 3

METHODOLOGY

This chapter provides an overview of the works involved in completing the project. There are basically two steps involved which are the literature review and laboratory works. As mentioned in Chapter 2, literature review is done to strengthen the knowledge on this topic. Various journals are referred to study the effect of gradation and creep characteristics in various bituminous mixtures. Also, the journals provide the knowledge on aggregates properties and helped in conducting the laboratory works.

The laboratory works were conducted in Universiti Teknologi PETRONAS Highway Lab. Four tests were conducted which are the Sieve Analysis, Marshall Mix, Marshall Stability Test and Dynamic Creep Test. The procedures and parameters involved in conducting these tests are discussed below.

3.1 LABORATORY WORKS

3.1.1 Sieve Analysis Test

References

1. UTP Highway Engineering Laboratory Manual
2. BS 594: Part 1 (gap graded, refer Table 4)
3. JKR Standard Specification for Road Works (continuous graded, refer Table 5 and dense graded, refer Table 7)
4. Federal Highway Administration (Open graded, refer Table 6)

Objective

1. To determine the gradation of the aggregates.

Tools/Equipment

1. Aggregates (coarse and fine)
2. Set of sieve
3. Mechanical sieve shaker

Procedures

1. 2 kg of coarse aggregate is weight to nearest gram. 500 g of sand is weight to the nearest 0.1 g.
2. Sieve sizes varying from 28 mm to 2.36 mm (depending on types of gradation) are used for coarse aggregate, while sieve sizes of 6.3 mm to 75 μ m (depending on types of gradation) are used for fine aggregates.
3. The aggregates are sieved for 5-10 minutes on the mechanical sieve shaker.
4. Aggregates retained on each sieve are weight and percentage passing each sieve is computed.
5. Aggregates proportions are then calculated.



Figure 1: Set of sieve and mechanical sieve shaker

Table 4: Gradation limit for gap graded mixture

Sieve size (mm)	Percentage by weight passing each sieve
20.0	100
14.0	85-100
10.0	60-90
2.36	60-72
0.600	45-72
0.212	15-50
0.075	8-12

Coarse aggregate : Retained on 2.36 mm sieve

Fine aggregate : Passing 2.36 mm sieve and retained on 0.075 mm sieve

Filler : Passing 0.075 mm sieve

Target binder content: 6.5 %

Table 5: Gradation limit for continuous graded mixture

Sieve size (mm)	Percentage by weight passing each sieve
28	100
20.0	95-100
14.0	65-85
10.0	52-72
6.3	39-55
3.35	32-46
0.300	7-21
0.075	2-8

Coarse aggregate : Retained on 6.30 mm sieve

Fine aggregate : Passing 6.30 mm sieve and retained on 0.075 mm sieve

Filler : Passing 0.075 mm sieve

Target binder content: 4.9 % ± 0.5 %

Table 6: Gradation limit for open graded mixture

Sieve size (mm)	Percentage by weight passing each sieve
12.5	100
9.5	95-100
4.75	30-50
2.36	5-15
0.075	2-2.5

Coarse aggregate : Retained on 2.36 mm sieve

Fine aggregate : Passing 2.36 mm sieve and retained on 0.075 mm sieve

Filler : Passing 0.075 mm sieve

Target binder content: 4.0 % \pm 0.5 %

Table 7: Gradation limit for dense graded mixture

Sieve size (mm)	Percentage by weight passing each sieve
28	100
20.0	76-100
14.0	64-89
10.0	56-81
5.0	46-71
3.35	32-58
1.18	20-42
0.425	12-28
0.150	6-16
0.075	4-8

Coarse aggregate : Retained on 5.00 mm sieve

Fine aggregate : Passing 5.00 mm sieve and retained on 0.075 mm sieve

Filler : Passing 0.075 mm sieve

Target binder content: 4.5 % - 6.5 %

3.1.2 Marshall Mix

Reference

1. UTP Highway Engineering Laboratory Manual.

Objective

1. To prepare the bituminous mixture samples.

Tools/Equipment

1. Aggregates (based on specified grading)
2. Bitumen
3. Marshall Compactor
4. 100 mm moulds
5. Mixer

Procedures:

1. Mixer is heated to 150 °C. Bitumen and moulds are kept at 150 – 160 °C.
2. Aggregates (which had been oven dried previously) and filler are placed in the mixer and mixed dry about 1 minute. Then, appropriate amount of bitumen (3.0 – 7.5 % with 0.5 % incremental) is added to the aggregate until all particles are coated with bitumen.
3. The material is then placed in the mould. The material is distributed evenly by tamping the material (using steel rod) 15 times around the edges and 5 times in the centre.
4. Material is then compacted using Marshall Compactor with 76 blows on both sides.
5. When specimen is cooled down to room temperature it is extruded from the moulds using extruder.
6. 3 samples are prepared for each bitumen contents.

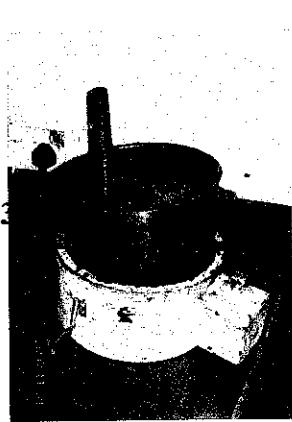


Figure 2: Mixer



Figure 3: Marshall Compactor

3.1.3 Marshall Stability Test

Reference

1. UTP Highway Engineering Laboratory Manual.

Objective

1. To determine the optimum binder content that should be used in the bituminous mixture.

Tools/Equipment

1. Bituminous mixture samples (prepared earlier)
2. Buoyancy Balance Test equipment
3. Bath water
4. Marshall Stability equipment

Procedures:

1. Sample is weighed in air and in water using the Buoyancy Balance Test equipment.
2. Sample is immersed in a bath water at a temperature of $60\text{ }^{\circ}\text{C} \pm 1\text{ }^{\circ}\text{C}$ for a period of 30 to 40 minutes.
3. Then, sample is placed in the Marshall Stability testing machine and loaded at a constant rate of deformation of 2 inch per minute until failure occurs.
4. The total load (kN) that causes failure of the specimen at $60\text{ }^{\circ}\text{C}$ is noted as the Marshall stability value of the specimen. The total amount of deformation (mm) is recorded as flow value.
5. Results are then analysed to determine the optimum binder content.

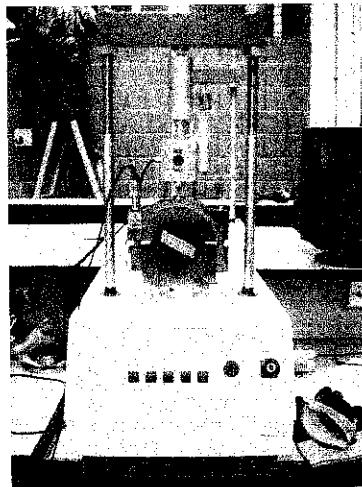


Figure 4: Marshall Stability Test equipment

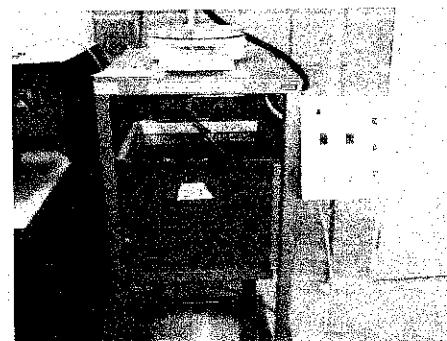


Figure 5: Buoyancy Balance equipment

3.1.4 Dynamic Creep Test

Reference

1. Universal Testing Machine manual.
2. British DD226 test setup.

Objective

1. To determine the creep performance of bituminous mixture samples.

Tools/Equipment

1. Bituminous mixture samples (prepared earlier)
2. Universal Testing Machine (UTM)

Procedures

1. The circulating-air environmental chamber is set to 40 °C.
2. The British DD226 standard testing is set up.
3. Setup parameters (pre-load):
 - stress 12 kPa
 - hold time 120 s
4. Setup parameters (loading):
 - Deviator stress 100 kPa
 - Contact stress 2 kPa
 - Rest period 1000 ms
 - Pulse width 1000 ms
 - Wave shape square pulse
 - Test duration 1800 cycles or until fail
5. Test data is stored automatically and graphs are shown in the next chapter.

CHAPTER 4

RESULTS & DISCUSSIONS

4.1 SIEVE ANALYSIS

4.1.1 Gap Graded

Table 8: Sieve analysis result for gap graded mixture

Passing sieve	Average % by weight passing		
	Coarse (A)	Fine (B)	Filler (C)
20	99.73	100.00	100.00
14	63.92	100.00	100.00
10	25.77	100.00	100.00
2.36	0.12	91.53	100.00
0.6	0.00	55.80	100.00
0.212	0.00	5.40	100.00
0.075	0.00	0.40	80.00

From calculation (sample calculation is attached in Appendix 1-a), the proportion of aggregates required to achieve the gap graded mixture is:

25 % coarse aggregate

63 % fine aggregate

12 % filler

The grading envelope for the gap graded mixture is shown in Figure 6. The “minimum” and “maximum” refer to the minimum and maximum percentage passing each sieve, based on the gradation limit specified by each standard. The grading envelope for the mixture (based on the

calculated aggregates proportion) must lie within the minimum and maximum range.

4.1.2 Continuous Graded

Table 9: Sieve analysis result for continuous graded mixture

Sieve size (mm)	Average % by weight passing		
	Coarse (A)	Fine (B)	Filler (C)
28	100.00	100.00	100.00
20	99.52	100.00	100.00
14	69.80	100.00	100.00
10	33.38	100.00	100.00
6.3	7.67	99.87	100.00
3.35	0.00	97.07	100.00
0.3	0.00	7.07	100.00
0.075	0.00	0.40	80.00

From calculation (attached in Appendix 1-b), the proportion of aggregates required to achieve the continuous graded mixture is:

54 % coarse aggregate

38 % fine aggregate

8 % filler

The grading envelope for the mixture is shown in Figure 7.

4.1.3 Open Graded

Table 10: Sieve analysis result for open graded mixture

Sieve size (mm)	Average % by weight passing		
	Coarse (A)	Fine (B)	Filler (C)
12.5	99.55	100.00	100.00
9.5	98.97	100.00	100.00
4.75	33.45	100.00	100.00
2.36	0.15	90.40	100.00
0.075	0.00	0.40	80.00

From calculation (attached in Appendix 1-c), the proportion of aggregates required to achieve the open graded mixture is:

90 % coarse aggregate

7.5 % fine aggregate

2.5 % filler

The grading envelope for the mixture is shown in Figure 8.

4.1.4 Dense Graded

Table 11: Sieve analysis result for dense graded mixture

Sieve size (mm)	Average % by weight passing		
	Coarse (A)	Fine (B)	Filler (C)
28	100.00	100.00	100.00
20	98.05	100.00	100.00
14	58.65	100.00	100.00
10	15.32	100.00	100.00
5	2.30	100.00	100.00
3.35	0.00	92.60	100.00
1.18	0.00	66.20	100.00
0.425	0.00	30.70	100.00
0.15	0.00	4.50	100.00
0.075	0.00	0.60	80.00

From calculation (attached in Appendix 1-d), the proportion of aggregates required to achieve the dense graded mixture is:

51 % coarse aggregate

42 % fine aggregate

7 % filler

The grading envelope for the mixture is shown in Figure 9.

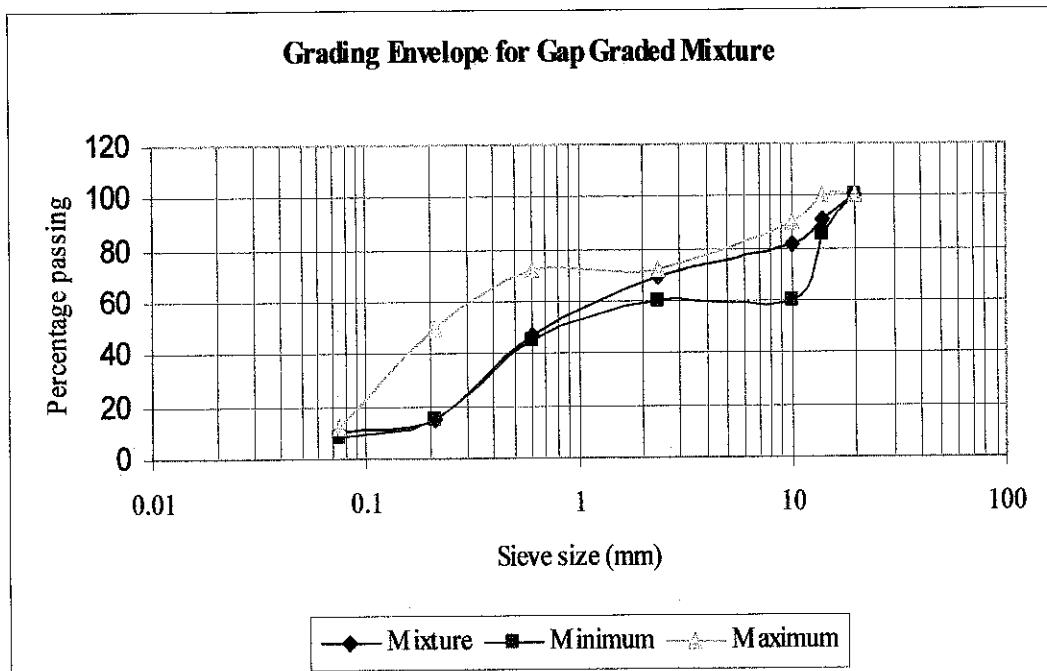


Figure 6: Grading envelope for gap graded mixture

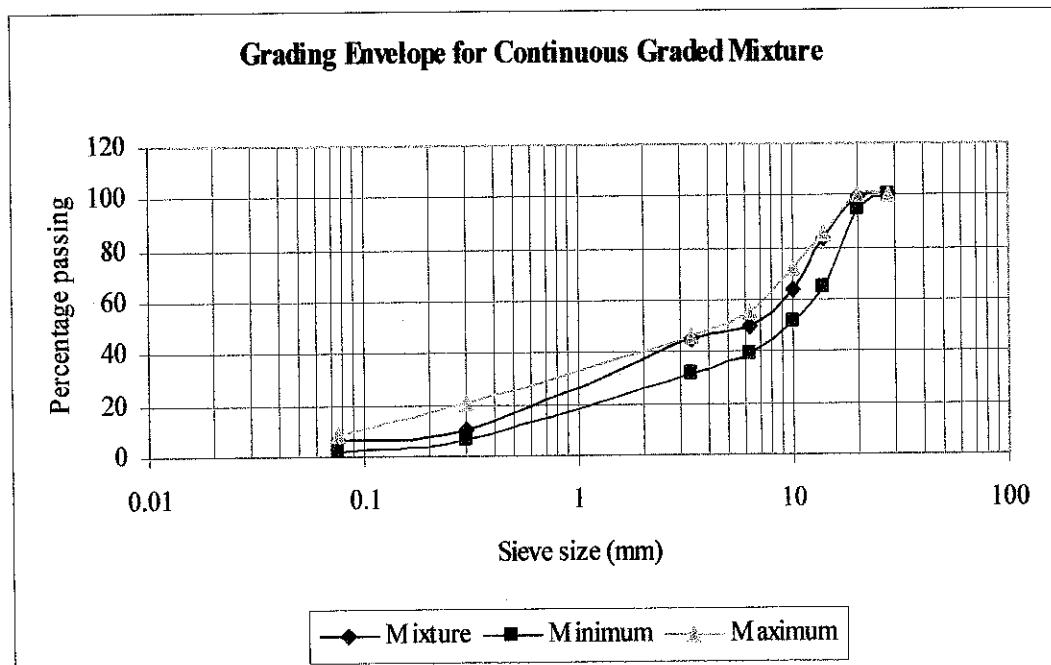


Figure 7: Grading envelope for continuous graded mixture

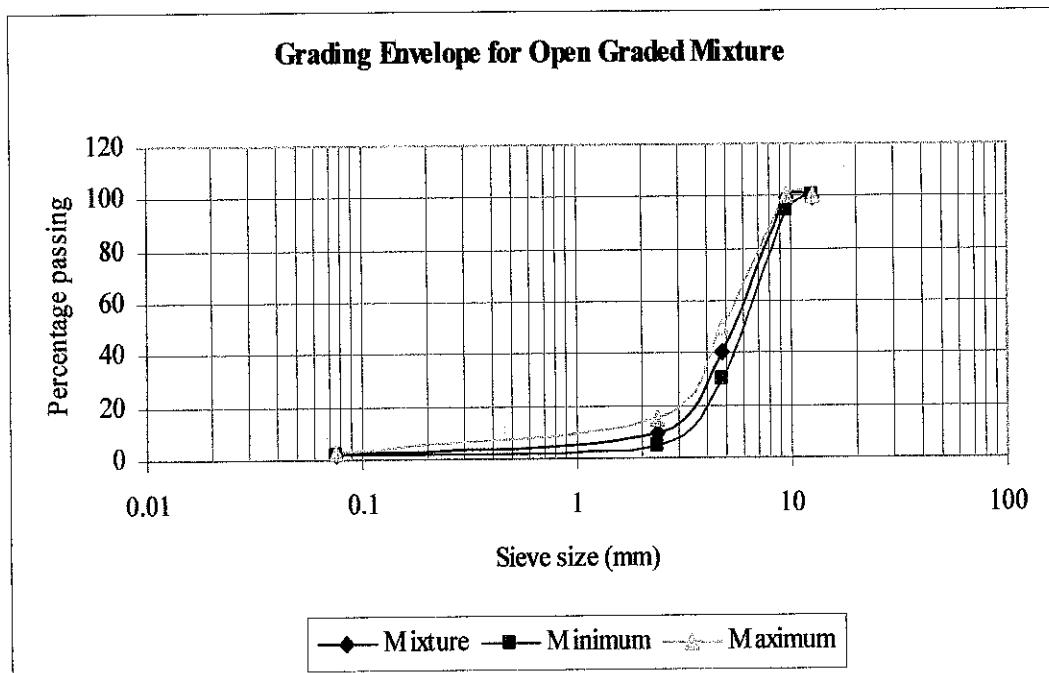


Figure 8: Grading envelope for open graded mixture

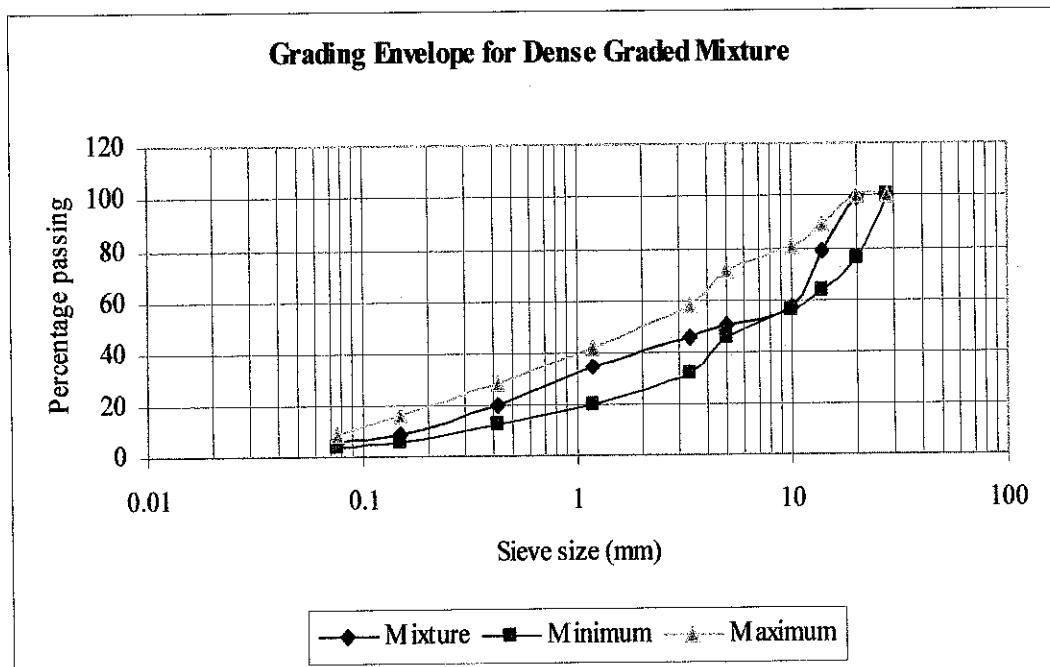


Figure 9: Grading envelope for dense graded mixture

4.2 MARSHALL STABILITY TEST

In Marshall Mix, three samples were prepared for each bitumen contents. The optimum binder content (OBC) for each gradation is determined through the Marshall Stability Test. The weight of the samples in air and in water, diameter, height and Marshall Stability value for each samples are presented in Appendix 2. The following are the summary of the results. Also, the steps in determining the OBC are discussed below. From literature review, the following specific gravity is used for the OBC calculation.

Table 12: Specific gravity for aggregates and filler

Type	Bulk specific gravity
Coarse	Crushed granite
Fine	River sand
Filler	OPC

4.2.1 Gap Graded

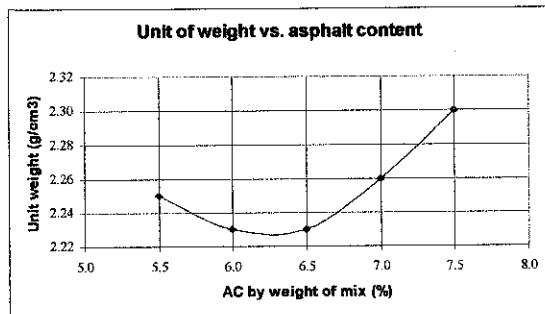
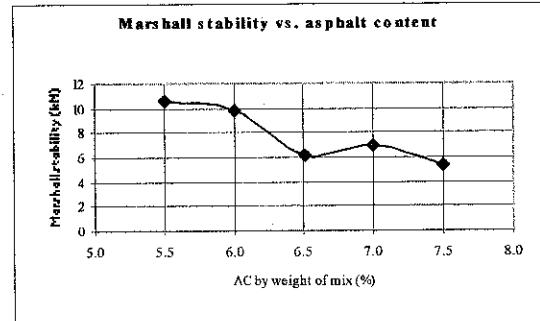
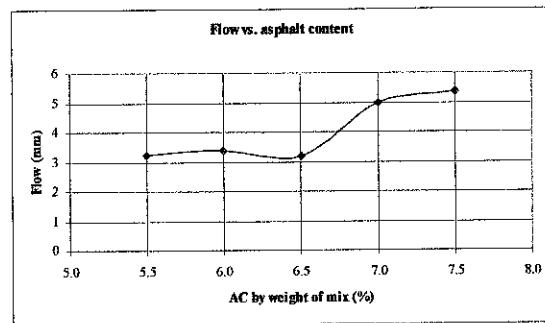
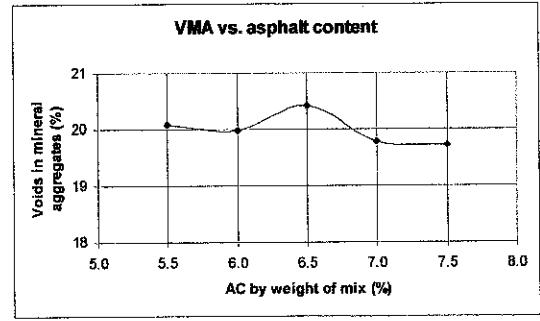
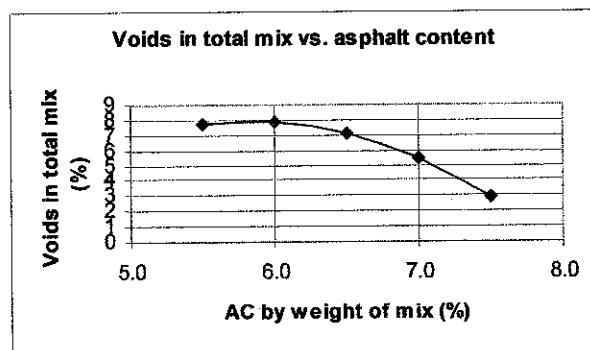
For gap graded mixture, 15 samples were prepared, ranging from 5.5 % to 7.5 %. The voids in mineral aggregates (VMA) and voids in total mix values are obtained from calculation. Refer Appendix 3 for sample calculation.

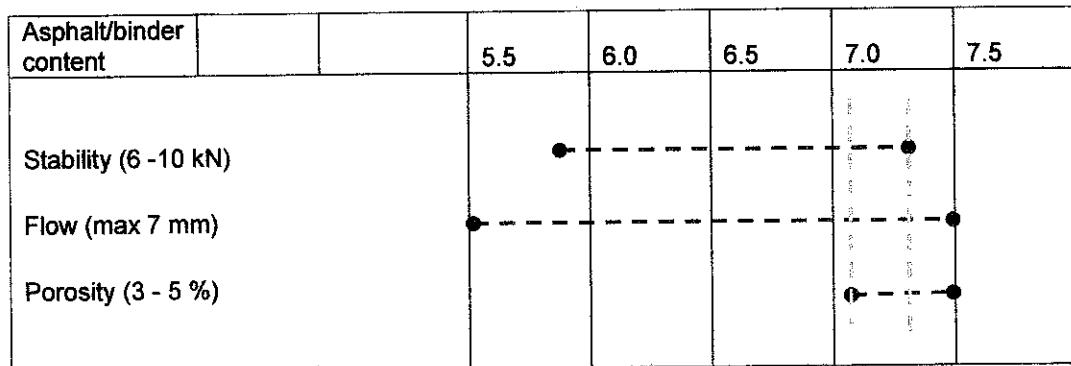
Table 13: Summary of Marshall Stability result for gap graded mixture

Bitumen content (%)	Weight (g)		Diameter (mm)	Height (mm)	Marshall Test	
	in air	in water			Load (kN)	Flow (mm)
5.5	1239.2	688.7	102.85	68.22	10.66	3.22
6.0	1257.8	694.2	102.72	69.92	9.76	3.37
6.5	1256.7	692.3	102.42	69.83	6.23	3.19
7.0	1266.3	707.2	102.52	68.97	6.88	4.99
7.5	1284.8	726.3	103.64	67.97	5.37	5.37

Table 14: Values for OBC calculation

Bitumen content (%)	Bulk density	Stability	Flow	VMA	Voids in total mix
5.5	2.25	10.66	3.22	20.07	7.79
6.0	2.23	9.76	3.37	19.99	7.85
6.5	2.23	6.23	3.19	20.42	7.08
7.0	2.26	6.88	4.99	19.78	5.44
7.5	2.30	5.37	5.37	19.72	2.95

*Figure 10: Unit weight vs. asphalt content**Figure 11: Stability vs. asphalt content**Figure 12: Flow vs. asphalt content**Figure 13: VMA vs. asphalt content**Figure 14: Voids in total mix vs. asphalt content*

*Figure 15: Determining OBC for gap graded mixture*

The stability, flow and porosity lines intersect between binder content 7.1 % to 7.3 %. Thus, taking the mean of these values, the OBC for gap graded mixture is 7.2 %.

4.2.2 Continuous Graded

For continuous graded mixture, 12 samples were prepared, ranging from 4.0 % to 5.5 %.

Table 15: Summary of Marshall Stability result for continuous graded mixture

Bitumen content (%)	Weight (g)		Diameter (mm)	Height (mm)	Marshall Test	
	in air	in water			Load (kN)	Flow (mm)
4.0	1218.8	703.5	103.87	65.27	17.98	4.62
4.5	1236.0	711.0	103.61	65.14	12.49	4.86
5.0	1228.2	709.0	102.51	65.55	14.21	4.49
5.5	1229.7	706.3	104.57	63.80	9.85	3.93

Table 16: Values for OBC calculation

Bitumen content (%)	Bulk density	Stability	Flow	VMA	Voids in total mix
4.0	2.35	17.98	4.62	14.55	5.66
4.5	2.36	12.49	4.86	14.99	4.68
5.0	2.37	14.21	4.49	14.72	3.84
5.5	2.35	9.85	3.93	15.88	3.70

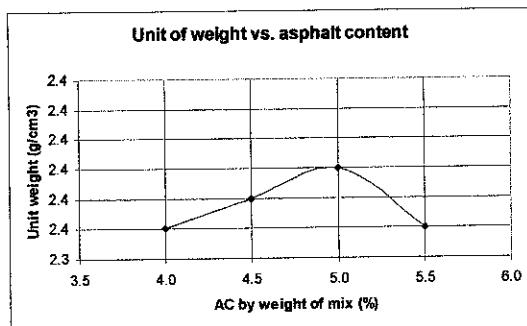


Figure 16: Unit weight vs. asphalt content

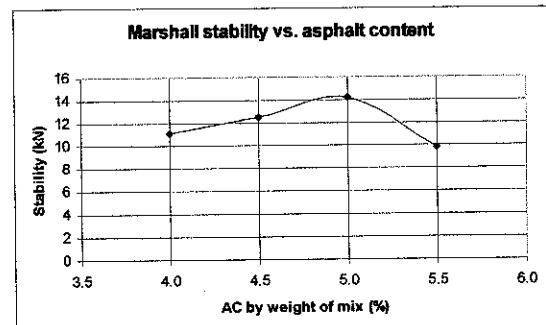


Figure 17: Stability vs. asphalt content

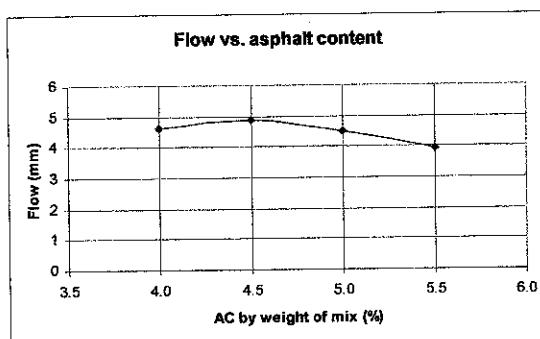


Figure 18: Flow vs. asphalt content

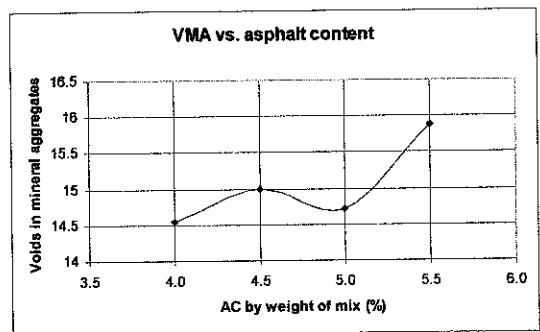


Figure 19: VMA vs. asphalt content

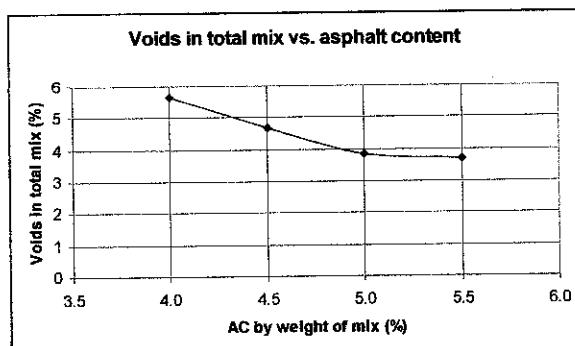


Figure 20: Voids in total mix vs. asphalt content

The asphalt content having the maximum value of unit weight and stability is selected from each of the respective plots.

Max unit weight = 5.0 %

Max stability = 4.9 %

Percent air voids in compacted mixture using mean of limits given in the JKR Standard Specification for Road Works [that is, $(3 + 5)/2 = 4$] = 4.90 %

Therefore, the optimum asphalt content or OBC for continuous graded mixture is;

$$\frac{5.0 + 4.90 + 4.90}{3} = 4.93 \%$$

4.2.3 Open Graded

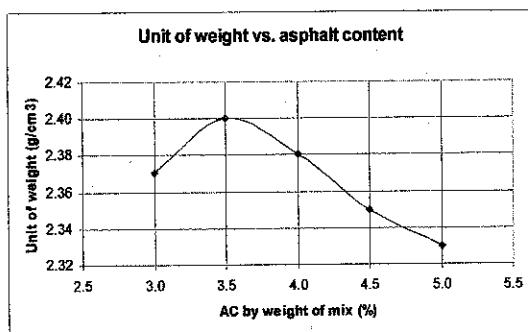
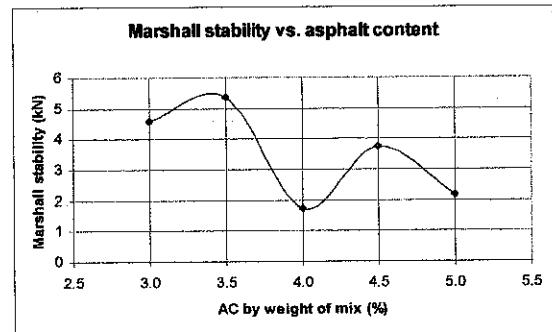
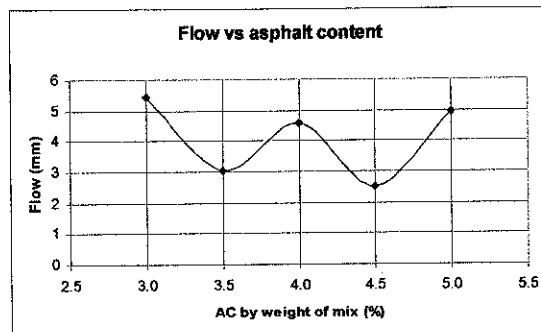
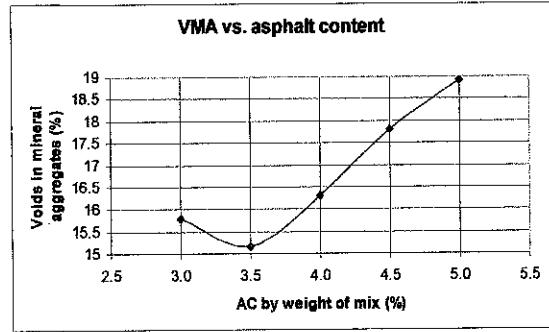
For open graded mixture, 15 samples were prepared, ranging from 3.0 % to 5.0 %.

Table 17: Summary of Marshall Stability result for open graded mixture

Bitumen content (%)	Weight (g)		Diameter (mm)	Height (mm)	Marshall Test	
	in air	in water			Load (kN)	Flow (mm)
3.0	1225.8	708.5	103.32	80.98	4.56	5.45
3.5	1224.5	714.0	102.71	81.53	5.37	3.04
4.0	1231.0	714.2	102.68	82.81	1.71	4.59
4.5	1235.2	710.0	101.48	82.83	3.77	2.52
5.0	1251.7	713.5	103.70	81.89	2.16	4.97

Table 18: Values for OBC calculation

Bitumen content (%)	Bulk density	Stability	Flow	VMA	Voids in total mix
3.0	2.37	4.56	5.45	15.79	6.32
3.5	2.40	5.37	3.04	15.16	4.38
4.0	2.38	1.71	4.59	16.31	4.42
4.5	2.35	3.77	2.52	17.79	4.86
5.0	2.33	2.16	4.97	18.92	5.28

*Figure 21: Unit weight vs. asphalt content**Figure 22: Stability vs. asphalt content**Figure 23: Flow vs. asphalt content**Figure 24: VMA vs. asphalt content*

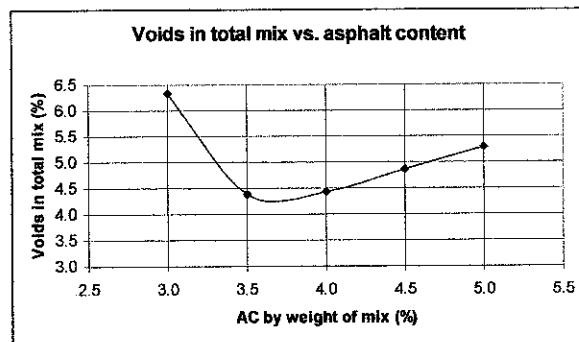


Figure 25: Voids in total mix vs. asphalt content

The asphalt content having the maximum value of unit weight and stability is selected from each of the respective plots.

Max unit weight = 3.5 %

Max stability = 3.4 %

Percent air voids in compacted mixture using maximum limits of 5 % = 3.3 %

Therefore, the optimum asphalt content or OBC for continuous graded mixture is;

$$\frac{3.5 + 3.4 + 3.3}{3} = \mathbf{3.40\%}$$

4.2.4 Dense Graded

For dense graded mixture, 24 samples were prepared, ranging from 3.0 % to 6.5 %.

Table 19: Summary of Marshall Stability result for dense graded mixture

Bitumen content (%)	Weight (g)		Diameter (mm)	Height (mm)	Marshall Test	
	in air	in water			Load (kN)	Flow (mm)
3.0	1229.2	690.3	101.68	69.00	10.36	2.61
3.5	1232.8	712.2	103.81	65.66	12.45	3.51
4.0	1225.7	709.7	103.87	64.77	14.13	0.29
4.5	1224.5	710.5	101.58	66.89	21.00	1.62
5.0	1210.7	702.3	102.77	64.20	14.45	2.79
5.5	1248.2	728.5	103.67	63.37	12.83	4.07
6.0	1241.0	725.3	103.73	63.01	10.36	4.98
6.5	1221.2	710.2	103.92	62.93	9.56	5.56

Table 20: Values for OBC calculation

Bitumen content (%)	Bulk density	Stability	Flow	VMA	Voids in total mix
3.0	2.28	10.36	2.61	16.23	9.88
3.5	2.37	12.45	3.51	13.37	5.58
4.0	2.38	14.13	0.29	13.45	4.42
4.5	2.38	21.00	1.62	13.91	3.64
5.0	2.38	14.45	2.79	14.36	3.25
5.5	2.40	12.83	4.07	14.09	1.64
6.0	2.41	10.36	4.98	14.19	0.41
6.5	2.39	9.56	5.56	15.35	0.42

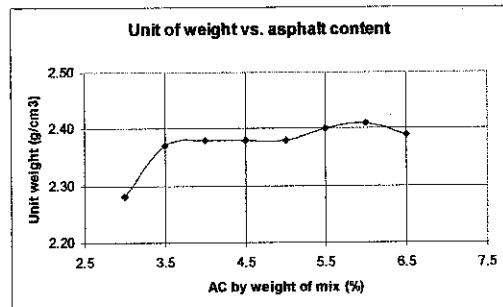


Figure 26: Unit weight vs. asphalt content

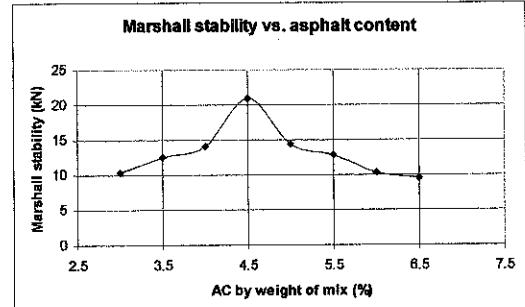


Figure 27: Stability vs. asphalt content

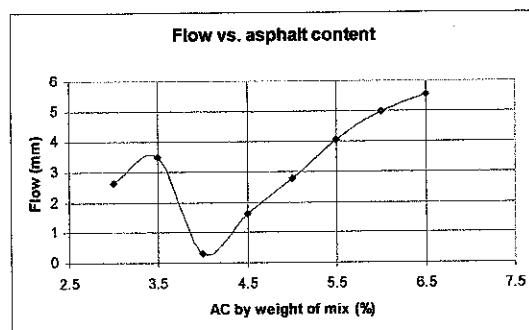


Figure 28: Flow vs. asphalt content

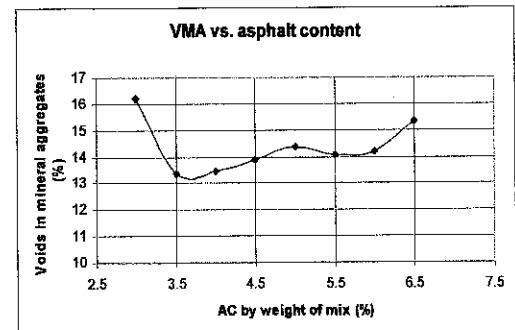


Figure 29: VMA vs. asphalt content

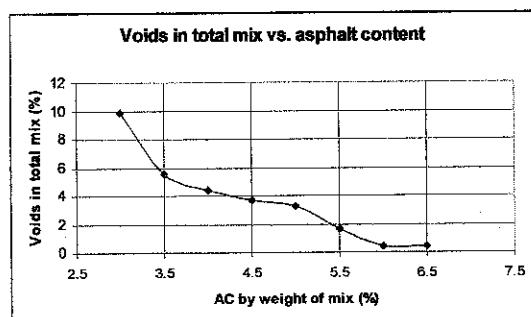


Figure 30: Voids in total mix vs. asphalt content

The asphalt content having the maximum value of unit weight and stability is selected from each of the respective plots.

Max unit weight = 6.0 %

Max stability = 4.5 %

Percent air voids in compacted mixture using mean of limits given in the JKR Standard Specification for Road Works [that is, $(3 + 5)/2 = 4$]
= 3.42 %

Therefore, the optimum asphalt content or OBC for dense graded mixture is;

$$\frac{6.0 + 4.5 + 3.42}{3} = 4.64 \%$$

4.3 DYNAMIC CREEP TEST

From the Dynamic Creep Test conducted, the creep stiffness for each samples are recorded (refer Appendix 4). The following table discussed the summary of the results.

The S_{mix} average refers to the average creep stiffness of mixture based on the Dynamic Creep Test results. Referring to “*Nomograph for stiffness modulus of bitumen*” in Appendix 5, the stiffness modulus of bitumen, S_{bit} are as follow:

Table 21: Summary of results from Dynamic Creep Test (S_{mix}) and S_{bit}

Cycles	Time of loading (s)	S_{mix} average (MPa)				S_{bit} (MPa)
		Gap	Continuous	Open	Dense	
10	20	19.562	44.531	29.904	150.433	1.50E-03
20	40	15.275	35.38	19.761	117.495	1.00E-03
30	60	13.365	30.989	17.479	103.902	7.50E-04
60	120	10.873	25.259	14.338	82.997	5.00E-04
180	360	8.126	18.848	10.63	56.016	1.00E-04
420	840	6.618	15.273	7.947	40.488	8.00E-05
600	1200	6.115	14.028	6.761	34.908	7.00E-05
1200	2400	5.356	11.831	5.327	25.576	1.05E-05
1800	3600	5.066	10.551	4.605	20.786	1.00E-05

Graph S_{mix} average vs. S_{bit} for all gradations are plotted (refer Figure 31). From the graph, linear equations for the new S_{mix} are obtained (equations are displayed on Figure 31). To find the rut depth, the first step is to calculate the viscous component of the stiffness modulus of the bitumen, $(S_{bit})_v$. Refer Appendix 7 for sample calculation.

$$(S_{bit})_v = \frac{3\eta}{NT_v}$$

- $(S_{bit})_v$ = the viscous component of the stiffness modulus of the bitumen
 η = the viscosity of the bitumen as a function of PI and ring and ball temperature from Appendix 6.
 N = the number of wheel passes in standard axles
 T_w = the time loading for one wheel pass, taken as 0.02s

The rut depth is then calculated using the stiffness linear relationship obtained from Figure 31. The equation below is used to calculate the rut depth:

$$R_d = C_m \times H \times \left(\frac{\sigma_{av}}{S_{mix}} \right)$$

- R_d = calculated rut depth of the pavement in mm
 C_m = correlation factor for dynamic effect, varying from 1.0 to 2.0, taken as 1.5
 H = pavement layer thickness, assumed 65mm
 σ_{av} = average stress in the pavement, related to wheel loading and stress, taken as 2.5 MPa
 S_{mix} = stiffness of the design mixture derived from creep test at a certain value of stiffness which is related to the viscous part of the bitumen

From the calculations using the above equation, a relationship between rut depths and cycles to standard axial loading can be established as in Figure 32. The following table shows the summary of calculated new S_{mix} , $(S_{bit})_v$ and rut depth.

Table 22: Calculated results for S_{mix} , $(S_{bv})v$, and rut depth

N	S_{mix} (MPa)			Sbit vis			Rut depth (mm)		
	Gap	Continuous	Open	Dense	Gap	Continuous	Open	Dense	
1.E+00	75.996	193.678	178.673	1319.953	7.50E-01	3.207	1.259	1.364	0.185
1.E+01	42.736	105.579	83.899	565.534	7.50E-02	5.704	2.309	2.905	0.431
1.E+02	24.032	57.555	39.397	242.303	7.50E-03	10.143	4.235	6.187	1.006
1.E+03	13.514	31.375	18.499	103.815	7.50E-04	18.037	7.769	13.176	2.348
1.E+04	7.600	17.103	8.687	44.479	7.50E-05	32.074	14.252	28.060	5.480
1.E+05	4.274	9.324	4.079	19.057	7.50E-06	57.037	26.144	59.757	12.790
1.E+06	2.403	5.083	1.915	8.165	7.50E-07	101.427	47.958	127.259	29.853

Graph Stiffness modulus of mixture vs. bitumen

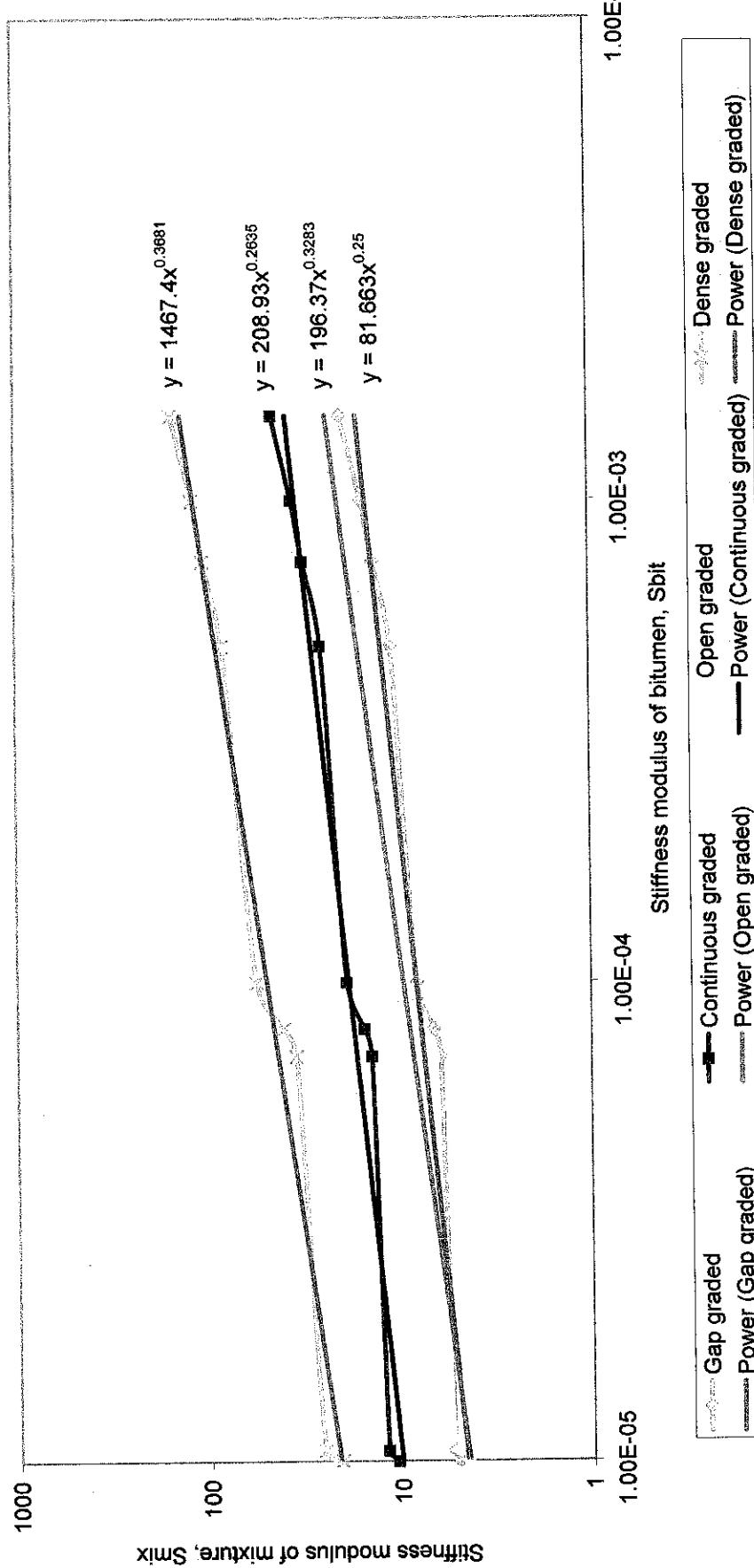


Figure 31: Graph stiffness modulus of mixture vs. bitumen

Graph rut depth vs. N

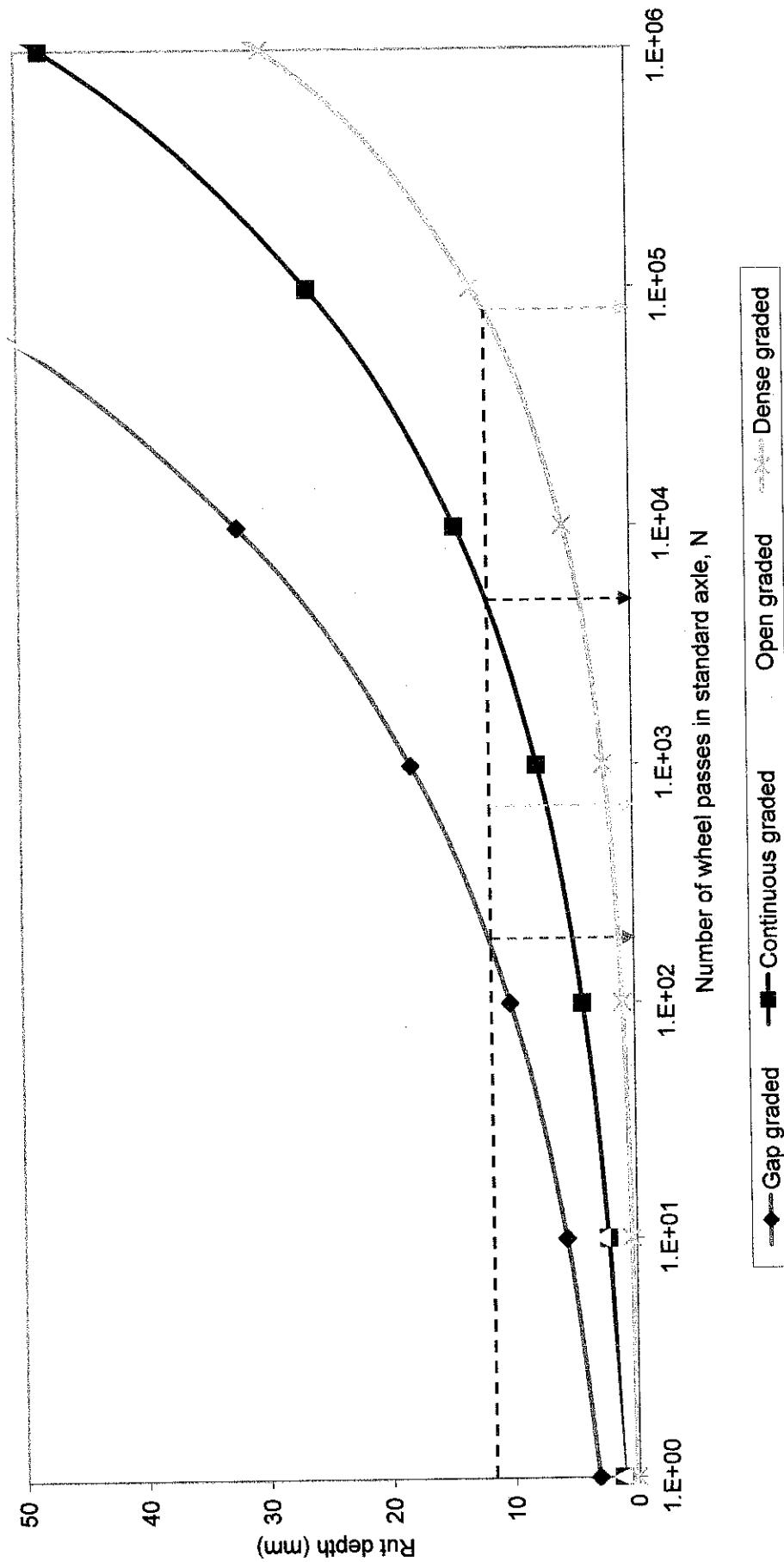


Figure 32: Graph rut depth vs. number of wheel pass in standard axle, N

As shown in Figure 32, the gradation with the highest rut depth is gap graded mixture, followed by open graded and continuous graded mixture. Dense graded mixture shows the lowest rut depth, thus indicating the highest resistance to creep. As discussed by Uzan (2004), 12 mm rut depth is taken as the maximum allowable rut depth for maintenance purposes. Referring to Figure 32, at 12 mm rut depth the N (numbers of wheels pass at standard axle) values are as follow:

Gap graded	1.9×10^2
Continuous graded	6.8×10^2
Open graded	5.0×10^3
Dense graded	8.0×10^4

The values indicate that the expected maintenance to be carried out for each gradation is at the specified N. Dense graded mixture shows the highest value of N thus indicates the most stable mixture.

For gap graded mixture, the coarse aggregate used is only 25 %, while the fine aggregate used is 63 %. From previous study by Huber and Heiman (1987), the mix that cause rutting problem was oversand mix. Since the amount of coarse aggregate is small, the stone-on-stone contact is minimal. Stone-on-stone contact is very important as bituminous mixture develop their strength from both aggregate interlock and viscosity of the binder. Coarse aggregate are more likely to establish physical contact or interlock due to their larger sizes. Interlock properties has shown to be significant for increased rutting resistance of the surface courses in high volume roads (Pan et al., 2006).

Comparing gap graded mixture with open graded mixture; the coarse aggregate used in open graded mixture is much higher (90 %) than those used in gap graded mixture (25 %). This proves that the interlock property is significant in rutting resistance. Thus, this shows that rutting resistance of bituminous mixture depends on the

stability of aggregate structure in the mixture. While conducting the Dynamic Creep Test, two samples from open graded mixtures failed. Though the average rut depth shows that open graded mixture is the third best mixture in term of creep resistance, other aspects such as durability and strength must also be considered.

For dense graded mixture, the coarse aggregate used is 51 %, the fine aggregate used is 42 % while the filler used is 7 %. As compared to gap graded and open graded mixture, the coarse aggregate and fine aggregate used is “optimum”; not too much coarse aggregate, not too much fine aggregate. The optimum content of coarse aggregate gives good interlocks property, while optimum amount of fine aggregate helps to fill the air voids. From Abdullah et al. (1998), air voids play important role in the durability and stability of pavement. High permeability to air causes embrittlement of bituminous binder due to oxidation, causing the pavement to crack. High permeability to water encourages stripping of the bitumen from aggregate particles, and endangering the subgraded layer and the base course as well. Low voids contents, is one of the main factors in causing rutting of pavement.

While for continuous graded mixture, the coarse aggregate used is 54 %, the fine aggregate used is 38 %, and the filler used is 8 %. The aggregate proportions are similar to dense graded, but continuous graded mixture shows lower rut resistance compared to dense graded mixture. The rut resistance is higher compared to open graded and gap graded mixtures.

The dense graded mixture shows the highest rut resistance, thus highest creep resistance. Apart from good rut resistance, the dense graded mixture also has other advantages. From previous studies, it is shown that using the dense graded mixture can increase the crack resistance, and increase pavement lifespan.

CHAPTER 5

CONCLUSION AND RECOMMENDATION

5.1 CONCLUSION

This project investigated the relationship between aggregate gradation and creep performance. There were four gradations studied, which are the gap graded, continuous graded, open graded, and dense graded mixture. The following are the summary of all experimental results.

Table 23: Summary of all experimental results

Properties		Gap Graded	Continuous Graded	Open Graded	Dense Graded
Mixture	Coarse	25 %	54 %	90 %	51 %
	Fine	63 %	38 %	7.50 %	42 %
	Filler	12 %	8 %	2.50 %	7 %
	OBC	7.20 %	4.93 %	3.40 %	4.64 %
Ranking on rut resistance		4	2	3	1

From the rut depth result, it can be concluded that dense graded mixture has the highest creep resistance, followed by continuous graded mixture, open graded mixture and gap graded mixture. Thus, it can be said that, based on the experimental result, the best mix for creep resistance is the dense graded mixture. The objectives of the project are achieved.

5.2 RECOMMENDATION

There are several aspects that can be improved for this project. For the Marshall Mix sample preparation, the used of Gyratory Compactor will enhance the quality of sample, thus improving the overall result. In Gyratory compaction, the test involves placing sample of hot mould and applying a static pressure of a controlled magnitude. The mould is then gyrated to allow aggregate particles to reorientate themselves under loading.

Tests for aggregate must also be conducted to get more accurate results. In this experiment, the specific gravities of aggregates are taken from literature review. Hence, the actual value may differ from the one used in calculation. Also, other tests such as flakiness and elongation index, and Los Angeles abrasion test should be conducted to study the affect of aggregates shape on the rutting behavior. Different types of aggregates may affect the creep performance of bituminous mixtures. Different aggregates have different properties, thus may affect its creep performance. Further study should be conducted to study this relationship.

In future, the creep performance can be studied by varying the aggregates types. Example of coarse aggregate that can be used is crushed limestone, while fine aggregate that can be used is mining sand. Apart from studying the aggregate properties and its effects on creep performance, other tests such as fatigue test and wheel tracking test can be conducted to measure the strength and durability of the mixture. The environmental and cost analyses should also be conducted.

REFERENCES

- Abdullah, W.S., Obaidat, M.T., and Abu-Sa'da, N.M. (1998). "Influence of Aggregate Type and Gradation on Voids of Asphalt Concrete Pavements." *Journal of Materials in Civil Engineering*.
- Atkins, H. N. (2003). "*Highway Materials, Soils, and Concretes*". Fourth Edition.
- BS 594: Part 1: 1992. "*Hot Rolled Asphalt for Roads and Other Paved Areas*" Part 1. Specification for Constituent Materials and Asphalt Mixtures.
- Dawley, C.B., Hogewiede, B.L., and Anderson, K.o. (1989). "Mitigation of Instability Rutting of Asphalt Concrete Pavements in Lethbridge, Alberta, Canada." Proc., Assoc. of Asphalt Paving Technologists, Vol. 58.
- Hawai'i Asphalt Paving Industry (HAPI), HAPI Asphalt Pavement Guide.*
www.hawaiiasphalt.com/HAPI/modules/04_pavement_types/04_mix_types.htm
- Huang, E. Y. (1970). "A study of strength characteristics of asphalt-aggregate mixtures as affected by the geometric characteristics and gradation of aggregates." Proc., Assoc. of Asphalt Paving Technologists (AAPT), St. Paul, Minn., Vol. 39, 98-133.
- Huber, G. A., and Heiman, G. H. (1987). "Effect of Asphalt Concrete Parameters on Rutting Performance: A Field Investigation." Proc., Assoc. of Asphalt Paving Technologists (AAPT), St. Paul, Minn., Vol. 56, 33-61.

Hunter, R. N. (2007). "*Bituminous Mixtures in Road Construction*", Google Book Search.

<http://books.google.com/books?hl=en&lr=&id=G33BAZUTX9oC&oi=fnd&pg=PR13&sig=G5iZ>

Jabatan Kerja Raya, Malaysia (1988). "*JKR Standard Specification for Road Works.*"

Krutz, N. C., and Sebaaly, P. E. (1993). "*The effect of aggregate gradation on permanent deformation on asphaltic concrete.*" Proc., Assoc. of Asphalt Paving Technologists (AAPT), St. Paul Minn., Vol. 62, 450-473.

Lee, G. (1970). "*The rational Design of Aggregate Grading for Dense Asphaltic Compositions.*" Proc., Assoc. of Asphalt Paving Technologists (AAPT), St. Paul Minn., Vol. 39, 60-97.

Lee, K. W, and Al-Dhalaan, M.A. (1989). "*Rutting, Asphalt Mix Design and Propose Test Road in Saudi Arabia.*" ASTM Philadelphia, Pa, 103-119.

Mannan, M.A., and Ganapathy, C. (2005). "*Concrete from an agricultural waste-oil palm shell (OPS)*".

Nicholas, J. G., and Lester, A.H. "*Traffic & Highway Engineering*". Third Edition.

Pan, T., Tutumluer, E., and Carpenter, S. H. (2006). "*Effect of Coarse Aggregate Morphology on Permanent Deformation Behavior of Hot Mix Asphalt.*" Journal of Transportation Engineering, Vol. 132, No. 7.

Perl, M., Uzan, J., and Sides, A. (1983). "Visco-elasto-palstic constitutive law for a bituminous mixture under repeated loading." *Transp. Res. Record 911*, Trans Rep Board, Washington D.C.

Pilat, J., Rasziszewski, P., and Klabiska, M. (2000). "The analysis of viscoelastic properties of mineral-asphlat mixes with lime and rubber powder." *2nd Eurasphalt & Eurobitume Congress*. Barcelona, Spain.

Read, J., and Whiteoak, D. (2003). "The Shell Bitumen Handbook". Fifth Edition.

Rubber-Modified Dense Graded Surface Course,
www.ces.clemson.edu/arts/DGSC.pdf

Rubber-Modified Open Graded Friction Course,
www.ces.clemson.edu/arts/OGFC.pdf

Sahu, C. S., and Rao, S. K. (1978). "Skip versus continuous gradations of aggregates for asphalt paving mixtures." *Indian Highways*, India, Vol. 9, 20-30.

Sousa, J. B., Craus, J., and Monismith, C. L. (1991). "Summary report on permanent deformation in asphalt concrete." *SHRP-A/IR-91-104*. Strategic Highway Research Program, National Academy of Sciences, Washington, D.C.

Stephens, J. E., and Sinha, K. C. (1978). "Effect of aggregate shape on bituminous mix characteristics." Proc., Assoc. of Asphalt Paving Technologists (AAPT), St. Paul Minn., Vol. 47, 434-456.

Tjan, A., and Adrian, Y. (2003). “*Analysis of Creep Properties of Bituminous Mixture with Constant Rate of Load Increment Method.*” Proceedings of the Eastern Asia Society for Transportation Studies, Vol. 4.

Uzan, J. (2004). “*Permanent Deformation in Flexible Pavement.*” Journal of Transportation Engineering.

APPENDIX

ENDIX 1-a

ulation for determining aggregates proportion for Gap Graded Mixture:

Analysis Result: Coarse Aggregate

Sieve size	Sieve weight (kg)	Weight Retained (kg)						Average weight retained	% by weight retained		
		Trial 1		Trial 2		Trial 3					
		Agg + sieve	Agg	Agg + sieve	Agg	Agg + sieve	Agg				
20	1.384	1.384	0.000	1.393	0.009	1.391	0.007	0.005	0.27		
14	1.135	1.861	0.726	1.867	0.732	1.826	0.691	0.716	35.82		
10	1.106	1.858	0.752	1.845	0.739	1.904	0.798	0.763	38.15		
.36	1.138	1.659	0.521	1.654	0.516	1.640	0.502	0.513	25.65		
00	0.985	0.986	0.001	0.989	0.004	0.987	0.002	0.002	0.12		
								Total	100.00		

Sieve size	Sieve weight (kg)	Weight (kg) and Percentage (%) Passing						Average Weight Passing	Average % Passing		
		Trial 1		Trial 2		Trial 3					
		Weight (kg)	%	Weight (kg)	%	Weight (kg)	%				
20	0.610	2.000	100.00	1.991	99.55	1.993	99.65	1.995	99.73		
14	0.507	1.274	63.70	1.259	62.95	1.302	65.10	1.278	63.92		
10	0.500	0.522	26.10	0.520	26.00	0.504	25.20	0.515	25.77		
.36	0.390	0.001	0.05	0.004	0.20	0.002	0.10	0.002	0.12		
00	0.393	0.000	0.00	0.000	0.00	0.000	0.00	0.000	0.00		

Analysis Result: Fine Aggregate

Sieve size	Sieve weight (kg)	Weight Retained (kg)						Average weight retained	% by weight retained		
		Trial 1		Trial 2		Trial 3					
		Agg + sieve	Agg	Agg + sieve	Agg	Agg + sieve	Agg				
.360	0.389	0.429	0.040	0.436	0.047	0.429	0.040	0.042	8.47		
.600	0.340	0.512	0.172	0.519	0.179	0.525	0.185	0.179	35.73		
.212	0.276	0.530	0.254	0.524	0.248	0.530	0.254	0.252	50.40		
.075	0.255	0.287	0.032	0.279	0.024	0.274	0.019	0.025	5.00		
000	0.393	0.395	0.002	0.395	0.002	0.395	0.002	0.002	0.40		
								Total	100.00		

Sieve size	Sieve weight (kg)	Weight (kg) and Percentage (%) Passing						Average Weight Passing	Average % Passing		
		Trial 1		Trial 2		Trial 3					
		Weight (kg)	%	Weight (kg)	%	Weight (kg)	%				
.360	0.389	0.460	92.00	0.453	90.60	0.460	92.00	0.458	91.53		
.600	0.340	0.288	57.60	0.274	54.80	0.275	55.00	0.279	55.80		
.212	0.276	0.034	6.80	0.026	5.20	0.021	4.20	0.027	5.40		
.075	0.255	0.002	0.40	0.002	0.40	0.002	0.40	0.002	0.40		
000	0.393	0.000	0.00	0.000	0.00	0.000	0.00	0.000	0.00		

culation for determining proportions of aggregates to obtain required gradation

issing Sieve	Average % by weight passing			JKR Standard	
	Coarse (A)	Fine (B)	Filler (C)	Minimum	Maximum
20	99.73	100.00	100.00	100	100
14	63.92	100.00	100.00	85	100
10	25.77	100.00	100.00	60	90
.36	0.12	91.53	100.00	60	72

.6	0.00	55.80	100.00	45	72
.12	0.00	5.40	100.00	15	50
.075	0.00	0.40	80.00	8	12

$$bB + cC = P$$

mix:	1	2
se (%)	35	25
(%)	55	63
(%)	10	12

AL 1 (a=35%, b=55%, c=10%)

ssing eve	Average % by weight passing			
	Coarse (A)	Fine (B)	Filler (C)	Total
20	34.91	55.00	10.00	100
14	35.16	55.00	10.00	100
10	14.17	55.00	10.00	79
.36	0.07	50.34	10.00	60
.6	0.00	30.69	10.00	[REDACTED]
.212	0.00	2.97	10.00	[REDACTED]
.075	0.00	0.22	8.00	8

[REDACTED] total aggregate not in the limit specified by BS 594

AL 2 (a=25%, b=63%, c=12%)

ssing eve	Average % by weight passing			
	Coarse (A)	Fine (B)	Filler (C)	Total
20	24.93	63.00	12.00	100
14	15.98	63.00	12.00	91
10	6.44	63.00	12.00	81
.36	0.03	57.66	12.00	70
.6	0.00	35.15	12.00	47
.212	0.00	3.40	12.00	15
.075	0.00	0.25	9.60	10

From calculation, Trial 2 meets the gradation limit specified by BS 594.

Thus, the proportions of aggregate required to obtain the Gap Graded mix are:

62% coarse aggregate, 35% fine aggregate, and 12% filler

ENDIX 1-b

ulation for determining aggregates proportion for Continuous Graded Mixture:

Analysis Result: Coarse Aggregate

Sieve size	Sieve weight (kg)	Weight Retained (kg)						Average weight retained	% by weight retained		
		Trial 1		Trial 2		Trial 3					
		Agg + sieve	Agg	Agg + sieve	Agg	Agg + sieve	Agg				
1.00	1.495	1.495	0.000	1.495	0.000	1.495	0.000	0.000	0.00		
1.00	1.406	1.432	0.026	1.409	0.003	1.406	0.000	0.010	0.48		
1.00	1.119	1.809	0.690	1.718	0.599	1.613	0.494	0.594	29.72		
1.00	1.105	1.892	0.787	1.790	0.685	1.818	0.713	0.728	36.42		
.30	1.290	1.734	0.444	1.850	0.560	1.829	0.539	0.514	25.72		
.00	0.767	0.820	0.053	0.920	0.153	1.021	0.254	0.153	7.67		
								Total	100.00		

Sieve size	Sieve weight (kg)	Weight (kg) and Percentage (%) Passing						Average Weight Passing	Average % Passing		
		Trial 1		Trial 2		Trial 3					
		Weight (kg)	%	Weight (kg)	%	Weight (kg)	%				
3.00	1.495	2.000	100.00	2.000	100.00	2.000	100.00	2.000	100.00		
1.00	1.406	1.974	98.70	1.997	99.85	2.000	100.00	1.990	99.52		
1.00	1.119	1.284	64.20	1.398	69.90	1.506	75.30	1.396	69.80		
1.00	1.105	0.497	24.85	0.713	35.65	0.793	39.65	0.668	33.38		
.30	1.290	0.053	2.65	0.153	7.65	0.254	12.70	0.153	7.67		
.00	0.767	0.000	0.00	0.000	0.00	0.000	0.00	0.000	0.00		

Analysis Result: Fine Aggregate

Sieve size	Sieve weight (kg)	Weight Retained (kg)						Average weight retained	% by weight retained		
		Trial 1		Trial 2		Trial 3					
		Agg + sieve	Agg	Agg + sieve	Agg	Agg + sieve	Agg				
.300	0.512	0.513	0.001	0.512	0.000	0.513	0.001	0.001	0.13		
.350	0.484	0.498	0.014	0.498	0.014	0.498	0.014	0.014	2.80		
.300	0.280	0.728	0.448	0.734	0.454	0.728	0.448	0.450	90.00		
.075	0.255	0.290	0.035	0.285	0.030	0.290	0.035	0.033	6.67		
.000	0.393	0.395	0.002	0.395	0.002	0.395	0.002	0.002	0.40		
								Total	100.00		

Sieve size	Sieve weight (kg)	Weight (kg) and Percentage (%) Passing						Average Weight Passing	Average % Passing		
		Trial 1		Trial 2		Trial 3					
		Weight (kg)	%	Weight (kg)	%	Weight (kg)	%				
.300	0.512	0.499	99.80	0.500	100.00	0.499	99.80	0.499	99.87		
.350	0.484	0.485	97.00	0.486	97.20	0.485	97.00	0.485	97.07		
.300	0.280	0.037	7.40	0.032	6.40	0.037	7.40	0.035	7.07		
.075	0.255	0.002	0.40	0.002	0.40	0.002	0.40	0.002	0.40		
.000	0.393	0.000	0.00	0.000	0.00	0.000	0.00	0.000	0.00		

ration for determining proportions of aggregates to obtain required gradation

Sieve	Average % by weight passing			JKR Standard	
	Coarse (A)	Fine (B)	Filler (C)	Minimum	Maximum
28	100.00	100.00	100.00	100	100
20	99.52	100.00	100.00	95	100
14	69.80	100.00	100.00	65	85
10	33.38	100.00	100.00	52	72
6.3	7.67	99.87	100.00	39	55
3.5	0.00	97.07	100.00	32	46
0.3	0.00	7.07	100.00	7	21
0.075	0.00	0.40	80.00	2	8

$$bB + cC = P$$

mix:	1	2	3
size (%)	50	52	54
(%)	42	40	38
(%)	8	8	8

AL 1

Sieve	Average % by weight passing			Total
	Coarse (A)	Fine (B)	Filler (C)	
28	50.00	42.00	8.00	100.00
20	49.76	42.00	8.00	99.76
14	34.90	42.00	8.00	84.90
10	16.69	42.00	8.00	66.69
6.3	3.39	41.95	8.00	53.34
3.5	0.00	40.77	8.00	
0.3	0.00	2.97	8.00	10.97
0.075	0.00	0.17	6.40	6.57

total aggregate not in
the limit specified by
JKR Standard

AL 2

Sieve	Average % by weight passing			Total
	Coarse (A)	Fine (B)	Filler (C)	
28	52.00	40.00	8.00	100.00
20	51.75	40.00	8.00	99.75
14	36.30	40.00	8.00	84.30
10	17.36	40.00	8.00	65.36
6.3	3.99	39.95	8.00	51.94
3.5	0.00	38.83	8.00	
0.3	0.00	2.83	8.00	10.83
0.075	0.00	0.16	6.40	6.56

AL 3

Sieve	Average % by weight passing			Total
	Coarse (A)	Fine (B)	Filler (C)	
28	54.00	38.00	8.00	100.00
20	53.74	38.00	8.00	99.74
14	37.69	38.00	8.00	83.69
10	18.03	38.00	8.00	64.03
6.3	4.14	37.95	8.00	50.09

35	0.00	36.89	8.00	44.89
.3	0.00	2.69	8.00	10.69
075	0.00	0.15	6.40	6.55

In calculation, Trial 3 meets the gradation limit specified by JKR.
, the proportions of aggregate requires to obtain the Continuous Graded mix are:

coarse aggregate, 38% fine aggregate, and 8% filler

ENDIX 1-c

ulation for determining aggregates proportion for Open Graded Mixture:

Analysis Result: Coarse Aggregate

Sieve Size	Sieve weight (kg)	Weight Retained (kg)						Average weight retained	% by weight retained		
		Trial 1		Trial 2		Trial 3					
		Agg + sieve	Agg	Agg + sieve	Agg	Agg + sieve	Agg				
.50	1.375	1.380	0.005	1.382	0.007	1.390	0.015	0.009	0.45		
.50	1.358	1.363	0.005	1.381	0.023	1.365	0.007	0.012	0.58		
.75	1.223	2.590	1.367	2.540	1.317	2.470	1.247	1.310	65.52		
.36	1.138	1.757	0.619	1.788	0.650	1.867	0.729	0.666	33.30		
0	0.985	0.989	0.004	0.988	0.003	0.987	0.002	0.003	0.15		
								Total	100.00		

Sieve Size	Sieve weight (kg)	Weight (kg) and Percentage (%) Passing						Average Weight Passing	Average % Passing		
		Trial 1		Trial 2		Trial 3					
		Weight (kg)	%	Weight (kg)	%	Weight (kg)	%				
.50	1.375	1.995	99.75	1.993	99.65	1.985	99.25	1.991	99.55		
.50	1.358	1.990	99.50	1.970	98.50	1.978	98.90	1.979	98.97		
.75	1.223	0.623	31.15	0.653	32.65	0.731	36.55	0.669	33.45		
.36	1.138	0.004	0.20	0.003	0.15	0.002	0.10	0.003	0.15		
0	0.985	0.000	0.00	0.000	0.00	0.000	0.00	0.000	0.00		

Analysis Result: Fine Aggregate

Sieve Size	Sieve weight (kg)	Weight Retained (kg)						Average weight retained	% by weight retained		
		Trial 1		Trial 2		Trial 3					
		Agg + sieve	Agg	Agg + sieve	Agg	Agg + sieve	Agg				
.360	0.389	0.432	0.043	0.440	0.051	0.439	0.050	0.048	9.60		
.075	0.255	0.710	0.455	0.702	0.447	0.703	0.448	0.450	90.00		
.000	0.393	0.395	0.002	0.395	0.002	0.395	0.002	0.002	0.40		
								Total	100.00		

Sieve Size	Sieve weight (kg)	Weight (kg) and Percentage (%) Passing						Average Weight Passing	Average % Passing		
		Trial 1		Trial 2		Trial 3					
		Weight (kg)	%	Weight (kg)	%	Weight (kg)	%				
.360	0.389	0.457	91.40	0.449	89.80	0.450	90.00	0.452	90.40		
.075	0.255	0.002	0.40	0.002	0.40	0.002	0.40	0.002	0.40		

culation for determining proportions of aggregates to obtain required gradation

Passing Sieve	Average % by weight passing		JKR Standard		
	Coarse (A)	Fine (B)	Filler (C)	Minimum	Maximum
12.5	99.55	100.00	100.00	100	100
9.5	98.97	100.00	100.00	95	100
4.75	33.45	100.00	100.00	30	50
2.36	0.15	90.40	100.00	5	15
0.075	0.00	0.40	80.00	2	2.5

$$+ bB + cC = P$$

mix:	1	3
se (%)	70	90
(%)	28	7.5
(%)	2	2.5

L 1 (a=70%, b=28%, c=2%)

sizing size	Average % by weight passing			
	Coarse (A)	Fine (B)	Filler (C)	Total
2.5	69.69	28.00	2.00	100
.5	69.28	28.00	2.00	99
.75	23.42	28.00	2.00	
.36	0.11	25.31	2.00	
075	0.00	0.11	1.60	2

[REDACTED] total aggregate not in the limit specified by FHWA

L 2 (a=90%, b=7.5%, c=2.5%)

sizing size	Average % by weight passing			
	Coarse (A)	Fine (B)	Filler (C)	Total
2.5	89.60	7.50	2.50	100
.5	89.07	7.50	2.50	99
.75	30.11	7.50	2.50	40
.36	0.14	6.78	2.50	9
075	0.00	0.03	2.00	2

From calculation, Trial 2 meets the gradation limit specified by FHWA.
Thus, the proportions of aggregate required to obtain the Open Graded mix are:

coarse aggregate, 7.5% fine aggregate, and 2% filler

ENDIX 1-d

ulation for determining aggregates proportion for Dense Graded Mixture:

Analysis Result: Coarse Aggregates

e Size (mm)	Sieve weight (kg)	Weight (kg) and Percentage (%) Passing						Average weight passing	Average % by weight		
		Sample 1		Sample 2		Sample 3					
		Weight (kg)	%	Weight (kg)	%	Weight (kg)	%				
1.00	1.708	2.000	100.00	2.000	100.00	2.000	100.00	2.000	100.00		
1.00	1.600	1.971	98.55	1.979	98.95	1.933	96.65	1.961	98.05		
1.00	1.281	1.165	58.25	1.176	58.80	1.178	58.90	1.173	58.65		
1.00	1.311	0.330	16.50	0.320	16.00	0.269	13.45	0.306	15.32		
1.00	1.322	0.039	1.95	0.057	2.85	0.042	2.10	0.046	2.30		
0	0.779	0.000	0.00	0.000	0.00	0.000	0.00	0.000	0.00		

Analysis Result: Fine Aggregates

e Size	Sieve weight (kg)	Weight (kg) and Percentage (%) Passing						Average weight passing	Average % by weight		
		Sample 1		Sample 2		Sample 3					
		Weight (kg)	%	Weight (kg)	%	Weight (kg)	%				
350	0.501	0.458	91.60	0.468	93.60	0.474	94.80	0.463	92.60		
180	0.433	0.333	66.60	0.329	65.80	0.097	19.40	0.331	66.20		
425	0.371	0.143	28.60	0.164	32.80	0.003	0.60	0.154	30.70		
150	0.336	0.019	3.80	0.026	5.20	0.000	0.00	0.023	4.50		
075	0.327	0.002	0.40	0.004	0.80	0.000	0.00	0.003	0.60		
000	0.246	0.000	0.00	0.000	0.00	0.000	0.00	0.000	0.00		

ulation for determining proportions of aggregates to obtain required gradation

ssing ieve	Average % by weight passing			JKR Standard	
	Coarse (A)	Fine (B)	Filler (C)	Minimum	Maximum
28	100.00	100.00	100.00	100	100
20	98.05	100.00	100.00	76	100
14	58.65	100.00	100.00	64	89
10	15.32	100.00	100.00	56	81
5	2.30	100.00	100.00	46	71
3.35	0.00	92.60	100.00	32	58
1.18	0.00	66.20	100.00	20	42
0.425	0.00	30.70	100.00	12	28
0.15	0.00	4.50	100.00	6	16
0.075	0.00	0.60	80.00	4	8

$$+ bB + cC = P$$

here $a + b + c = 1 \rightarrow \text{eqn 1}$

ive size (mm):

	Equation:
0.425	$30.7b + 100c = 20$
1.18	$66.2b + 100c = 31$
3.35	$92.6b + 100c = 45$
0.15	$4.50b + 100c = 11$
0.075	$0.60b + 80c = 6$

lulation for determining trial mix

Mix 1

sieve size 0.425 mm & 1.18 mm

$$\begin{aligned} b + 100c &= 20 \\ 3b + 100c &= 31 \end{aligned}$$

Solving these 2 equations, get;
b = 0.31
c = 0.10

Substituting b & c into eqn 1, get;
a = 0.59

Thus, in %: a = 59%, b = 31% and c = 10%

Mix 3

sieve size 0.425 mm & 0.15 mm

$$\begin{aligned} b + 100c &= 20 \\ b + 100c &= 11 \end{aligned}$$

Solving these 2 equations, get;
b = 0.34
c = 0.09

Substituting b & c into eqn 1, get;
a = 0.57

Thus, in %: a = 57%, b = 34% and c = 9%

Mix 5

sieve size 1.18 mm & 3.35 mm

$$\begin{aligned} 2b + 100c &= 31 \\ 3b + 100c &= 45 \end{aligned}$$

Solving these 2 equations, get;
b = 0.53
c = -0.04

Since the c value is -ve, eliminate this mix

Trial Mix 2

Take sieve size 0.425 mm & 3.35 mm

$$\begin{aligned} 30.7b + 100c &= 20 \\ 92.6b + 100c &= 45 \end{aligned}$$

Solving these 2 equations, get;
b = 0.40
c = 0.08

Substituting b & c into eqn 1, get;
a = 0.52

Thus, in %: a = 52%, b = 40% and c = 8%

Trial Mix 4

Take sieve size 0.425 mm & 0.075 mm

$$\begin{aligned} 30.7b + 100c &= 20 \\ 0.60b + 80c &= 6 \end{aligned}$$

Solving these 2 equations, get;
b = 0.42
c = 0.07

Substituting b & c into eqn 1, get;
a = 0.51

Thus, in %: a = 51%, b = 42% and c = 7%

Trial Mix 6

Take sieve size 1.18 mm & 0.15 mm

$$\begin{aligned} 66.2b + 100c &= 31 \\ 4.50b + 100c &= 11 \end{aligned}$$

Solving these 2 equations, get;
b = 0.32
c = 0.10

Substituting b & c into eqn 1, get;
a = 0.58

Thus, in %: a = 58%, b = 32% and c = 10%

Mix 7

sieve size 1.18 mm & 0.075 mm

$$\begin{aligned} 2b + 100c &= 31 \\ 0.6b + 80c &= 6 \end{aligned}$$

Trial Mix 8

Take sieve size 3.35 mm & 0.15 mm

$$\begin{aligned} 92.6b + 100c &= 45 \\ 4.50b + 100c &= 11 \end{aligned}$$

ng these 2 equations, get;

1.36

1.07

; into eqn 1, get;

1.57

, in %: $a = 57\%$, $b = 36\%$ and $c = 7\%$

Solving these 2 equations, get;

$b = 0.39$

$c = 0.09$

b & c into eqn 1, get;

1.52

Thus, in %: $a = 52\%$, $b = 39\%$ and $c = 9\%$

Mix 9

Take sieve size 3.35 mm & 0.075 mm

$$b + 100c = 45$$

$$b + 80c = 6$$

ing these 2 equations, get;

0.41

0.07

c into eqn 1, get;

0.52

, in %: $a = 52\%$, $b = 41\%$ and $c = 7\%$

Trial Mix 10

Take sieve size 0.15 mm & 0.075 mm

$$4.50b + 100c = 11$$

$$0.60b + 80c = 6$$

Solving these 2 equations, get;

$b = 0.94$

$c = 0.07$

b & c into eqn 1, get;

1.01

Since the a value is -ve, eliminate this mix

trial mix can be summarized as follow:

Mix:	1	2	3	4	6	7	8	9
Pass (%)	59	52	57	51	58	57	52	52
Re (%)	31	40	34	42	32	36	39	41
Fr (%)	10	8	9	7	10	7	9	7

IAL 1

Sieve	Average % by weight passing			
	Coarse (A)	Fine (B)	Filler (C)	Total
37.5				
28	59.00	31.00	10.00	100.00
20	57.85	31.00	10.00	98.85
14	34.60	31.00	10.00	75.60
10	9.04	31.00	10.00	
5	1.36	31.00	10.00	
3.35	0.00	28.71	10.00	38.71
1.18	0.00	20.52	10.00	30.52
0.425	0.00	9.52	10.00	19.52
0.15	0.00	1.40	10.00	11.40
0.075	0.00	0.19	8.00	8.19

[redacted] total aggregate not in the limit specified by JKR Standard

IAL 2

Sieve	Average % by weight passing			
	Coarse (A)	Fine (B)	Filler (C)	Total
37.5				
28	52.00	40.00	8.00	100.00
20	50.99	40.00	8.00	98.99

4	30.50	40.00	8.00	78.50
0	7.96	40.00	8.00	
5	1.20	40.00	8.00	49.20
35	0.00	37.04	8.00	45.04
18	0.00	26.48	8.00	34.48
425	0.00	12.28	8.00	20.28
15	0.00	1.80	8.00	9.80
075	0.00	0.24	6.40	6.64

AL 3

Passing sieve	Average % by weight passing			
	Coarse (A)	Fine (B)	Filler (C)	Total
7.5				
28	57.00	34.00	9.00	100.00
20	55.89	34.00	9.00	98.89
14	33.43	34.00	9.00	76.43
10	8.73	34.00	9.00	
5	1.31	34.00	9.00	
3.35	0.00	31.48	9.00	40.48
1.18	0.00	22.51	9.00	31.51
0.425	0.00	10.44	9.00	19.44
0.15	0.00	1.53	9.00	10.53
0.075	0.00	0.20	7.20	7.40

AL 4

Passing sieve	Average % by weight passing			
	Coarse (A)	Fine (B)	Filler (C)	Total
37.5				
28	51.00	42.00	7.00	100.00
20	50.01	42.00	7.00	99.01
14	29.91	42.00	7.00	78.91
10	7.81	42.00	7.00	56.81
5	1.17	42.00	7.00	50.17
3.35	0.00	38.89	7.00	45.89
1.18	0.00	27.80	7.00	34.80
0.425	0.00	12.89	7.00	19.89
0.15	0.00	1.89	7.00	8.89
0.075	0.00	0.25	5.60	5.85

AL 6

Passing sieve	Average % by weight passing			
	Coarse (A)	Fine (B)	Filler (C)	Total
37.5				
28	58.00	32.00	10.00	100.00
20	56.87	32.00	10.00	98.87
14	34.02	32.00	10.00	76.02
10	8.88	32.00	10.00	
5	1.33	32.00	10.00	
3.35	0.00	29.63	10.00	39.63
1.18	0.00	21.18	10.00	31.18
0.425	0.00	9.82	10.00	19.82
0.15	0.00	1.44	10.00	11.44
0.075	0.00	0.19	8.00	

L 7

Sieve	Average % by weight passing			
	Coarse (A)	Fine (B)	Filler (C)	Total
7.5				
.28	57.00	36.00	7.00	100.00
.20	55.89	36.00	7.00	98.89
.14	33.43	36.00	7.00	76.43
.10	8.73	36.00	7.00	
.05	1.31	36.00	7.00	
.35	0.00	33.34	7.00	40.34
.18	0.00	23.83	7.00	30.83
.425	0.00	11.05	7.00	18.05
.15	0.00	1.62	7.00	8.62
.075	0.00	0.22	5.60	5.82

AL 8

Sieve	Average % by weight passing			
	Coarse (A)	Fine (B)	Filler (C)	Total
7.5				
.28	52.00	39.00	9.00	100.00
.20	50.99	39.00	9.00	98.99
.14	30.50	39.00	9.00	78.50
.10	7.96	39.00	9.00	
.05	1.20	39.00	9.00	49.20
.35	0.00	36.11	9.00	45.11
.18	0.00	25.82	9.00	34.82
.425	0.00	11.97	9.00	20.97
.15	0.00	1.76	9.00	10.76
.075	0.00	0.23	7.20	7.43

AL 9

Sieve	Average % by weight passing			
	Coarse (A)	Fine (B)	Filler (C)	Total
.37.5				
.28	52.00	41.00	7.00	100.00
.20	50.99	41.00	7.00	98.99
.14	30.50	41.00	7.00	78.50
.10	7.96	41.00	7.00	
.05	1.20	41.00	7.00	49.20
.35	0.00	37.97	7.00	44.97
.18	0.00	27.14	7.00	34.14
.425	0.00	12.59	7.00	19.59
.15	0.00	1.85	7.00	8.85
.075	0.00	0.25	5.60	5.85

From calculation, Trial 4 meets the gradation limit specified by JKR.

Thus, the proportions of aggregate required to obtain the Asphaltic Concrete are:

% coarse aggregate, 42% fine aggregate, and 7% filler

ENDIX 2

Its from Bouyancy Balance Test and Marshall Stability Test

ip Graded

umen tent	Sample	Weight (g)		Diameter (mm)	Height (mm)			Average height	Marshall Test	
		in air	in water		1	2	3		Load (kN)	Flow (mm)
.5	1	1254.5	699.0	104.98	66.75	66.43	66.30	66.49	6.07	1.82
	2	1236.0	692.5	101.96	68.65	68.02	68.54	68.40	11.43	3.88
	3	1227.0	674.5	101.62	69.57	69.67	70.04	69.76	14.47	3.95
.0	1	1276.5	707.5	104.62	68.72	69.01	69.57	69.10	6.93	5.68
	2	1235.0	688.0	101.87	68.36	68.23	68.37	68.32	10.48	2.78
	3	1262.0	687.0	101.66	72.21	72.49	72.36	72.35	11.86	1.64
.5	1	1254.5	705.5	104.15	65.62	67.04	66.13	66.26	4.03	3.64
	2	1263.0	692.5	101.60	71.48	71.35	71.36	71.40	7.08	1.44
	3	1252.5	679.0	101.50	71.93	71.55	71.98	71.82	7.57	4.49
.0	1	1283.0	708.0	101.54	71.69	72.12	71.84	71.88	7.98	5.16
	2	1248.0	695.0	104.41	66.66	65.54	66.61	66.27	4.51	3.88
	3	1268.0	718.5	101.62	68.56	68.57	69.12	68.75	8.14	5.93
.5	1	1279.5	727.5	101.54	69.53	69.59	68.99	69.37	7.15	6.41
	2	1256.5	710.5	104.62	65.63	65.66	65.46	65.58	3.91	5.09
	3	1318.5	741.0	104.77	69.03	68.60	69.21	68.95	5.04	4.61

mary values obtained from the above table

umen tent	Weight (g)		Diameter (mm)	Height (mm)	Marshall Test	
	in air	in water			Load (kN)	Flow (mm)
5.5	1239.2	688.7	102.85	68.22	10.66	3.22
6.0	1257.8	694.2	102.72	69.92	9.76	3.37
6.5	1256.7	692.3	102.42	69.83	6.23	3.19
7.0	1266.3	707.2	102.52	68.97	6.88	4.99
7.5	1284.8	726.3	103.64	67.97	5.37	5.37

culated VMA and voids in total mix

umen tent	Gbcm	Bulk density	Stability	Flow	VMA	Gmp	Voids in total mix
5.5	2.25	2.25	10.66	3.22	20.07	2.44	7.79
6.0	2.23	2.23	9.76	3.37	19.99	2.42	7.85
6.5	2.23	2.23	6.23	3.19	20.42	2.40	7.08
7.0	2.26	2.26	6.88	4.99	19.78	2.39	5.44
7.5	2.30	2.30	5.37	5.37	19.72	2.37	2.95

continuous Graded

umen tent	Sample	Weight (g)		Diameter (mm)	Height (mm)			Average height	Marshall Test	
		in air	in water		1	2	3		Load (kN)	Flow (mm)
4.0	1	1214.5	703.5	104.80	64.02	62.87	64.24	63.71	13.04	5.76
	2	1229.5	709.5	105.07	65.36	65.57	65.48	65.47	15.92	5.34
	3	1212.5	697.5	101.75	66.36	67.19	66.35	66.63	24.97	2.75
4.5	1	1211.5	697.0	104.61	64.64	63.29	64.30	64.08	10.88	4.91
	2	1243.0	714.0	101.44	64.65	64.93	65.24	64.94	15.77	4.37
	3	1253.5	722.0	104.78	66.60	66.13	66.46	66.40	10.82	5.29
5.0	1	1223.5	708.5	101.41	65.45	65.78	66.65	65.96	15.91	3.78
	2	1225.0	705.5	104.69	65.00	64.81	63.47	64.43	9.66	5.81
	3	1236.0	713.0	101.44	66.06	66.82	65.88	66.25	17.06	3.89
5.5	1	1233.0	707.0	104.58	64.85	63.95	64.12	64.31	11.16	5.41
	2	1240.0	713.5	104.51	63.94	63.94	64.16	64.01	11.03	4.32
	3	1216.0	698.5	104.61	62.94	63.21	63.12	63.09	7.36	2.07

Summary values obtained from the above table

umen tent	Weight (g)		Diameter (mm)	Height (mm)	Marshall Test	
	in air	in water			Load (kN)	Flow (mm)
4.0	1218.8	703.5	103.87	65.27	17.98	4.62
4.5	1236.0	711.0	103.61	65.14	12.49	4.86
5.0	1228.2	709.0	102.51	65.55	14.21	4.49
5.5	1229.7	706.3	104.57	63.80	9.85	3.93

Calculated VMA and voids in total mix

umen tent	Gbcm	Bulk density	Stability	Flow	VMA	Gmp	Voids in total mix
4.0	2.35	2.35	17.98	4.62	14.55	2.49	5.66
4.5	2.36	2.36	12.49	4.86	14.99	2.47	4.45
5.0	2.37	2.37	14.21	4.49	14.72	2.46	3.84
5.5	2.35	2.35	9.85	3.93	15.88	2.44	3.70

ven Graded

umen tent	Sample	Weight (g)		Diameter (mm)	Height (mm)			Average height	Marshall Test	
		in air	in water		1	2	3		Load (kN)	Flow (mm)
.0	1	1207.5	687.0	104.10	79.55	79.68	79.73	79.65	2.75	4.81
	2	1247.0	716.5	104.27	80.07	81.03	80.57	80.56	4.18	5.27
	3	1223.0	722.0	101.60	82.70	82.71	82.80	82.74	6.75	6.27
.5	1	1221.5	713.0	101.71	83.12	82.88	83.41	83.14	5.13	2.72
	2	1233.5	719.5	101.65	82.36	83.25	82.80	82.80	8.42	2.24
	3	1218.5	709.5	104.78	78.10	79.21	78.64	78.65	2.55	4.16
.0	1	1227.5	711.5	104.76	80.37	80.11	80.33	80.27	1.71	4.59
	2	1239.5	721.0	101.61	84.44	84.65	84.83	84.64		
	3	1226.0	710.0	101.68	83.68	83.20	83.68	83.52		
.5	1	1247.5	716.0	101.74	82.52	82.34	82.16	82.34	4.34	2.30
	2	1229.5	706.5	101.64	81.84	82.21	82.10	82.05	3.83	3.02
	3	1228.5	707.5	101.05	84.07	84.31	83.88	84.09	3.13	2.24
.0	1	1269.0	722.5	104.81	81.42	80.52	80.68	80.87	2.23	4.67
	2	1240.0	704.5	104.81	80.09	80.36	79.71	80.05	1.52	5.75
	3	1246.0	713.5	101.49	85.02	84.66	84.59	84.76	2.74	4.48

mary values obtained from the above table

umen tent	Weight (g)		Diameter (mm)	Height (mm)	Marshall Test	
	in air	in water			Load (kN)	Flow (mm)
3.0	1225.8	708.5	103.32	80.98	4.56	5.45
3.5	1224.5	714.0	102.71	81.53	5.37	3.04
4.0	1231.0	714.2	102.68	82.81	1.71	4.59
4.5	1235.2	710.0	101.48	82.83	3.77	2.52
5.0	1251.7	713.5	103.70	81.89	2.16	4.97

ulated VMA and voids in total mix

umen tent	Gbcm	Bulk density	Stability	Flow	VMA	Gmp	Voids in total mix
3.0	2.37	2.37	4.56	5.45	15.79	2.53	6.32
3.5	2.40	2.40	5.37	3.04	15.16	2.51	4.38
4.0	2.38	2.38	1.71	4.59	16.31	2.49	4.42
4.5	2.35	2.35	3.77	2.52	17.79	2.47	4.86
5.0	2.33	2.33	2.16	4.97	18.92	2.46	5.28

Unfused Graded

Volumen content	Sample	Weight (g)		Diameter (mm)	Height (mm)			Average height	Marshall Test	
		in air	in water		1	2	3		Load (kN)	Flow (mm)
3.0	1	1251.0	706.0							
	2	1235.5	700.0	101.68	69.71	68.61	68.67	69.00	10.36	2.61
	3	1201.0	665.0							
3.5	1	1242.5	717.0	104.93	64.86	66.29	65.33	65.49	10.63	5.39
	2	1230.0	710.0	101.58	67.57	68.01	67.96	67.85	17.63	
	3	1226.0	709.5	104.91	62.97	63.10	64.85	63.64	9.09	1.62
4.0	1	1218.0	703.5	104.85	63.18	64.86	63.39	63.81	10.45	0.42
	2	1225.0	710.0	101.60	65.39	66.16	66.60	66.05	20.77	0.07
	3	1234.0	715.5	105.17	64.32	64.70	64.30	64.44	11.18	0.39
4.5	1	1237.5	717.5	101.57	67.11	67.02	66.76	66.96	17.05	1.04
	2	1237.0	720.5	101.53	66.05	65.25	65.64	65.65	20.97	
	3	1199.0	693.5	101.64	67.68	68.45	68.07	68.07	24.99	2.20
5.0	1	1218.0	709.0	101.58	65.34	64.19	65.65	65.06	16.44	1.81
	2	1210.0	703.0	105.16	62.31	61.49	63.19	62.33	10.93	4.68
	3	1204.0	695.0	101.57	65.19	65.20	65.27	65.22	15.97	1.88
5.5	1	1267.5	741.5	104.67	63.09	62.94	63.77	63.27	9.66	4.44
	2	1252.5	729.0	101.51	65.94	65.69	65.69	65.77	18.64	3.63
	3	1224.5	715.0	104.82	61.23	60.95	61.02	61.07	10.18	4.13
6.0	1	1234.0	719.5	104.71	61.28	62.08	63.20	62.19	8.52	6.00
	2	1248.5	730.5	101.64	65.92	65.05	65.43	65.47	13.11	4.10
	3	1240.5	726.0	104.83	61.76	61.02	61.33	61.37	9.46	4.85
6.5	1	1199.0	694.0	104.66	62.40	62.21	60.87	61.83	8.18	6.30
	2	1221.0	711.0	105.15	60.78	61.04	60.90	60.91	9.87	6.12
	3	1243.5	725.5	101.95	65.64	65.86	66.65	66.05	10.63	4.26

Summary values obtained from the above table

Volumen content	Weight (g)		Diameter (mm)	Height (mm)	Marshall Test	
	in air	in water			Load (kN)	Flow (mm)
3.0	1229.2	690.3	101.68	69.00	10.36	2.61
3.5	1232.8	712.2	103.81	65.66	12.45	3.51
4.0	1225.7	709.7	103.87	64.77	14.13	0.29
4.5	1224.5	710.5	101.58	66.89	21.00	1.62
5.0	1210.7	702.3	102.77	64.20	14.45	2.79
5.5	1248.2	728.5	103.67	63.37	12.83	4.07
6.0	1241.0	725.3	103.73	63.01	10.36	4.98
6.5	1221.2	710.2	103.92	62.93	9.56	5.56

Calculated VMA and voids in total mix

Volumen content	Gbcm	Bulk density	Stability	Flow	VMA	Gmp	Voids in total mix
3.0	2.28	2.28	10.36	2.61	16.23	2.53	9.88
3.5	2.37	2.37	12.45	3.51	13.37	2.51	5.58
4.0	2.38	2.38	14.13	0.29	13.45	2.49	4.42
4.5	2.38	2.38	21.00	1.62	13.91	2.47	3.64
5.0	2.38	2.38	14.45	2.79	14.36	2.46	3.25
5.5	2.40	2.40	12.83	4.07	14.09	2.44	1.64
6.0	2.41	2.41	10.36	4.98	14.19	2.42	0.41
6.5	2.39	2.39	9.56	5.56	15.35	2.40	0.42

APPENDIX 3

Calculation for determining VMA and voids in total mix:

E.g.: For 5.5 % bitumen content Gap graded mixture

→ Bulk specific gravity of compacted mixture, G_{bcm}

$$G_{bcm} = \frac{W_a}{W_a - W_w}$$

Where W_a = weight in air

W_w = weight in water

$$G_{bcm} = \frac{1239.2}{1239.2 - 688.7} = 2.25$$

→ Therefore, the bulk density is $2.25 \times 1.0 \text{ g/cm}^3 = 2.25 \text{ g/cm}^3$

Where 1.0 g/cm^3 is the density of water

→ Percent voids in the mineral aggregate, VMA

$$VMA = 100 - \frac{G_{bcm} * P_{ta}}{G_{bam}}$$

$$G_{bam} = \frac{P_{ca} + P_{fa} + P_{mf}}{\frac{P_{ca}}{G_{bca}} + \frac{P_{fa}}{G_{bfa}} + \frac{P_{mf}}{G_{bmf}}}$$

Where G_{bam} = bulk specific gravity of aggregates in the paving mixture (asphalt concrete)

P_{ta} = aggregate percent by weight of total paving mixture (asphalt concrete)

P_{ca} = percent by weight of coarse aggregate

P_{fa} = percent by weight of fine aggregate

P_{mf} = percent by weight of mineral filler

G_{bca} = bulk specific gravity of coarse aggregate

G_{bfa} = bulk specific gravity of fine aggregate

G_{bmf} = bulk specific gravity of mineral filler

Determine P_{ca} , P_{fa} and P_{mf} in terms of total aggregates:

$$P_{ca} = 0.25 \times 94.5 = 23.63$$

$$P_{fa} = 0.63 \times 94.5 = 59.54$$

$$P_{mf} = 0.12 \times 94.5 = 11.34$$

$$\text{Therefore, } G_{bam} = \frac{\frac{23.63 + 59.54 + 11.34}{23.63}}{\frac{23.63}{2.61} + \frac{59.54}{2.60} + \frac{11.34}{3.15}} = 2.66$$

$$\text{and } VMA = 100 - \frac{2.25 * 94.5}{2.66} = 20.07 \%$$

→ Percent air voids in total mix, P_{av}

$$P_{av} = 100 \frac{G_{mp} - G_{bcm}}{G_{mp}}$$

Where G_{mp} = maximum specific gravity of the compacted paving mixture

$$G_{mp} = \frac{100}{\frac{P_{ta}}{G_{ea}} + \frac{P_{ac}}{G_{ac}}}$$

Where G_{ea} = effective specific gravity of aggregates (assumed to be constant for different asphalt cement contents) = 2.65

G_{ac} = specific gravity of asphalt = 1.03

$$\text{Therefore, } G_{mp} = \frac{100}{\frac{94.5}{2.65} + \frac{5.5}{1.03}} = 2.44$$

$$\text{and } P_{av} = 100 \frac{2.44 - 2.25}{2.44} = 7.79 \%$$

Result from Dynamic Creep Test:

Cycle	Creep Stiffness (Smix)												Dense Graded				
	Gap Graded			Continuous Graded			Open Graded			Average			Average				
	1	2	3	Average	1	2	3	Average	1	2	3	Average	1	2	3		
0	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000		
1	41.123	21.600	54.514	39.079	17.161	208.949	21.261	82.457	55.501	43.460	42.601	47.187	511.148	288.718	63.176	287.681	
2	30.260	17.088	37.735	28.361	14.770	157.778	15.810	62.786	40.224	32.879	31.913	35.005	392.276	208.245	51.084	217.202	
3	6	25.574	15.056	30.983	23.871	13.641	138.056	13.015	54.904	33.924	28.550	27.529	30.001	337.358	168.068	45.795	183.740
4	8	22.772	14.002	27.044	21.273	12.950	119.862	11.536	48.116	30.028	25.744	24.810	26.861	311.236	146.146	42.027	166.470
5	10	20.980	13.203	24.504	19.562	12.619	110.360	10.614	44.631	27.701	23.993	23.019	24.504	276.526	132.902	39.871	150.433
6	12	19.775	12.509	22.619	18.301	12.165	103.363	9.911	41.813	25.658	22.662	21.378	23.233	270.091	120.472	38.284	142.949
7	14	18.652	11.977	20.977	17.202	11.867	98.117	9.325	40.103	24.198	21.565	20.452	22.072	254.648	109.767	37.019	133.811
8	16	17.855	11.691	19.904	16.483	11.588	93.909	8.866	38.121	23.182	20.781	19.655	21.206	242.691	106.087	35.935	128.238
9	18	17.335	11.262	18.913	15.837	11.437	90.595	8.413	36.815	22.312	20.160	18.793	20.422	229.392	100.877	35.087	121.785
10	20	16.644	11.017	18.164	16.275	11.306	86.886	7.967	35.380	21.681	19.474	18.127	19.761	221.946	98.259	34.300	117.495
11	22	16.157	10.745	17.442	14.781	11.092	85.427	7.647	34.722	20.977	18.833	17.741	19.184	221.946	93.551	33.546	116.348
12	24	15.854	10.533	16.752	14.380	11.009	81.811	7.352	33.391	20.265	18.664	17.248	18.726	211.970	90.359	33.204	111.844
13	26	15.415	10.336	16.340	14.030	10.800	79.702	7.083	32.528	19.644	18.084	16.789	18.172	208.245	87.398	32.453	109.365
14	28	15.042	10.193	15.935	13.723	10.708	76.007	6.891	31.869	19.227	17.712	16.562	17.834	196.138	85.314	31.947	104.466
15	30	14.642	10.008	15.446	13.365	10.570	75.498	6.900	30.989	18.902	17.412	16.123	17.470	197.727	82.386	31.592	103.902
16	32	14.353	9.874	15.175	13.134	10.477	74.697	6.882	30.685	18.518	17.153	15.856	17.176	192.479	80.415	31.010	101.301
17	34	14.160	9.782	14.700	12.881	10.453	72.593	6.836	29.961	18.169	16.892	15.525	16.862	186.063	78.251	30.838	98.384
18	36	13.917	9.616	14.443	12.659	10.274	70.928	6.872	29.358	17.834	16.590	15.339	16.588	182.018	77.022	30.725	96.588
19	38	13.671	9.527	14.099	12.432	10.199	69.374	6.860	28.811	17.551	16.393	15.085	16.343	182.682	75.706	30.172	96.187
20	40	13.516	9.417	13.781	12.238	10.079	69.028	6.845	28.651	17.296	16.204	14.869	16.123	176.913	73.769	30.068	93.583
21	42	13.277	9.326	13.593	12.065	10.099	68.146	6.833	28.359	16.999	15.908	14.744	15.884	176.913	73.392	29.694	93.333
22	44	13.148	9.231	13.412	11.930	10.014	66.099	6.845	27.653	16.759	15.714	14.380	15.618	173.898	72.085	29.382	91.788
23	46	12.961	9.155	13.144	11.753	9.917	69.984	6.779	27.227	16.543	15.540	14.281	15.455	169.765	70.360	28.972	89.699
24	48	12.904	9.033	12.936	11.624	10.029	64.440	6.740	27.070	16.397	15.294	14.069	15.263	165.798	69.197	29.027	88.007
25	50	12.614	9.071	12.745	11.477	9.824	63.893	6.755	26.824	16.255	15.309	13.892	15.152	164.772	68.350	28.933	87.352
26	52	12.610	8.897	11.385	9.741	62.612	6.740	26.364	16.040	15.130	13.636	14.935	162.191	66.983	28.534	85.903	
27	54	12.380	8.856	12.418	11.218	9.725	61.358	6.734	25.939	15.873	14.880	13.665	14.806	160.064	66.693	28.287	85.015
28	56	12.262	8.800	12.936	11.624	9.682	61.358	6.725	25.922	15.666	14.811	13.457	14.645	156.501	66.129	28.302	83.644
29	58	12.156	8.696	12.109	10.987	9.590	60.396	6.740	25.575	15.478	14.640	13.353	14.490	158.554	65.634	28.011	84.066
30	60	12.089	8.672	11.858	10.873	9.574	6.734	25.269	15.279	14.532	13.204	14.338	157.073	64.148	27.771	82.897	
31	62	11.986	8.619	11.769	10.791	9.576	58.550	6.670	24.932	15.129	14.437	13.059	14.208	152.233	63.413	27.743	81.130
32	64	11.876	8.595	11.628	10.700	9.449	58.327	6.687	24.821	15.055	14.273	12.961	14.096	151.412	62.706	27.452	80.523
33	66	11.841	8.568	11.507	10.639	9.500	57.453	6.705	24.553	15.027	14.238	12.976	14.080	152.789	62.243	27.417	80.816
34	68	11.659	8.463	11.372	10.498	9.394	57.253	6.646	24.431	14.883	14.121	12.737	13.814	146.677	61.790	27.383	78.617
35	70	11.608	8.370	11.168	10.382	9.377	56.414	6.664	24.152	14.712	13.988	12.602	13.767	144.886	60.674	27.151	77.570
36	72	11.585	8.362	11.084	10.344	9.331	56.004	6.679	24.005	14.645	14.010	12.590	13.748	143.648	60.239	27.261	77.049
37	74	11.482	8.322	10.974	10.259	9.298	55.600	6.644	23.847	14.519	13.880	12.429	13.809	142.430	59.165	26.774	76.123
38	76	11.397	8.240	10.932	10.190	9.301	55.420	6.638	23.786	14.357	13.726	12.330	13.471	142.430	59.186	26.538	76.051

Cycle	Gap Graded			Continuous Graded			Open Slotted			Average		
	1	2	3	Average	1	2	3	Average	1	2	3	
78	11.286	8.229	10.818	10.111	9.142	54.614	6.659	23.472	14.181	13.708	12.204	140.566
80	11.293	8.263	10.755	10.104	9.146	54.253	6.618	23.339	14.147	13.557	12.137	137.137
82	11.140	8.127	10.640	9.969	9.160	53.875	6.618	23.218	14.087	13.540	12.052	132.226
84	11.032	8.117	10.504	9.984	9.117	53.291	6.613	23.007	13.968	13.416	11.920	131.101
86	11.041	8.051	10.419	9.837	9.054	53.135	6.599	22.929	13.788	13.452	11.876	130.039
88	11.002	8.036	10.384	9.807	9.099	52.385	6.599	22.694	13.744	13.307	11.766	129.939
90	10.975	8.012	10.294	9.760	9.032	51.856	6.593	22.494	13.725	13.261	11.760	129.915
92	10.778	7.978	10.213	9.656	9.000	51.682	6.611	22.431	13.557	13.141	11.606	12.768
94	10.753	7.954	10.179	9.629	8.984	51.713	6.608	22.435	13.516	13.096	11.519	12.710
96	10.700	7.890	10.054	9.548	8.902	51.030	6.599	22.177	13.381	13.048	11.530	12.653
98	10.621	7.893	9.937	9.484	8.968	50.682	6.568	22.066	13.288	12.983	11.445	12.572
100	10.612	7.822	9.919	9.451	8.951	50.495	6.591	21.979	13.348	12.939	11.363	128.296
102	10.576	7.856	9.856	9.429	8.861	49.842	6.528	21.744	13.141	12.845	11.242	12.409
104	10.449	7.793	9.763	9.335	8.801	49.842	6.580	21.741	13.074	12.853	11.265	12.397
106	10.530	7.762	9.708	9.333	8.786	49.010	6.574	21.457	13.048	12.707	11.172	12.309
108	10.391	7.743	9.660	9.265	8.790	49.086	6.597	21.491	12.983	12.705	11.120	12.269
110	10.404	7.716	9.562	9.227	8.736	48.738	6.540	21.338	12.907	12.703	11.088	12.233
112	10.315	7.666	9.535	9.172	8.741	48.816	6.534	21.364	12.792	12.612	10.966	12.123
114	10.260	7.677	9.408	9.115	8.775	48.128	6.555	21.153	12.718	12.600	10.869	12.062
116	10.220	7.643	9.382	9.082	8.687	48.510	6.555	21.251	12.686	12.520	10.800	12.002
118	10.193	7.583	9.368	9.048	8.672	47.492	6.520	20.895	12.684	12.390	10.790	11.955
120	10.139	7.684	9.270	9.031	8.657	47.430	6.538	20.875	12.641	12.380	10.745	11.922
122	10.106	7.542	9.215	8.954	8.599	47.871	6.538	21.003	12.540	12.282	10.636	11.819
124	10.067	7.513	9.176	8.919	8.575	46.714	6.535	20.608	12.480	12.301	10.627	11.803
126	10.067	7.521	9.148	8.912	8.590	46.812	6.478	20.627	12.360	12.272	10.576	11.736
128	10.054	7.473	9.146	8.891	8.537	46.529	6.501	20.522	12.330	12.213	10.560	11.701
130	9.970	7.451	9.011	8.811	8.557	46.389	6.521	20.489	12.350	12.213	10.476	11.680
132	9.944	7.470	9.009	8.808	8.538	46.111	6.519	20.389	12.233	12.118	10.426	11.592
134	9.828	7.441	8.927	8.732	8.481	46.156	6.487	20.375	12.252	12.128	10.356	11.579
136	9.856	7.452	8.942	8.750	8.500	45.973	6.510	20.328	12.165	11.985	10.247	11.466
138	9.760	7.409	8.876	8.682	8.448	45.882	6.505	20.278	12.089	11.986	10.233	11.436
140	9.729	7.410	8.761	8.633	8.454	45.431	6.476	20.120	12.014	11.949	11.399	11.286
142	9.737	7.334	8.760	8.610	8.431	44.641	6.471	19.848	11.948	11.903	10.159	11.337
144	9.739	7.349	8.741	8.610	8.388	44.901	6.468	19.919	12.006	11.960	10.179	11.382
146	9.702	7.299	8.785	8.595	8.375	44.641	6.489	19.835	11.912	11.839	10.033	11.261
148	9.716	7.282	8.711	8.570	8.343	44.384	6.435	19.721	11.875	11.785	10.166	11.257
150	9.604	7.272	8.566	8.481	8.372	44.129	6.480	19.680	11.821	11.732	10.008	11.187
152	9.574	7.276	8.562	8.471	8.377	44.100	6.455	19.644	11.758	11.723	9.989	11.157
154	9.594	7.256	8.553	8.468	8.265	43.927	6.449	19.547	11.769	11.661	9.905	11.135
156	9.521	7.214	8.486	8.407	8.280	44.100	6.447	19.609	11.716	11.588	9.913	11.072
158	9.449	7.314	8.454	8.406	8.243	43.679	6.498	19.473	11.580	11.591	9.842	11.004
160	9.494	7.215	8.455	8.388	8.249	43.506	6.439	19.398	11.536	11.556	9.760	10.951
162	9.440	7.195	8.361	8.332	8.269	43.556	6.436	19.420	11.519	11.505	9.762	10.929
164	9.411	7.178	8.386	8.325	8.223	43.191	6.459	19.291	11.451	11.442	9.788	10.894

Cycle	Gap Graded			Continuous Graded			Open Graded			Average	
	1	2	3	Average	1	2	3	Average	1	2	3
166	9.357	7.133	8.355	8.282	8.247	43.070	6.431	19.249	11.408	11.453	9.720
168	9.295	7.124	8.286	8.235	8.192	43.191	6.456	19.280	11.437	11.428	9.678
170	9.331	7.160	8.255	8.249	8.207	42.376	6.397	18.993	11.423	11.394	9.604
172	9.357	7.144	8.257	8.253	8.153	42.764	6.445	19.121	11.353	11.408	9.562
174	9.250	7.081	8.194	8.175	8.149	42.427	6.417	18.998	11.356	11.266	9.559
176	9.253	7.118	8.164	8.178	8.132	41.913	6.417	18.821	11.262	11.278	9.449
178	9.195	7.039	8.198	8.144	8.151	42.080	6.386	18.872	11.257	11.298	9.414
180	9.229	7.036	8.112	8.126	8.170	41.965	6.409	18.848	11.196	11.245	9.448
182	9.218	7.103	8.047	8.123	8.089	42.017	6.406	18.837	11.120	11.152	9.414
184	9.160	7.035	8.017	8.071	8.108	41.737	6.378	18.741	11.176	11.172	9.320
186	9.128	7.019	8.115	8.087	8.091	41.565	6.398	18.695	11.180	11.132	9.340
188	9.106	6.985	7.995	8.029	8.115	41.290	6.421	18.609	11.088	11.116	9.275
190	9.121	6.993	7.989	8.034	8.030	41.343	6.419	18.597	10.977	11.076	9.237
192	9.063	7.011	7.950	8.008	8.005	41.070	6.416	18.497	10.989	11.052	9.204
194	9.042	6.968	7.925	7.978	8.068	40.960	6.416	18.481	10.958	11.029	9.207
196	9.026	6.928	7.924	7.959	7.992	41.123	6.416	18.510	10.971	10.997	9.216
198	9.036	6.937	7.857	7.943	7.984	40.636	6.411	18.344	10.866	10.958	9.148
200	8.949	6.961	7.814	7.908	8.003	40.905	6.383	18.430	10.886	10.892	9.110
202	8.928	6.918	7.844	7.897	7.943	40.423	6.377	18.248	10.762	10.869	9.042
204	8.943	6.912	7.816	7.890	7.974	40.371	6.375	18.240	10.740	10.853	9.117
206	8.952	6.897	7.835	7.885	7.926	40.266	6.375	18.189	10.763	10.838	8.984
208	8.897	6.918	7.777	7.864	7.950	40.317	6.395	18.221	10.800	10.878	9.019
210	8.841	6.902	7.720	7.821	7.942	39.851	6.372	18.055	10.678	10.778	8.927
212	8.856	6.857	7.731	7.815	7.925	39.851	6.344	18.040	10.642	10.775	8.897
214	8.876	6.881	7.704	7.820	7.944	39.592	6.389	17.975	10.556	10.768	8.861
216	8.825	6.809	7.673	7.769	7.932	39.490	6.379	17.934	10.632	10.695	8.796
218	8.805	6.863	7.688	7.785	7.881	39.545	6.384	17.937	10.526	10.688	8.800
220	8.751	6.848	7.579	7.726	7.850	39.490	6.354	17.898	10.477	10.735	8.770
222	8.736	6.835	7.613	7.728	7.877	39.189	6.381	17.816	10.504	10.706	8.741
224	8.716	6.767	7.538	7.674	7.857	39.244	6.328	17.810	10.462	10.600	8.731
226	8.736	6.817	7.576	7.710	7.818	38.948	6.376	17.714	10.482	10.627	8.682
228	8.746	6.779	7.550	7.692	7.864	38.892	6.348	17.701	10.453	10.605	8.609
230	8.716	6.770	7.484	7.657	7.829	39.046	6.321	17.732	10.329	10.583	8.633
232	8.682	6.728	7.484	7.631	7.840	38.948	6.393	17.778	10.343	10.562	8.638
234	8.662	6.743	7.503	7.636	7.767	38.446	6.338	17.517	10.322	10.574	8.576
236	8.599	6.764	7.467	7.610	7.782	38.656	6.366	17.601	10.261	10.511	8.600
238	8.624	6.734	7.489	7.616	7.739	38.615	6.366	17.573	10.308	10.504	8.543
240	8.614	6.720	7.372	7.569	7.778	38.407	6.363	17.516	10.226	10.510	8.477
242	8.600	6.758	7.326	7.555	7.731	38.463	6.360	17.518	10.206	10.406	8.463
244	8.624	6.702	7.359	7.582	7.777	38.368	6.335	17.493	10.193	10.426	8.440
246	8.600	6.697	7.382	7.580	7.739	37.841	6.358	17.313	10.146	10.371	8.413
248	8.547	6.708	7.332	7.529	7.727	38.027	6.305	17.353	10.173	10.432	8.385
250	8.538	6.647	7.336	7.507	7.685	37.783	6.327	17.265	10.133	10.363	8.329
252	8.476	6.644	7.290	7.470	7.700	37.893	6.297	17.288	10.073	10.356	8.300

Cycle	Gap Graded			Continuous Graded			Open Graded			Average	
	1	2	3	Average	1	2	3	Average	1	2	3
254	8.486	6.685	7.273	7.481	7.689	37.805	6.347	17.280	10.093	10.329	8.267
256	8.520	6.645	7.278	7.481	7.693	37.748	6.317	17.253	10.054	10.267	8.245
258	8.472	6.613	7.264	7.450	7.677	37.472	6.322	17.157	9.989	10.328	8.270
260	8.463	6.660	7.212	7.445	7.670	37.564	6.342	17.192	9.963	10.220	8.229
262	8.449	6.622	7.227	7.433	7.605	37.656	6.367	17.209	9.898	10.253	8.194
264	8.431	6.613	7.172	7.405	7.658	37.439	6.315	17.137	9.873	10.274	8.140
266	8.384	6.602	7.162	7.383	7.609	36.964	6.337	16.970	9.886	10.172	8.114
268	8.413	6.591	7.152	7.385	7.632	37.316	6.337	17.095	9.828	10.233	8.157
270	8.385	6.585	7.129	7.366	7.598	37.406	6.309	17.104	9.791	10.159	8.108
272	8.372	6.556	7.078	7.335	7.583	37.080	6.307	16.990	9.882	10.152	8.030
274	8.358	6.545	7.086	7.330	7.636	37.137	6.284	17.019	9.825	10.106	8.049
276	8.307	6.563	7.067	7.312	7.591	36.845	6.329	16.922	9.749	10.119	8.032
278	8.355	6.565	7.057	7.322	7.561	36.815	6.302	16.893	9.698	10.093	8.003
280	8.317	6.552	7.007	7.292	7.542	36.727	6.302	16.857	9.706	10.093	7.978
282	8.313	6.538	7.050	7.300	7.557	36.640	6.324	16.840	9.644	10.067	7.966
284	8.286	6.568	7.006	7.283	7.554	36.640	6.324	16.839	9.658	10.093	7.973
286	8.310	6.496	6.978	7.261	7.520	36.382	6.296	16.733	9.646	10.080	7.922
288	8.227	6.501	6.941	7.223	7.535	36.382	6.346	16.754	9.666	9.968	7.951
290	8.218	6.510	6.977	7.235	7.532	36.488	6.321	16.774	9.598	9.995	7.900
292	8.205	6.476	6.903	7.195	7.521	36.153	6.294	16.656	9.568	9.969	7.845
294	8.238	6.494	6.942	7.225	7.517	36.068	6.316	16.634	9.519	9.969	7.852
296	8.248	6.491	6.869	7.203	7.473	36.068	6.291	16.611	9.565	9.917	7.801
298	8.166	6.486	6.881	7.178	7.425	36.042	6.311	16.593	9.509	9.976	7.812
300	8.190	6.470	6.836	7.165	7.455	35.959	6.261	16.558	9.480	9.885	7.781
302	8.136	6.464	6.845	7.148	7.444	35.959	6.309	16.571	9.474	9.879	7.762
304	8.164	6.459	6.860	7.161	7.466	35.651	6.286	16.468	9.483	9.866	7.742
306	8.155	6.425	6.818	7.133	7.459	35.509	6.306	16.425	9.368	9.886	7.693
308	8.174	6.471	6.806	7.150	7.448	35.509	6.256	16.404	9.405	9.867	7.692
310	8.125	6.443	6.788	7.119	7.441	35.486	6.303	16.410	9.340	9.842	7.643
312	8.085	6.457	6.746	7.096	7.437	35.627	6.276	16.447	9.323	9.830	7.617
314	8.072	6.449	6.767	7.096	7.397	35.405	6.301	16.368	9.368	9.778	7.591
316	8.127	6.388	6.770	7.095	7.387	35.464	6.301	16.384	9.309	9.819	7.512
318	8.083	6.403	6.705	7.064	7.376	35.243	6.271	16.297	9.267	9.774	7.610
320	8.062	6.405	6.720	7.062	7.372	35.243	6.273	16.296	9.270	9.755	7.532
322	8.021	6.420	6.676	7.039	7.358	35.162	6.248	16.256	9.259	9.808	7.513
324	8.017	6.440	6.691	7.049	7.355	35.141	6.296	16.264	9.257	9.757	7.521
326	8.072	6.384	6.653	7.036	7.351	35.003	6.293	16.216	9.209	9.656	7.470
328	8.028	6.376	6.673	7.026	7.337	34.923	6.293	16.184	9.176	9.650	7.500
330	8.016	6.343	6.656	7.005	7.334	35.062	6.261	16.219	9.124	9.676	7.430
332	8.003	6.363	6.616	6.994	7.320	35.200	6.263	16.261	9.086	9.620	7.402
334	7.963	6.333	6.628	6.975	7.320	34.747	6.293	16.120	9.070	9.634	7.432
336	8.006	6.350	6.590	6.982	7.274	34.884	6.285	16.148	9.054	9.648	7.330
338	8.006	6.390	6.605	7.000	7.332	34.747	6.233	16.104	9.099	9.622	7.339
340	7.993	6.365	6.565	6.974	7.296	34.747	6.283	16.109	9.058	9.586	7.318

Appendix 4
Dynamic Creep Test results

Cycle	Gap Graded			Continuous Graded			Open graded			Average		
	1	2	3	Average	1	2	3	Average	1	2	3	
342	7.950	6.327	6.580	6.952	7.289	34.514	6.283	16.029	9.032	9.530	7.268	8.610
344	7.958	6.329	6.563	6.950	7.250	34.377	6.231	15.953	9.005	9.513	7.269	8.596
346	7.894	6.344	6.557	6.932	7.300	34.360	6.228	15.963	8.984	9.501	7.274	8.586
348	7.948	6.314	6.543	6.935	7.261	34.377	6.253	15.964	8.979	9.527	7.232	8.579
350	7.882	6.309	6.561	6.917	7.254	34.437	6.250	15.980	8.943	9.498	7.240	8.560
352	7.932	6.323	6.490	6.915	7.309	34.225	6.250	15.928	8.917	9.561	7.188	8.555
354	7.889	6.321	6.508	6.906	7.269	34.208	6.250	15.909	8.967	9.494	7.193	8.551
356	7.916	6.315	6.522	6.918	7.288	34.073	6.273	15.878	8.871	9.489	7.173	8.511
358	7.904	6.280	6.455	6.880	7.227	34.149	6.223	15.866	8.871	9.471	7.132	8.491
360	7.834	6.303	6.498	6.878	7.256	34.073	6.245	15.858	8.861	9.417	7.109	8.462
362	7.853	6.273	6.487	6.871	7.239	33.923	6.270	15.811	8.865	9.368	7.086	8.440
364	7.822	6.240	6.484	6.849	7.204	33.567	6.268	15.680	8.810	9.394	7.032	8.412
366	7.806	6.250	6.437	6.831	7.225	33.700	6.240	15.722	8.751	9.340	7.013	8.368
368	7.790	6.252	6.424	6.822	7.197	33.700	6.240	15.712	8.770	9.414	6.994	8.393
370	7.817	6.225	6.444	6.829	7.187	33.687	6.240	15.705	8.711	9.360	7.003	8.358
372	7.809	6.220	6.411	6.813	7.205	33.834	6.243	15.761	8.736	9.343	6.963	8.347
374	7.763	6.210	6.369	6.781	7.170	33.687	6.268	15.708	8.716	9.315	6.941	8.324
376	7.797	6.230	6.359	6.795	7.167	33.481	6.263	15.837	8.706	9.303	6.956	8.322
378	7.812	6.252	6.379	6.814	7.157	33.687	6.263	15.702	8.643	9.323	6.949	8.305
380	7.816	6.220	6.341	6.792	7.122	33.336	6.260	15.573	8.662	9.275	6.882	8.273
382	7.735	6.261	6.330	6.775	7.147	33.541	6.211	15.633	8.614	9.222	6.881	8.239
384	7.724	6.210	6.312	6.749	7.137	33.528	6.257	15.641	8.653	9.263	6.884	8.263
386	7.747	6.224	6.332	6.768	7.158	33.264	6.230	15.551	8.604	9.231	6.842	8.226
388	7.800	6.192	6.296	6.763	7.155	33.050	6.255	15.487	8.581	9.226	6.803	8.203
390	7.689	6.192	6.309	6.730	7.117	33.384	6.230	15.577	8.571	9.204	6.832	8.202
392	7.720	6.187	6.306	6.738	7.132	33.324	6.230	15.562	8.586	9.193	6.752	8.177
394	7.712	6.204	6.293	6.736	7.129	33.468	6.203	15.600	8.500	9.207	6.740	8.149
396	7.735	6.175	6.253	6.721	7.125	33.252	6.230	15.536	8.510	9.171	6.749	8.143
398	7.696	6.146	6.268	6.703	7.119	32.918	6.228	15.422	8.491	9.155	6.729	8.125
400	7.655	6.146	6.238	6.680	7.087	32.908	6.247	15.414	8.477	9.097	6.685	8.086
402	7.708	6.165	6.250	6.708	7.112	32.908	6.223	15.414	8.459	9.169	6.691	8.106
404	7.700	6.129	6.240	6.690	7.074	32.838	6.247	15.386	8.416	9.148	6.753	8.106
406	7.636	6.150	6.282	6.689	7.068	32.768	6.245	15.360	8.398	9.065	6.709	8.057
408	7.655	6.116	6.220	6.664	7.090	32.699	6.193	15.327	8.379	9.090	6.607	8.025
410	7.670	6.138	6.215	6.674	7.058	32.898	6.242	15.399	8.390	9.085	6.640	8.038
412	7.666	6.116	6.180	6.654	7.045	33.028	6.242	15.438	8.367	9.033	6.628	8.009
414	7.632	6.104	6.168	6.635	7.067	32.629	6.242	15.313	8.353	9.053	6.585	7.997
416	7.621	6.148	6.177	6.649	7.084	32.690	6.213	15.322	8.340	9.042	6.566	7.983
418	7.583	6.121	6.221	6.642	7.057	32.759	6.262	15.359	8.355	9.037	6.580	7.991
420	7.579	6.114	6.160	6.618	7.048	32.560	6.210	15.273	8.300	8.985	6.555	7.947
422	7.606	6.109	6.153	6.623	7.013	32.492	6.210	15.238	8.332	9.011	6.493	7.945
424	7.587	6.080	6.119	6.595	6.982	32.612	6.213	15.269	8.306	8.975	6.522	7.934
426	7.557	6.095	6.155	6.602	7.007	32.423	6.235	15.222	8.255	8.990	6.480	7.908
428	7.542	6.092	6.126	6.587	6.997	32.423	6.232	15.217	8.270	8.939	6.467	7.892

Cycle	Gap Graded			Continuous Graded			Open Graded			Average		
	1	2	3	Average	1	2	3	Average	1	2	3	
430	7.565	6.061	6.116	6.581	6.991	32.220	6.232	15.148	8.220	8.928	6.448	7.885
432	7.531	6.104	6.087	6.574	6.988	32.025	6.205	15.073	8.211	8.928	6.427	7.855
434	7.546	6.073	6.145	6.588	7.003	32.476	6.230	15.236	8.162	8.913	6.439	7.838
436	7.516	6.076	6.061	6.551	7.009	32.086	6.274	15.123	8.177	8.933	6.400	7.837
438	7.505	6.040	6.083	6.543	6.941	32.280	6.203	15.141	8.127	8.917	6.409	7.818
440	7.465	6.061	6.068	6.531	7.000	32.146	6.227	15.124	8.106	8.877	6.363	7.782
442	7.494	6.064	6.056	6.538	6.963	32.019	6.227	15.070	8.097	8.942	6.353	7.797
444	7.349	6.054	6.033	6.479	6.950	31.826	6.176	14.984	8.080	8.871	6.307	7.753
446	7.038	6.052	6.109	6.400	6.919	32.146	6.200	15.088	8.068	8.846	6.316	7.743
448	6.536	6.018	6.031	6.195	6.910	32.153	6.225	15.096	8.083	8.861	6.291	7.745
450	6.474	6.018	6.000	6.164	6.928	31.820	6.247	14.998	8.070	8.831	6.283	7.728
452	7.183	6.038	6.019	6.413	6.956	31.755	6.217	14.976	8.053	8.916	6.275	7.748
454	7.350	6.031	6.074	6.485	7.004	31.947	6.171	15.041	8.041	8.836	6.225	7.701
456	7.403	6.007	5.986	6.465	6.891	31.881	6.195	14.989	8.024	8.820	6.235	7.693
458	7.447	6.026	5.982	6.485	6.940	31.820	6.217	14.992	8.043	8.870	6.217	7.710
460	7.421	6.019	5.953	6.464	6.906	31.750	6.195	14.950	7.999	8.795	6.200	7.665
462	7.444	5.993	5.946	6.461	6.897	31.560	6.190	14.882	7.978	8.785	6.182	7.648
464	7.437	5.986	5.977	6.467	6.867	31.875	6.166	14.969	7.974	8.706	6.170	7.617
466	7.459	6.005	5.973	6.479	6.912	31.690	6.217	14.940	7.958	8.765	6.126	7.616
468	7.422	6.026	5.964	6.471	6.912	31.745	6.215	14.957	7.942	8.716	6.165	7.608
470	7.452	6.000	5.957	6.470	6.851	31.431	6.188	14.823	7.921	8.721	6.119	7.587
472	7.474	5.946	5.922	6.447	6.930	31.560	6.185	14.892	7.917	8.741	6.104	7.587
474	7.493	5.991	5.943	6.476	6.894	31.556	6.215	14.888	7.932	8.731	6.068	7.577
476	7.419	6.003	5.911	6.444	6.884	31.300	6.212	14.799	7.846	8.716	6.095	7.552
478	7.423	5.970	5.895	6.429	6.854	31.240	6.158	14.751	7.869	8.692	6.054	7.538
480	7.412	5.953	5.888	6.418	6.821	31.240	6.185	14.749	7.830	8.682	6.014	7.509
482	7.464	5.970	5.879	6.438	6.869	31.300	6.210	14.793	7.833	8.633	6.000	7.489
484	7.424	5.965	5.870	6.420	6.866	31.364	6.232	14.821	7.856	8.657	6.031	7.515
486	7.398	5.984	5.889	6.424	6.860	31.113	6.183	14.719	7.786	8.726	6.010	7.507
488	7.362	5.942	5.878	6.394	6.857	31.364	6.183	14.801	7.797	8.638	5.998	7.478
490	7.384	5.930	5.890	6.401	6.821	31.174	6.183	14.726	7.812	8.619	5.952	7.461
492	7.341	5.949	5.839	6.376	6.848	30.926	6.180	14.651	7.705	8.585	5.940	7.410
494	7.337	5.952	5.824	6.371	6.845	30.988	6.205	14.679	7.735	8.624	5.915	7.425
496	7.370	5.961	5.824	6.385	6.782	30.864	6.202	14.616	7.750	8.595	5.925	7.423
498	7.356	5.938	5.792	6.362	6.797	30.986	6.254	14.679	7.731	8.610	5.863	7.401
500	7.404	5.938	5.778	6.331	6.800	31.049	6.173	14.692	7.727	8.576	5.804	7.323
502	7.339	5.903	5.821	6.354	6.794	30.864	6.197	14.618	7.704	8.562	5.855	7.374
504	7.339	5.905	5.766	6.337	6.758	30.863	6.178	14.600	7.658	8.620	5.814	7.364
506	7.335	5.901	5.803	6.346	6.785	30.924	6.175	14.628	7.685	8.500	5.865	7.350
508	7.303	5.913	5.778	6.331	6.800	30.679	6.173	14.551	7.636	8.529	5.804	7.323
510	7.325	5.913	5.788	6.342	6.764	30.924	6.197	14.628	7.681	8.490	5.810	7.327
512	7.343	5.936	5.782	6.354	6.785	30.986	6.197	14.666	7.639	8.471	5.794	7.301
514	7.314	5.881	5.750	6.315	6.758	30.497	6.170	14.475	7.624	8.510	5.752	7.295
516	7.307	5.902	5.762	6.324	6.779	30.619	6.195	14.531	7.639	8.486	5.758	7.294

Cycle	Gap Graded			Continuous Graded			Open Graded			Average		
	1	2	3	Average	1	2	3	Average	1	2	3	Average
518	7.300	5.876	5.778	6.318	6.743	30.619	6.170	14.511	7.598	8.453	5.718	7.256
520	7.290	5.890	5.727	6.302	6.770	30.679	6.165	14.538	7.591	8.539	5.704	7.278
522	7.302	5.863	5.720	6.295	6.767	30.618	6.168	14.518	7.576	8.497	5.693	7.255
524	7.251	5.881	5.687	6.273	6.734	30.497	6.143	14.458	7.598	8.507	5.654	7.253
526	7.276	5.856	5.683	6.272	6.722	30.739	6.190	14.550	7.520	8.411	5.656	7.196
528	7.244	5.875	5.740	6.286	6.717	30.618	6.190	14.508	7.572	8.431	5.643	7.215
530	7.259	5.891	5.687	6.279	6.717	30.139	6.165	14.340	7.502	8.388	5.631	7.174
532	7.263	5.868	5.656	6.262	6.743	30.317	6.187	14.416	7.510	8.413	5.608	7.177
534	7.246	5.840	5.674	6.253	6.699	30.198	6.165	14.354	7.528	8.399	5.574	7.167
536	7.220	5.864	5.668	6.251	6.726	30.558	6.160	14.481	7.455	8.394	5.580	7.143
538	7.217	5.859	5.633	6.236	6.720	30.198	6.185	14.368	7.503	8.381	5.536	7.140
540	7.207	5.878	5.651	6.245	6.687	30.438	6.160	14.428	7.466	8.385	5.552	7.134
542	7.235	5.827	5.641	6.234	6.708	30.198	6.136	14.347	7.448	8.396	5.554	7.133
544	7.197	5.867	5.661	6.242	6.708	30.318	6.212	14.413	7.430	8.320	5.496	7.082
546	7.222	5.869	5.647	6.246	6.699	30.319	6.158	14.392	7.390	8.340	5.501	7.077
548	7.212	5.835	5.663	6.237	6.699	29.963	6.158	14.273	7.409	8.359	5.485	7.084
550	7.240	5.853	5.616	6.236	6.638	29.961	6.180	14.260	7.362	8.392	5.460	7.068
552	7.202	5.833	5.612	6.216	6.685	30.141	6.134	14.320	7.403	8.317	5.460	7.060
554	7.163	5.849	5.582	6.198	6.644	30.201	6.182	14.342	7.337	8.313	5.458	7.036
556	7.224	5.824	5.616	6.221	6.676	29.847	6.156	14.226	7.316	8.300	5.427	7.014
558	7.157	5.824	5.588	6.190	6.638	29.671	6.153	14.164	7.320	8.262	5.435	7.006
560	7.178	5.817	5.608	6.201	6.638	30.084	6.177	14.300	7.361	8.245	5.416	7.007
562	7.178	5.790	5.580	6.183	6.656	29.963	6.177	14.265	7.321	8.273	5.424	7.006
564	7.137	5.790	5.544	6.157	6.656	29.966	6.175	14.266	7.311	8.231	5.346	6.963
566	7.137	5.806	5.536	6.160	6.650	29.986	6.175	14.264	7.258	8.288	5.348	6.965
568	7.158	5.793	5.550	6.167	6.650	29.847	6.175	14.224	7.302	8.270	5.357	6.976
570	7.158	5.800	5.570	6.176	6.616	29.905	6.175	14.232	7.263	8.168	5.340	6.924
572	7.155	5.795	5.562	6.171	6.636	29.968	6.148	14.251	7.281	8.229	5.325	6.945
574	7.089	5.770	5.560	6.140	6.607	29.621	6.148	14.125	7.246	8.225	5.295	6.922
576	7.167	5.787	5.526	6.160	6.622	29.735	6.146	14.168	7.235	8.179	5.284	6.899
578	7.173	5.782	5.496	6.150	6.619	29.561	6.173	14.118	7.244	8.194	5.244	6.894
580	7.125	5.799	5.512	6.145	6.613	29.443	6.151	14.069	7.170	8.190	5.268	6.876
582	7.094	5.753	5.528	6.125	6.613	29.504	6.173	14.097	7.163	8.144	5.233	6.847
584	7.097	5.753	5.526	6.101	6.533	29.391	6.141	14.083	7.168	8.144	5.196	6.836
586	7.087	5.790	5.515	6.131	6.634	29.621	6.192	14.149	7.120	8.132	5.203	6.842
588	7.109	5.745	5.481	6.112	6.591	29.561	6.185	14.112	7.152	8.140	5.189	6.827
590	7.078	5.784	5.454	6.105	6.568	29.561	6.165	14.098	7.145	8.151	5.156	6.817
592	7.071	5.736	5.497	6.101	6.533	29.500	6.146	14.022	7.132	8.138	5.159	6.810
594	7.099	5.759	5.469	6.109	6.559	29.391	6.165	14.038	7.112	8.157	5.169	6.813
596	7.099	5.752	5.458	6.103	6.580	29.396	6.163	14.046	7.134	8.089	5.105	6.776
598	7.061	5.723	5.454	6.079	6.582	29.280	6.114	13.992	7.065	8.076	5.117	6.753
600	7.127	5.746	5.472	6.116	6.545	29.396	6.143	14.028	7.049	8.132	5.101	6.761
602	7.052	5.737	5.421	6.070	6.568	29.229	6.165	13.987	7.073	8.091	5.086	6.750
604	7.080	5.735	5.437	6.084	6.537	29.340	6.136	14.004	7.026	8.091	5.078	6.732

Cycle	Gap Graded			Continuous Graded			Open Graded			Average		
	1	2	3	Average	1	2	3	Average	1	2	3	
606	7.070	5.733	5.474	6.092	6.531	29.108	6.114	13.918	7.075	8.074	5.031	6.727
608	7.045	5.756	5.445	6.082	6.580	29.285	6.141	14.002	7.007	8.034	5.023	6.688
610	7.057	5.729	5.482	6.089	6.518	29.229	6.136	13.961	6.984	8.057	5.027	6.693
612	7.057	5.706	5.389	6.051	6.518	29.059	6.160	13.912	7.012	8.086	5.030	6.703
614	7.020	5.766	5.403	6.063	6.532	29.053	6.158	13.914	7.024	8.013	4.978	6.672
616	7.048	5.698	5.395	6.047	6.535	29.119	6.109	13.921	7.015	8.036	4.985	6.679
618	7.013	5.716	5.372	6.034	6.527	29.004	6.185	13.905	6.984	8.060	4.969	6.671
620	7.038	5.737	5.426	6.067	6.498	29.114	6.133	13.915	6.928	8.052	4.951	6.644
622	7.031	5.687	5.357	6.025	6.547	28.955	6.131	13.878	6.925	8.035	4.943	6.634
624	7.028	5.699	5.355	6.027	6.542	29.004	6.158	13.901	6.937	8.031	4.926	6.631
626	7.077	5.726	5.367	6.057	6.487	29.010	6.155	13.884	6.955	7.991	4.891	6.612
628	7.019	5.675	5.340	6.011	6.482	29.064	6.129	13.892	6.915	8.014	4.902	6.610
630	7.012	5.718	5.360	6.030	6.471	28.841	6.131	13.814	6.879	7.978	4.902	6.586
632	7.009	5.711	5.352	6.024	6.468	28.841	6.129	13.813	6.927	7.943	4.888	6.586
634	6.978	5.691	5.380	6.016	6.463	28.787	6.153	13.801	6.857	7.970	4.852	6.560
636	7.055	5.689	5.358	6.034	6.460	28.847	6.129	13.812	6.863	7.918	4.878	6.553
638	6.996	5.709	5.353	6.019	6.457	28.794	6.153	13.801	6.860	7.950	4.846	6.552
640	6.990	5.656	5.328	5.991	6.478	28.787	6.129	13.798	6.848	7.910	4.809	6.522
642	6.987	5.699	5.321	6.002	6.470	28.901	6.102	13.824	6.869	7.894	4.813	6.525
644	6.953	5.650	5.291	5.985	6.498	28.733	6.102	13.778	6.800	7.917	4.759	6.492
646	6.953	5.670	5.331	5.985	6.467	28.794	6.153	13.801	6.860	7.950	4.846	6.499
648	6.978	5.690	5.304	5.991	6.402	28.687	6.119	13.736	6.803	7.897	4.769	6.490
650	6.968	5.646	5.318	5.977	6.513	28.513	6.146	13.724	6.764	7.866	4.760	6.463
652	6.962	5.646	5.311	5.973	6.420	28.619	6.097	13.712	6.779	7.858	4.737	6.458
654	6.989	5.684	5.269	5.985	6.448	28.407	6.121	13.669	6.800	7.877	4.752	6.476
656	6.931	5.660	5.265	5.949	6.471	28.521	6.165	13.719	6.725	7.936	4.782	6.499
658	6.922	5.651	5.277	5.950	6.420	28.407	6.170	13.666	6.743	7.865	4.727	6.445
660	6.922	5.676	5.282	5.980	6.409	28.581	6.167	13.719	6.723	7.849	4.741	6.438
662	6.940	5.645	5.278	5.984	6.406	28.528	6.119	13.684	6.726	7.853	4.703	6.427
664	6.971	5.643	5.257	5.957	6.398	28.634	6.146	13.726	6.685	7.837	4.701	6.408
666	6.909	5.643	5.252	5.935	6.424	28.641	6.167	13.744	6.697	7.821	4.716	6.411
668	6.903	5.619	5.273	5.932	6.416	28.311	6.119	13.615	6.670	7.813	4.711	6.398
670	6.928	5.651	5.260	5.946	6.408	28.588	6.141	13.712	6.703	7.852	4.708	6.421
672	6.924	5.653	5.233	5.937	6.385	28.147	6.116	13.549	6.662	7.801	4.705	6.389
674	6.918	5.607	5.251	5.925	6.411	28.311	6.116	13.613	6.627	7.797	4.721	6.382
676	6.921	5.625	5.225	5.932	6.403	28.371	6.090	13.621	6.642	7.812	4.678	6.377
678	6.882	5.625	5.241	5.916	6.369	28.267	6.138	13.591	6.630	7.770	4.697	6.366
680	6.885	5.641	5.234	5.920	6.344	28.595	6.138	13.692	6.651	7.774	4.691	6.372
682	6.872	5.616	5.210	5.899	6.361	28.156	6.114	13.544	6.582	7.762	4.687	6.344
684	6.866	5.614	5.203	5.894	6.381	28.267	6.136	13.595	6.599	7.754	4.683	6.345
686	6.897	5.590	5.199	5.895	6.374	28.267	6.112	13.584	6.559	7.773	4.661	6.331
688	6.915	5.614	5.170	5.900	6.376	28.371	6.112	13.620	6.568	7.693	4.714	6.325
690	6.887	5.606	5.185	5.893	6.371	28.216	6.112	13.566	6.560	7.720	4.691	6.324
692	6.878	5.602	5.182	5.887	6.346	28.104	6.133	13.528	6.497	7.746	4.669	6.304

Cycle	Gap Graded			Continuous Graded			Open Graded			Average
	1		2	1		2	1		2	
	Average	1	2	Average	1	2	3	Average	1	2
694	6.899	5.600	5.197	5.899	6.313	28.224	6.109	13.549	6.538	6.298
695	6.872	5.572	5.171	5.872	6.360	28.216	6.131	13.569	6.530	6.296
696	6.845	5.596	5.144	5.862	6.333	28.002	6.109	13.481	6.524	6.299
698	700	6.836	5.612	5.181	5.876	6.353	28.062	6.109	13.508	6.482
702	6.890	5.564	5.174	5.876	6.320	27.951	6.131	13.467	6.497	6.649
704	6.824	5.610	5.171	5.868	6.347	27.951	6.080	13.459	6.491	6.664
706	6.845	5.584	5.168	5.866	6.340	27.850	6.153	13.448	6.472	6.643
708	6.851	5.582	5.137	5.857	6.284	28.011	6.131	13.475	6.492	6.628
710	6.836	5.604	5.154	5.865	6.307	27.850	6.104	13.420	6.448	6.634
712	6.890	5.604	5.128	5.874	6.324	27.790	6.129	13.414	6.465	6.639
714	6.812	5.602	5.145	5.853	6.299	27.910	6.126	13.445	6.432	6.598
716	6.803	5.578	5.140	5.840	6.319	27.910	6.102	13.444	6.419	6.624
718	6.857	5.556	5.155	5.856	6.294	27.910	6.124	13.443	6.408	6.637
720	6.797	5.600	5.110	5.836	6.341	27.860	6.075	13.425	6.405	6.609
722	6.821	5.578	5.106	5.835	6.281	27.860	6.078	13.406	6.417	6.602
724	6.818	5.576	5.078	5.824	6.301	27.910	6.124	13.445	6.384	6.587
726	6.785	5.554	5.095	5.811	6.303	27.810	6.102	13.405	6.346	6.580
728	6.815	5.574	5.093	5.827	6.318	27.860	6.099	13.426	6.355	6.542
730	6.776	5.574	5.064	5.805	6.293	27.700	6.097	13.363	6.345	6.572
732	6.853	5.572	5.081	5.835	6.261	27.650	6.145	13.352	6.334	6.561
734	6.794	5.618	5.111	5.841	6.288	27.810	6.124	13.407	6.302	6.557
736	6.764	5.572	5.070	5.802	6.308	27.442	6.095	13.282	6.314	6.524
738	6.791	5.548	5.061	5.800	6.275	27.770	6.119	13.388	6.356	6.543
740	6.764	5.592	5.081	5.812	6.221	27.453	6.119	13.264	6.268	6.572
742	6.808	5.568	5.056	5.811	6.270	27.491	6.143	13.301	6.250	6.498
744	6.805	5.568	5.051	5.808	6.240	27.661	6.095	13.332	6.297	6.513
746	6.746	5.564	5.026	5.779	6.260	27.671	6.097	13.343	6.268	6.510
748	6.767	5.568	5.020	5.780	6.235	27.502	6.092	13.276	6.255	6.443
750	6.743	5.542	5.078	5.788	6.247	27.562	6.090	13.300	6.240	6.491
752	6.764	5.564	5.015	5.781	6.247	27.611	6.116	13.325	6.235	6.488
754	6.798	5.564	5.047	5.800	6.277	27.404	6.092	13.258	6.244	6.506
756	6.728	5.564	5.020	5.771	6.215	27.562	6.114	13.297	6.210	6.447
758	6.728	5.542	5.015	5.762	6.240	27.355	6.114	13.236	6.197	6.470
760	6.773	5.540	4.992	5.768	6.225	27.355	6.090	13.223	6.185	7.437
762	6.749	5.584	4.987	5.773	6.205	27.307	6.116	13.209	6.148	7.452
764	6.711	5.560	4.947	5.800	6.198	27.415	6.087	13.233	6.187	7.500
766	6.734	5.562	4.994	5.763	6.220	27.210	6.087	13.172	6.131	7.467
768	6.708	5.582	4.970	5.753	6.195	27.534	6.109	13.279	6.170	7.460
770	6.758	5.560	5.003	5.774	6.185	27.210	6.112	13.169	6.129	7.423
772	6.755	5.534	4.959	5.749	6.185	27.259	6.109	13.184	6.124	7.416
774	6.746	5.556	4.977	5.760	6.205	27.318	6.085	13.203	6.167	7.405
776	6.740	5.580	4.988	5.769	6.202	27.222	6.083	13.169	6.099	7.369
778	6.687	5.578	4.982	5.749	6.197	27.318	6.083	13.199	6.116	7.417
780	6.676	5.598	4.957	5.744	6.165	27.318	6.085	13.189	6.104	7.391

Appendix 4
Dynamic Creep Test results

Cycle	Gap Graded			Continuous Graded			Open graded			Average		
	1	2	3	Average	1	2	3	Average	1	2	3	
782	6.682	5.554	4.954	5.730	6.244	26.818	6.083	13.048	6.076	7.348	4.509	5.978
784	6.708	5.576	4.949	5.744	6.187	27.174	6.083	13.148	6.064	7.403	4.490	5.986
786	6.676	5.554	4.927	5.719	6.158	27.115	6.107	13.127	6.028	7.363	4.485	5.959
788	6.697	5.552	4.934	5.728	6.170	27.019	6.058	13.082	6.016	7.359	4.517	5.964
790	6.667	5.550	4.935	5.717	6.124	27.127	6.080	13.110	6.011	7.313	4.478	5.934
792	6.694	5.528	4.929	5.717	6.119	27.127	6.080	13.109	6.000	7.352	4.473	5.942
794	6.661	5.550	4.945	5.719	6.143	26.938	6.080	13.054	5.986	7.332	4.487	5.935
796	6.708	5.524	4.944	5.725	6.114	26.912	6.104	13.043	6.000	7.321	4.465	5.929
798	6.679	5.570	4.916	5.722	6.133	27.019	6.104	13.085	5.991	7.263	4.479	5.911
800	6.679	5.590	4.912	5.727	6.131	27.032	6.102	13.088	5.984	7.278	4.475	5.912
802	6.671	5.566	4.928	5.722	6.129	27.127	6.102	13.119	5.972	7.275	4.470	5.906
804	6.671	5.546	4.923	5.713	6.121	27.032	6.078	13.077	5.937	7.290	4.504	5.910
806	6.671	5.592	4.902	5.722	6.097	26.725	6.078	12.967	5.954	7.265	4.465	5.895
808	6.659	5.522	4.914	5.698	6.112	26.725	6.099	12.979	5.945	7.290	4.475	5.903
810	6.662	5.544	4.869	5.692	6.131	26.891	6.078	13.033	5.952	7.305	4.451	5.903
812	6.682	5.542	4.905	5.710	6.133	26.785	6.124	13.014	5.920	7.273	4.451	5.881
814	6.680	5.542	4.879	5.700	6.131	26.692	6.078	12.967	5.908	7.295	4.463	5.889
816	6.648	5.522	4.872	5.681	6.129	26.891	6.099	13.040	5.902	7.224	4.441	5.856
818	6.648	5.562	4.872	5.694	6.124	26.798	6.102	13.008	5.867	7.246	4.456	5.856
820	6.627	5.540	4.886	5.684	6.114	26.798	6.099	13.004	5.881	7.242	4.450	5.858
822	6.616	5.540	4.862	5.673	6.109	26.846	6.073	12.943	5.868	7.232	4.464	5.855
824	6.639	5.518	4.872	5.676	6.109	26.738	6.102	12.983	5.885	7.225	4.407	5.839
826	6.630	5.538	4.871	5.680	6.099	26.798	6.049	12.982	5.897	7.247	4.422	5.822
828	6.610	5.538	4.823	5.657	6.078	26.751	6.071	12.967	5.835	7.215	4.417	5.822
830	6.599	5.520	4.843	5.654	6.044	26.751	6.049	12.948	5.837	7.240	4.431	5.836
832	6.619	5.534	4.820	5.658	6.090	26.659	6.121	12.957	5.796	7.202	4.393	5.797
834	6.645	5.536	4.853	5.678	6.068	26.494	6.097	12.886	5.842	7.160	4.424	5.809
836	6.616	5.554	4.827	5.686	6.061	26.659	6.047	12.922	5.804	7.192	4.421	5.806
838	6.587	5.578	4.823	5.663	6.083	26.417	6.095	12.885	5.839	7.182	4.399	5.807
840	6.637	5.556	4.818	5.670	6.059	26.613	6.092	12.921	5.762	7.172	4.413	5.782
842	6.579	5.510	4.828	5.639	6.080	26.522	6.042	12.881	5.774	7.165	4.425	5.788
844	6.602	5.506	4.825	5.644	6.073	26.613	6.095	12.927	5.784	7.130	4.389	5.768
846	6.605	5.510	4.803	5.639	6.044	26.627	6.044	12.905	5.732	7.127	4.367	5.742
848	6.597	5.530	4.781	5.636	6.068	26.476	6.092	12.879	5.766	7.142	4.398	5.769
850	6.602	5.532	4.815	5.650	6.066	26.417	6.087	12.857	5.762	7.167	4.393	5.774
852	6.565	5.528	4.787	5.627	6.083	26.417	6.066	12.865	5.745	7.160	4.354	5.753
854	6.597	5.508	4.767	5.624	6.083	26.811	6.068	12.987	5.737	7.125	4.354	5.739
856	6.582	5.506	4.765	5.618	6.006	26.431	6.039	12.825	5.728	7.091	Fail	6.410
858	6.580	5.522	4.775	5.626	6.049	26.581	6.063	12.898	5.699	7.116	Fail	6.408
860	6.597	5.528	4.755	5.627	6.045	26.627	6.063	12.912	5.687	7.131	Fail	6.409
862	6.577	5.548	4.793	5.639	5.992	26.237	6.112	12.780	5.678	7.127	Fail	6.403
864	6.574	5.524	4.766	5.621	6.040	26.237	6.063	12.780	5.666	7.114	Fail	6.390
866	6.597	5.524	4.798	5.640	6.014	26.237	6.087	12.779	5.658	7.090	Fail	6.374
868	6.583	5.502	4.755	5.607	6.031	26.536	6.039	12.869	5.690	7.077	Fail	6.384

Cycle	Gap Graded			Continuous Graded			Open Graded			Average	
	1	2	3	Average	1	2	3	Average	1	2	3
870	6.568	5.526	4.752	5.615	6.028	26.522	6.035	12.862	5.659	7.064	Fail
872	6.534	5.544	4.747	5.608	5.978	26.341	6.059	12.793	5.633	7.008	Fail
874	6.555	5.520	4.744	5.606	6.019	26.192	6.059	12.757	5.602	7.060	Fail
876	6.555	5.516	4.734	5.602	6.007	26.252	6.061	12.773	5.633	7.048	Fail
878	6.578	5.520	4.731	5.610	6.016	26.400	6.087	12.834	5.624	7.016	Fail
880	6.526	5.498	4.770	5.598	6.012	26.163	6.059	12.745	5.590	7.035	Fail
882	6.569	5.495	4.726	5.597	5.983	26.296	6.083	12.787	5.580	7.028	Fail
884	6.541	5.518	4.723	5.594	5.957	26.104	6.056	12.706	5.548	7.019	Fail
886	6.564	5.516	4.701	5.594	5.976	26.104	6.059	12.713	5.558	7.016	Fail
888	6.538	5.538	4.714	5.597	5.993	26.104	6.032	12.710	5.552	7.037	Fail
890	6.535	5.538	4.730	5.601	5.969	26.163	6.080	12.737	5.514	7.000	Fail
892	6.558	5.514	4.710	5.594	5.982	26.163	6.059	12.735	5.512	6.993	Fail
894	6.532	5.514	4.684	5.577	6.010	25.956	6.056	12.674	5.548	6.990	Fail
896	6.501	5.514	4.697	5.571	5.958	26.015	6.054	12.676	5.514	7.008	Fail
898	6.527	5.491	4.674	5.564	5.970	25.972	6.032	12.658	5.526	6.981	Fail
900	6.498	5.534	4.688	5.573	5.951	26.178	6.078	12.736	5.536	6.941	Fail
902	6.493	5.534	4.667	5.565	5.927	25.928	6.078	12.644	5.465	6.928	Fail
904	6.570	5.489	4.681	5.580	5.994	26.090	6.078	12.721	5.452	6.928	Fail
906	6.485	5.530	4.693	5.569	5.961	26.075	6.054	12.697	5.440	6.949	Fail
908	6.508	5.489	4.653	5.550	5.937	26.075	6.052	12.688	5.436	6.946	Fail
910	6.505	5.508	4.666	5.560	5.937	25.781	6.078	12.599	5.450	6.937	Fail
912	6.502	5.510	4.663	5.558	5.935	25.987	6.073	12.665	5.435	6.903	Fail
914	6.497	5.508	4.660	5.555	5.926	25.900	6.052	12.626	5.406	6.924	Fail
916	6.474	5.506	4.654	5.545	5.947	25.797	6.052	12.599	5.394	6.946	Fail
918	6.514	5.485	4.650	5.560	5.940	25.943	6.049	12.644	5.385	6.921	Fail
920	6.468	5.506	4.666	5.547	5.919	26.090	6.049	12.686	5.403	6.900	Fail
922	6.463	5.506	4.660	5.543	5.938	25.900	6.076	12.638	5.368	6.900	Fail
924	6.460	5.505	4.638	5.534	5.889	25.797	6.073	12.586	5.380	6.894	Fail
926	6.455	5.505	4.634	5.531	5.901	25.813	6.068	12.594	5.349	6.887	Fail
928	6.483	5.526	4.631	5.547	5.950	25.754	6.073	12.592	5.352	6.905	Fail
930	6.478	5.505	4.624	5.536	5.929	25.813	6.025	12.589	5.352	6.878	Fail
932	6.506	5.524	4.604	5.545	5.922	25.711	6.097	12.577	5.364	6.860	Fail
934	6.472	5.501	4.616	5.530	5.871	25.857	6.044	12.591	5.311	6.860	Fail
936	6.447	5.501	4.628	5.525	5.913	25.669	6.047	12.543	5.321	6.863	Fail
938	6.441	5.522	4.627	5.530	5.934	25.711	6.044	12.563	5.287	6.877	Fail
940	6.439	5.501	4.621	5.520	5.885	25.770	6.044	12.566	5.327	6.869	Fail
942	6.444	5.479	4.584	5.502	5.927	25.915	6.044	12.629	5.297	6.836	Fail
944	6.459	5.477	4.616	5.517	5.869	25.685	6.068	12.541	5.265	6.833	Fail
946	6.459	5.518	4.594	5.524	5.897	25.600	6.116	12.538	5.277	6.791	Fail
948	6.482	5.516	4.573	5.524	5.897	25.642	6.068	12.536	5.270	6.818	Fail
950	6.464	5.518	4.567	5.516	5.865	25.727	6.042	12.546	5.257	6.815	Fail
952	6.422	5.495	4.604	5.507	5.837	25.685	6.020	12.514	5.250	6.829	Fail
954	6.443	5.495	4.577	5.505	5.858	25.701	6.042	12.534	5.234	6.823	Fail
956	6.422	5.493	4.609	5.508	5.877	25.583	6.040	12.500	5.233	6.773	Fail

Appendix 4
Dynamic Creep Test results

Cycle	Gap Graded			Continuous Graded			Open Graded			Average		
	1	2	3	Average	1	2	3	Average	1	2	3	
958	6.443	5.471	4.564	5.493	5.851	25.583	6.044	12.493	5.201	6.785	Fail	5.993
960	6.414	5.516	4.583	5.504	5.847	25.642	6.040	12.510	5.213	6.817	Fail	6.015
962	6.463	5.515	4.560	5.513	5.898	6.064	12.535	5.203	6.773	Fail	5.988	
964	6.483	5.493	4.538	5.505	5.840	25.456	6.013	12.436	5.175	6.770	Fail	5.973
966	6.412	5.518	4.552	5.494	5.815	25.473	6.064	12.451	5.189	6.767	Fail	5.978
968	6.435	5.491	4.546	5.491	5.836	25.473	6.035	12.448	5.177	6.737	Fail	5.957
970	6.457	5.469	4.564	5.497	5.832	25.372	6.037	12.414	5.170	6.755	Fail	5.963
972	6.424	5.465	4.578	5.489	5.802	25.456	6.013	12.424	5.163	6.728	Fail	5.946
974	6.421	5.489	4.542	5.484	5.823	25.473	6.013	12.436	5.151	6.740	Fail	5.946
976	6.393	5.489	4.523	5.468	5.821	25.331	6.009	12.387	5.168	6.764	Fail	5.966
978	6.436	5.465	4.538	5.480	5.823	25.473	6.059	12.462	5.135	6.731	Fail	5.933
980	6.439	5.489	4.537	5.488	5.862	25.289	6.059	12.403	5.125	6.726	Fail	5.926
982	6.388	5.465	4.518	5.467	5.833	25.431	6.035	12.433	5.142	6.743	Fail	5.943
984	6.413	5.463	4.534	5.470	5.830	25.431	6.035	12.432	5.111	6.720	Fail	5.916
986	6.431	5.487	4.551	5.490	5.807	25.431	6.059	12.432	5.100	6.758	Fail	5.929
988	6.405	5.485	4.550	5.480	5.847	25.289	6.056	12.397	5.090	6.673	Fail	5.882
990	6.405	5.483	4.547	5.478	5.796	25.532	6.033	12.454	5.085	6.670	Fail	5.878
992	6.375	5.461	4.544	5.460	5.817	25.448	6.011	12.425	5.095	6.691	Fail	5.893
994	6.400	5.463	4.509	5.457	5.813	25.390	6.009	12.404	5.068	6.708	Fail	5.888
996	6.392	5.524	4.525	5.480	5.794	25.490	6.009	12.431	5.035	6.656	Fail	5.846
998	6.415	5.479	4.522	5.472	5.783	25.224	6.056	12.354	5.028	6.697	Fail	5.863
1000	6.367	5.481	4.521	5.456	5.779	25.206	6.033	12.339	5.060	6.641	Fail	5.851
1002	6.361	5.458	4.520	5.446	5.798	25.024	6.006	12.276	5.048	6.691	Fail	5.870
1004	6.387	5.419	4.538	5.488	5.798	25.206	6.054	12.363	5.022	6.659	Fail	5.841
1006	6.412	5.461	4.518	5.464	5.752	25.165	6.033	12.317	5.027	6.680	Fail	5.854
1008	6.381	5.479	4.532	5.464	5.791	25.224	6.078	12.364	4.999	6.618	Fail	5.809
1010	6.407	5.477	4.533	5.472	5.741	24.984	6.030	12.252	4.994	6.616	Fail	5.805
1012	6.368	5.501	4.531	5.487	5.782	25.083	6.054	12.306	5.022	6.665	Fail	5.844
1014	6.343	5.499	4.494	5.445	5.776	25.124	6.028	12.309	4.957	6.628	Fail	5.793
1016	6.348	5.456	4.511	5.438	5.778	25.101	6.030	12.303	5.008	6.654	Fail	5.831
1018	6.368	5.475	4.527	5.457	5.774	25.183	6.030	12.329	4.977	6.616	Fail	5.797
1020	6.358	5.475	4.509	5.447	5.749	25.183	6.030	12.321	4.967	6.648	Fail	5.808
1022	6.366	5.452	4.509	5.442	5.765	25.124	6.025	12.305	4.942	6.605	Fail	5.774
1024	6.338	5.473	4.524	5.445	5.742	25.183	6.028	12.318	4.913	6.576	Fail	5.745
1026	6.335	5.471	4.524	5.443	5.711	25.142	6.049	12.301	4.903	6.591	Fail	5.747
1028	6.335	5.475	4.523	5.444	5.711	24.961	6.049	12.240	4.934	6.643	Fail	5.789
1030	6.333	5.452	4.486	5.424	5.752	25.101	6.025	12.293	4.906	6.582	Fail	5.744
1032	6.380	5.475	4.504	5.453	5.748	24.921	6.025	12.231	4.898	6.603	Fail	5.751
1034	6.358	5.471	4.518	5.449	5.694	24.961	6.047	12.234	4.871	6.582	Fail	5.727
1036	6.378	5.478	4.518	5.456	5.723	24.840	6.025	12.196	4.902	6.588	Fail	5.735
1038	6.353	5.491	4.500	5.448	5.694	25.020	6.025	12.246	4.870	6.534	Fail	5.702
1040	6.370	5.469	4.499	5.446	5.710	24.979	6.049	12.246	4.864	6.552	Fail	5.708
1042	6.317	5.448	4.481	5.415	5.737	24.921	6.021	12.226	4.837	6.575	Fail	5.706
1044	6.337	5.446	4.496	5.426	5.710	25.020	6.023	12.251	4.828	6.543	Fail	5.686

Appendix 4
Dynamic Creep Test results

Cycle	Gap Graded			Continuous Graded			Open Graded			Average		
	1	2	3	Average	1	2	3	Average	1	2	3	
1046	6.309	5.446	4.477	5.411	5.706	24.979	6.071	12.252	4.836	6.535	Fail	5.686
1048	6.334	5.489	4.495	5.439	5.700	25.020	6.021	12.247	4.848	6.532	Fail	5.690
1050	6.309	5.466	4.511	5.429	5.702	24.702	5.997	12.134	4.801	6.524	Fail	5.683
1052	6.329	5.444	4.492	5.422	5.673	24.702	6.021	12.132	4.811	6.544	Fail	5.678
1054	6.304	5.468	4.507	5.426	5.725	24.899	6.021	12.216	4.821	6.539	Fail	5.680
1056	6.327	5.466	4.491	5.428	5.710	24.899	5.997	12.202	4.813	6.519	Fail	5.666
1058	6.319	5.487	4.490	5.432	5.685	25.060	6.045	12.263	4.804	6.505	Fail	5.655
1060	6.296	5.442	4.487	5.408	5.708	24.859	6.045	12.204	4.774	6.497	Fail	5.636
1062	6.319	5.468	4.488	5.425	5.681	24.721	5.995	12.132	4.765	6.520	Fail	5.643
1064	6.291	5.444	4.486	5.407	5.679	24.899	6.016	12.198	4.758	6.480	Fail	5.619
1066	6.291	5.440	4.487	5.406	5.699	24.761	6.016	12.159	4.768	6.478	Fail	5.623
1068	6.311	5.462	4.486	5.420	5.675	24.899	6.018	12.197	4.723	6.498	Fail	5.611
1070	6.346	5.440	4.484	5.423	5.675	24.642	6.018	12.112	4.733	6.495	Fail	5.614
1072	6.314	5.440	4.483	5.412	5.668	24.544	6.018	12.077	4.707	6.492	Fail	5.600
1074	6.311	5.483	4.482	5.425	5.664	24.642	5.992	12.099	4.734	6.456	Fail	5.595
1076	6.281	5.438	4.464	5.394	5.683	24.837	6.016	12.179	4.727	6.405	Fail	5.566
1078	6.278	5.458	4.481	5.406	5.680	24.740	6.016	12.145	4.698	6.473	Fail	5.586
1080	6.278	5.458	4.479	5.405	5.656	24.740	6.038	12.145	4.708	6.443	Fail	5.576
1082	6.273	5.438	4.479	5.397	5.650	24.700	6.014	12.121	4.697	6.465	Fail	5.581
1084	6.323	5.479	4.496	5.433	5.670	24.642	5.992	12.101	4.674	6.435	Fail	5.555
1086	6.268	5.456	4.496	5.407	5.668	24.740	6.040	12.149	4.684	6.432	Fail	5.558
1088	6.268	5.434	4.477	5.393	5.668	24.602	5.990	12.087	4.639	6.419	Fail	5.529
1090	6.288	5.458	4.459	5.402	5.664	24.602	6.016	12.094	4.631	6.455	Fail	5.543
1092	6.263	5.456	4.477	5.399	5.637	24.602	6.038	12.092	4.624	6.411	Fail	5.518
1094	6.261	5.434	4.475	5.390	5.635	24.563	6.040	12.079	4.621	6.405	Fail	5.513
1096	6.283	5.454	4.474	5.404	5.653	24.524	6.038	12.072	4.653	6.400	Fail	5.527
1098	6.305	5.432	4.474	5.404	5.633	24.349	6.014	11.999	4.615	6.397	Fail	5.508
1100	6.305	5.476	4.473	5.418	5.656	24.621	6.033	12.103	4.635	6.415	Fail	5.525
1102	6.273	5.454	4.490	5.406	5.629	24.758	6.035	12.141	4.612	6.407	Fail	5.510
1104	6.250	5.432	4.470	5.384	5.643	24.543	6.014	12.067	4.628	6.376	Fail	5.602
1106	6.248	5.452	4.452	5.384	5.641	24.543	6.011	12.065	4.625	6.376	Fail	5.501
1108	6.278	5.452	4.470	5.400	5.641	24.582	6.011	12.078	4.621	6.374	Fail	5.498
1110	6.268	5.452	4.452	5.391	5.592	24.543	6.035	12.057	4.639	6.399	Fail	5.519
1112	6.275	5.448	4.468	5.396	5.627	24.369	6.031	12.009	4.607	6.312	Fail	5.489
1114	6.235	5.429	4.468	5.392	5.598	24.523	6.033	12.051	4.695	6.332	Fail	5.469
1116	6.260	5.448	4.465	5.404	5.596	24.292	6.007	11.965	4.605	6.324	Fail	5.485
1118	6.260	5.429	4.484	5.391	5.600	24.292	6.033	11.975	4.628	6.342	Fail	5.468
1120	6.270	5.450	4.468	5.466	5.396	24.466	6.035	12.051	4.617	6.360	Fail	5.479
1122	6.257	5.470	4.449	5.392	5.598	24.523	6.033	12.051	4.597	6.360	Fail	5.485
1124	6.280	5.468	4.465	5.404	5.596	24.427	6.033	12.029	4.613	6.350	Fail	5.482
1126	6.299	5.425	4.481	5.402	5.616	24.330	5.988	11.978	4.599	6.294	Fail	5.485
1128	6.272	5.427	4.462	5.387	5.604	24.388	6.035	12.009	4.596	6.339	Fail	5.468
1130	6.242	5.444	4.461	5.382	5.592	24.446	6.035	12.024	4.592	6.311	Fail	5.452
1132	6.245	5.446	4.460	5.384	5.608	24.157	6.028	11.931	4.588	6.303	Fail	5.446

Cycle	Gap Graded			Continuous Graded			Open Graded			General Summary		
	1	2	3	Average	1	2	3	Average	1	2	3	Average
1134	6.237	5.423	4.459	5.373	5.604	24.446	6.007	12.019	4.587	6.298	Fail	5.443
1136	6.213	5.419	4.460	5.364	5.606	24.485	6.028	12.040	4.584	6.318	Fail	5.451
1138	6.215	5.423	4.459	5.366	5.654	24.254	6.033	11.947	4.598	6.263	Fail	5.431
1140	6.237	5.444	4.459	5.380	5.598	24.254	6.007	11.953	4.579	6.253	Fail	5.416
1142	6.232	5.419	4.459	5.370	5.568	24.274	6.028	11.957	4.593	6.245	Fail	5.419
1144	6.257	5.443	4.475	5.392	5.594	24.157	6.004	11.918	4.590	6.270	Fail	5.430
1146	6.232	5.441	4.437	5.370	5.590	24.216	6.052	11.953	4.586	6.312	Fail	5.449
1148	6.249	5.419	4.436	5.368	5.586	24.178	6.028	11.931	4.565	6.260	Fail	5.413
1150	6.200	5.443	4.436	5.360	5.588	24.140	6.004	11.911	4.562	6.250	Fail	5.406
1152	6.205	5.419	4.472	5.365	5.540	24.198	6.028	11.922	4.576	6.223	Fail	5.400
1154	6.249	5.441	4.453	5.381	5.586	24.485	6.026	12.032	4.556	6.250	Fail	5.403
1156	6.190	5.439	4.471	5.367	5.584	24.235	5.983	11.934	4.557	6.264	Fail	5.411
1158	6.188	5.462	4.451	5.367	5.598	24.312	6.026	11.979	4.532	6.257	Fail	5.395
1160	6.212	5.437	4.485	5.378	5.532	24.160	6.023	11.905	4.548	6.227	Fail	5.388
1162	6.210	5.437	4.448	5.365	5.570	24.102	6.028	11.900	4.562	6.195	Fail	5.379
1164	6.180	5.437	4.448	5.355	5.570	24.122	6.023	11.905	4.568	6.222	Fail	5.390
1166	6.188	5.458	4.431	5.359	5.594	24.160	5.976	11.910	4.556	6.259	Fail	5.408
1168	6.207	5.439	4.447	5.364	5.544	24.064	6.004	11.871	4.532	6.185	Fail	5.369
1170	6.207	5.476	4.429	5.371	5.538	24.160	5.997	11.898	4.530	6.175	Fail	5.353
1172	6.205	5.415	4.462	5.361	5.560	23.989	5.976	11.842	4.562	6.170	Fail	5.366
1174	6.200	5.456	4.426	5.361	5.556	24.027	6.023	11.859	4.506	6.165	Fail	5.336
1176	6.246	5.456	4.426	5.376	5.516	24.102	6.021	11.880	4.503	6.185	Fail	5.344
1178	6.202	5.433	4.442	5.359	5.554	24.122	5.976	11.884	4.517	6.153	Fail	5.336
1180	6.170	5.456	4.441	5.396	5.570	24.122	5.997	11.896	4.498	6.221	Fail	5.360
1182	6.192	5.454	4.426	5.361	5.526	24.160	5.997	11.894	4.511	6.170	Fail	5.341
1184	6.214	5.435	4.439	5.363	5.548	24.084	6.021	11.884	4.511	6.168	Fail	5.340
1186	6.185	5.454	4.421	5.363	5.522	24.027	6.002	11.850	4.524	6.180	Fail	5.362
1188	6.217	5.431	4.421	5.356	5.518	24.047	6.021	11.862	4.503	6.153	Fail	5.328
1190	6.212	5.454	4.442	5.363	5.546	24.047	6.023	11.865	4.499	6.167	Fail	5.334
1192	6.153	5.431	4.453	5.346	5.494	24.047	6.023	11.855	4.499	6.167	Fail	5.333
1194	6.156	5.453	4.437	5.349	5.536	23.783	5.995	11.771	4.477	6.133	Fail	5.305
1196	6.204	5.431	4.452	5.362	5.512	23.952	5.995	11.820	4.492	6.126	Fail	5.309
1198	6.175	5.429	4.434	5.346	5.508	24.047	6.021	11.859	4.471	6.148	Fail	5.310
1200	6.170	5.429	4.469	5.366	5.523	23.972	5.985	11.831	4.486	6.167	Fail	5.327
1202	6.177	5.429	4.433	5.346	5.502	24.237	5.995	11.911	4.501	6.119	Fail	5.310
1204	6.148	5.472	4.432	5.351	5.522	23.935	6.043	11.833	4.479	6.107	Fail	5.293
1206	6.148	5.406	4.465	5.340	5.518	23.783	6.019	11.773	4.475	6.155	Fail	5.315
1208	6.168	5.425	4.447	5.347	5.469	23.841	5.995	11.768	4.473	6.095	Fail	5.284
1210	6.187	5.425	4.430	5.347	5.516	23.935	6.019	11.823	4.454	6.092	Fail	5.273
1212	6.163	5.470	4.410	5.348	5.491	23.708	6.019	11.740	4.449	6.059	Fail	5.264
1214	6.158	5.427	4.427	5.337	5.510	23.804	5.993	11.769	4.465	6.080	Fail	5.273
1216	6.141	5.427	4.443	5.337	5.505	23.804	6.014	11.774	4.443	6.128	Fail	5.286
1218	6.158	5.425	4.428	5.337	5.465	23.861	5.990	11.772	4.440	6.044	Fail	5.242
1220	6.160	5.425	4.408	5.331	5.501	23.993	5.967	11.820	4.436	6.071	Fail	5.254

Appendix 4
Dynamic Creep Test results

Cycle	Gap Graded			Continuous Graded			Open Graded			Average		
	1	2	3	Average	1	2	3	Average	1	2	3	Average
1222	6.170	5.423	4.406	5.333	5.522	23.861	6.019	4.452	6.059	Fail	5.256	38.094
1224	6.177	5.423	4.422	5.341	5.455	23.731	5.993	11.726	4.450	6.052	Fail	5.251
1226	6.150	5.445	4.438	5.344	5.477	23.804	5.988	11.756	4.446	6.049	Fail	5.248
1228	6.126	5.400	4.420	5.315	5.495	23.767	5.993	11.752	4.458	6.066	Fail	5.262
1230	6.143	5.443	4.419	5.335	5.515	23.679	5.990	11.728	4.459	6.014	Fail	5.237
1232	6.119	5.398	4.417	5.311	5.465	23.731	5.990	11.729	4.418	6.028	Fail	5.223
1234	6.146	5.422	4.434	5.334	5.444	23.825	5.993	11.754	4.415	6.026	Fail	5.221
1236	6.116	5.443	4.417	5.325	5.505	23.825	5.990	11.773	4.447	6.000	Fail	5.224
1238	6.114	5.422	4.415	5.317	5.438	23.673	5.964	11.692	4.427	6.014	Fail	5.221
1240	6.133	5.420	4.415	5.323	5.481	23.788	6.014	11.761	4.405	6.009	Fail	5.207
1242	6.109	5.422	4.413	5.315	5.436	23.751	5.990	11.726	4.402	6.026	Fail	5.214
1244	6.133	5.441	4.430	5.335	5.452	23.788	5.986	11.742	4.399	5.969	Fail	5.184
1246	6.131	5.418	4.413	5.321	5.466	23.643	5.988	11.699	4.397	5.967	Fail	5.182
1248	6.099	5.441	4.409	5.316	5.469	23.600	5.990	11.686	4.393	5.986	Fail	5.190
1250	6.126	5.418	4.411	5.318	5.446	23.549	5.988	11.661	4.425	5.979	Fail	5.202
1252	6.104	5.416	4.428	5.316	5.440	23.679	6.009	11.709	4.403	5.951	Fail	5.177
1254	6.177	5.394	4.409	5.327	5.485	23.751	5.986	11.741	4.399	5.946	Fail	5.173
1256	6.121	5.416	4.424	5.320	5.460	23.658	5.986	11.701	4.395	5.961	Fail	5.178
1258	6.095	5.418	4.443	5.319	5.415	23.528	5.988	11.644	4.410	5.937	Fail	5.174
1260	6.095	5.396	4.423	5.305	5.432	23.643	5.960	11.678	4.372	5.924	Fail	5.148
1262	6.090	5.393	4.405	5.295	5.448	23.606	5.962	11.672	4.404	5.968	Fail	5.186
1264	6.138	5.414	4.387	5.313	5.427	23.679	6.033	11.713	4.381	5.940	Fail	5.161
1266	6.170	5.416	4.404	5.330	5.415	23.420	6.007	11.614	4.378	5.912	Fail	5.145
1268	6.087	5.435	4.403	5.308	5.421	23.585	6.009	11.672	4.357	5.936	Fail	5.147
1270	6.107	5.414	4.400	5.307	5.443	23.606	5.960	11.670	4.354	5.899	Fail	5.127
1272	6.131	5.435	4.382	5.316	5.437	23.477	6.036	11.660	4.368	5.887	Fail	5.128
1274	6.083	5.412	4.399	5.298	5.419	23.679	5.983	11.694	4.364	5.883	Fail	5.124
1276	6.083	5.433	4.415	5.310	5.435	23.442	6.036	11.638	4.360	5.904	Fail	5.132
1278	6.126	5.410	4.415	5.317	5.429	23.349	5.983	11.587	4.340	5.897	Fail	5.119
1280	6.102	5.391	4.414	5.302	5.429	23.513	5.960	11.634	4.353	5.893	Fail	5.123
1282	6.087	5.432	4.395	5.305	5.425	23.499	5.986	11.637	4.349	5.886	Fail	5.118
1284	6.073	5.412	4.395	5.293	5.396	23.442	6.007	11.615	4.347	5.875	Fail	5.111
1286	6.073	5.389	4.395	5.286	5.377	23.442	5.955	11.591	4.376	5.854	Fail	5.115
1288	6.116	5.412	4.411	5.313	5.416	23.535	5.979	11.643	4.320	5.868	Fail	5.094
1290	6.114	5.432	4.427	5.324	5.414	23.406	5.981	11.600	4.316	5.838	Fail	5.077
1292	6.071	5.385	4.373	5.276	5.410	23.592	5.981	11.661	4.330	5.857	Fail	5.094
1294	6.087	5.408	4.409	5.301	5.408	23.520	6.009	11.646	4.326	5.827	Fail	5.077
1296	6.066	5.408	4.410	5.295	5.385	23.499	6.005	11.630	4.306	5.848	Fail	5.077
1298	6.085	5.430	4.422	5.312	5.406	23.428	5.955	11.596	4.320	5.841	Fail	5.081
1300	6.056	5.406	4.390	5.284	5.383	23.428	5.955	11.589	4.314	5.860	Fail	5.087
1302	6.102	5.404	4.405	5.304	5.304	23.399	5.981	11.591	4.312	5.807	Fail	5.060
1304	6.080	5.426	4.389	5.298	5.376	23.428	5.955	11.586	4.308	5.778	Fail	5.043
1306	6.080	5.385	4.385	5.283	5.393	23.463	6.002	11.619	4.323	5.865	Fail	5.094
1308	6.078	5.385	4.384	5.282	5.393	23.300	5.979	11.557	4.283	5.806	Fail	5.045

Cycle	Gap Graded			Continuous Graded			Open Graded			Average		
	1	2	3	Average	1	2	3	Average	1	2	3	Average
1310	6.073	5.404	4.385	5.287	5.368	23.335	6.002	11.568	4.312	5.827	Fail	5.070
1312	6.100	5.403	4.401	5.301	5.388	23.392	6.005	11.595	4.276	5.804	Fail	5.040
1314	6.121	5.403	4.383	5.302	5.382	23.335	5.974	11.564	4.272	5.768	Fail	5.020
1316	6.064	5.403	4.381	5.283	5.382	23.182	6.000	11.521	4.300	5.810	Fail	5.055
1318	6.044	5.403	4.398	5.282	5.378	23.322	5.976	11.569	4.281	5.782	Fail	5.032
1320	6.073	5.381	4.380	5.278	5.357	23.392	6.000	11.583	4.260	5.732	Fail	4.996
1322	6.088	5.422	4.379	5.296	5.376	23.265	5.976	11.539	4.270	5.742	Fail	5.006
1324	6.088	5.401	4.378	5.289	5.349	23.102	5.974	11.475	4.229	5.738	Fail	4.984
1326	6.085	5.399	4.396	5.293	5.349	23.286	5.998	11.550	4.260	5.759	Fail	5.010
1328	6.080	5.401	4.377	5.286	5.367	23.286	5.998	11.543	4.255	5.730	Fail	4.993
1330	6.078	5.399	4.377	5.285	5.346	23.286	5.998	11.539	4.250	5.748	Fail	4.999
1332	6.054	5.399	4.394	5.282	5.383	23.090	6.026	11.500	4.250	5.746	Fail	4.989
1334	6.078	5.420	4.391	5.296	5.338	23.194	6.024	11.519	4.231	5.733	Fail	4.979
1336	6.056	5.401	4.373	5.277	5.356	23.286	5.998	11.547	4.224	5.727	Fail	4.983
1338	6.052	5.374	4.389	6.272	5.375	23.056	5.974	11.468	4.239	5.747	Fail	5.000
1340	6.025	5.376	4.354	5.252	5.331	23.068	6.000	11.466	4.252	5.716	Fail	4.973
1342	6.049	5.418	4.370	5.219	5.347	23.068	6.024	11.480	4.230	5.689	Fail	4.950
1344	6.069	5.397	4.388	5.285	5.345	23.251	5.972	11.523	4.210	5.685	Fail	4.954
1346	6.016	5.397	4.386	5.266	5.343	23.343	5.974	11.653	4.222	5.699	Fail	4.951
1348	6.018	5.372	4.365	5.262	5.320	23.182	5.972	11.491	4.202	5.695	Fail	4.955
1350	6.045	5.395	4.385	5.275	5.316	23.147	5.972	11.478	4.214	5.695	Fail	4.963
1352	6.016	5.397	4.384	5.266	5.313	22.998	5.972	11.428	4.213	5.713	Fail	4.947
1354	6.040	5.395	4.401	5.219	5.293	23.090	5.972	11.452	4.210	5.683	Fail	4.943
1356	6.016	5.372	4.383	5.257	5.330	22.998	5.969	11.432	4.206	5.680	Fail	4.937
1358	6.011	5.414	4.363	5.263	5.284	23.125	5.995	11.468	4.202	5.672	Fail	4.937
1360	6.038	5.393	4.363	5.265	5.305	23.182	5.998	11.495	4.215	5.666	Fail	4.941
1362	6.035	5.374	4.362	5.257	5.321	23.147	5.969	11.479	4.196	5.660	Fail	4.928
1364	6.009	5.374	4.346	5.243	5.321	23.135	5.969	11.476	4.193	5.678	Fail	4.936
1366	6.031	5.434	4.360	5.275	5.319	23.078	5.991	11.463	4.188	5.649	Fail	4.919
1368	6.028	5.372	4.376	5.259	5.338	22.930	5.995	11.421	4.202	5.647	Fail	4.925
1370	6.023	5.391	4.378	5.264	5.291	22.987	5.969	11.416	4.166	5.665	Fail	4.916
1372	6.026	5.413	4.358	5.266	5.291	22.952	5.993	11.412	4.197	5.637	Fail	4.917
1374	6.019	5.370	4.374	5.254	5.310	22.952	5.967	11.410	4.193	5.631	Fail	4.912
1376	6.000	5.368	4.356	5.241	5.283	22.896	6.014	11.398	4.173	5.625	Fail	4.899
1378	6.043	5.413	4.356	5.271	5.280	23.100	5.993	11.458	4.170	5.621	Fail	4.896
1380	5.997	5.388	4.373	5.253	5.280	23.044	5.993	11.439	4.167	5.633	Fail	4.900
1390	6.016	5.366	4.370	5.254	5.310	23.044	5.944	11.414	4.163	5.614	Fail	4.889
1392	6.012	5.430	4.348	5.263	5.246	23.032	5.941	11.406	4.152	5.582	Fail	4.867
1394	6.007	5.409	4.347	5.254	5.262	23.044	5.988	11.431	4.166	5.606	Fail	4.886
1396	5.986	5.409	4.348	5.248	5.284	22.760	5.988	11.344	4.163	5.558	Fail	4.881

Appendix 4
Dynamic Creep Test results

Cycle	Gap Graded			Continuous Graded			Open Graded			Average		
	1	2	3	Average	1	2	3	Average	1	2	3	
1398	6.005	5.407	4.347	5.253	5.260	22.880	5.988	11.386	4.127	5.572	Fail	4.850
1400	5.981	5.407	4.363	5.250	5.256	22.952	5.988	11.399	4.141	5.566	Fail	4.854
1402	5.998	5.363	4.346	5.236	5.275	22.880	5.985	11.363	4.136	5.586	Fail	4.861
1404	5.998	5.386	4.360	5.248	5.251	22.884	5.988	11.374	4.118	5.552	Fail	4.835
1406	5.995	5.380	4.342	5.239	5.230	22.884	5.962	11.369	4.130	5.524	Fail	4.827
1408	5.976	5.386	4.359	5.240	5.247	22.884	5.965	11.365	4.145	5.582	Fail	4.864
1410	6.002	5.384	4.340	5.242	5.238	22.749	5.960	11.316	4.156	5.538	Fail	4.847
1412	6.021	5.382	4.323	5.242	5.265	22.783	5.988	11.345	4.121	5.532	Fail	4.827
1414	5.995	5.386	4.321	5.234	5.240	22.806	5.937	11.328	4.132	5.534	Fail	4.833
1416	5.993	5.403	4.338	5.245	5.235	22.806	5.965	11.335	4.112	5.522	Fail	4.817
1418	5.998	5.355	4.320	5.224	5.233	22.659	5.988	11.293	4.108	5.496	Fail	4.802
1420	5.967	5.359	4.336	5.221	5.254	22.749	5.939	11.314	4.088	5.538	Fail	4.813
1422	5.962	5.401	4.336	5.233	5.247	22.772	6.007	11.342	4.099	5.532	Fail	4.816
1424	6.033	5.359	4.351	5.248	5.231	22.749	5.962	11.314	4.095	5.524	Fail	4.810
1426	5.984	5.401	4.351	5.245	5.205	22.839	5.958	11.334	4.073	5.495	Fail	4.784
1428	6.003	5.380	4.334	5.239	5.222	22.749	5.986	11.319	4.071	5.483	Fail	4.777
1430	5.958	5.376	4.331	5.222	5.264	22.682	5.984	11.310	4.081	5.483	Fail	4.782
1432	5.993	5.398	4.351	5.247	5.217	22.649	5.986	11.284	4.061	5.503	Fail	4.782
1434	5.956	5.399	4.331	5.229	5.236	22.559	5.960	11.252	4.072	5.471	Fail	4.772
1436	6.001	5.357	4.347	5.236	5.238	22.772	5.986	11.332	4.069	5.468	Fail	4.769
1438	6.003	5.399	4.328	5.243	5.234	22.749	5.984	11.322	4.068	5.444	Fail	4.756
1440	5.956	5.355	4.361	5.224	5.208	22.705	5.982	11.298	Fail	5.464	Fail	4.744
1442	5.972	5.376	4.326	5.225	5.210	22.739	5.958	11.302	Fail	5.456	Fail	4.744
1444	5.998	5.396	4.325	5.240	5.181	22.649	5.960	11.263	Fail	5.491	Fail	4.741
1446	5.994	5.374	4.325	5.231	5.224	22.649	5.934	11.269	Fail	5.446	Fail	4.736
1448	5.949	5.353	4.323	5.208	5.243	22.582	5.979	11.268	Fail	5.415	Fail	4.731
1450	5.963	5.399	4.323	5.228	5.220	22.672	5.979	11.290	Fail	5.435	Fail	4.731
1452	5.944	5.374	4.321	5.213	5.217	22.615	5.982	11.271	Fail	5.474	Fail	4.729
1454	5.970	5.392	4.339	5.234	5.215	22.672	5.937	11.275	Fail	5.398	Fail	4.724
1456	5.935	5.396	4.319	5.217	5.189	22.549	5.937	11.225	Fail	5.418	Fail	4.719
1458	5.937	5.373	4.302	5.204	5.187	22.516	5.958	11.220	Fail	5.412	Fail	4.718
1460	5.961	5.394	4.353	5.236	5.210	22.549	5.956	11.238	Fail	5.408	Fail	4.714
1462	5.961	5.373	4.334	5.223	5.187	22.672	5.956	11.272	Fail	5.401	Fail	4.709
1464	5.956	5.373	4.317	5.215	5.197	22.705	5.956	11.286	Fail	5.395	Fail	4.704
1466	5.928	5.371	4.313	5.220	5.203	22.639	5.956	11.266	Fail	5.372	Fail	4.699
1468	5.933	5.392	4.331	5.219	5.221	22.639	5.979	11.280	Fail	5.411	Fail	4.695
1470	5.933	5.348	4.297	5.193	5.175	22.573	5.977	11.242	Fail	5.380	Fail	4.691
1472	5.933	5.369	4.330	5.211	5.190	22.639	5.977	11.269	Fail	5.378	Fail	4.687
1474	5.977	5.371	4.313	5.220	5.192	22.418	5.934	11.181	Fail	5.423	Fail	4.683
1476	5.952	5.390	4.295	5.212	5.190	22.540	5.932	11.221	Fail	5.369	Fail	4.679
1478	5.971	5.348	4.328	5.216	5.185	22.540	5.953	11.226	Fail	5.342	Fail	4.675
1480	5.921	5.388	4.309	5.206	5.165	22.540	5.956	11.220	Fail	5.360	Fail	4.671
1482	5.926	5.367	4.309	5.201	5.163	22.549	5.932	11.215	Fail	5.358	Fail	4.667
1484	5.954	5.388	4.291	5.177	5.119	22.296	5.975	11.149	Fail	5.348	Fail	4.663

Appendix 4
Dynamic Creep Test results

Cycle	Gap Graded			Continuous Graded			Open Graded			Average	
	1	2	3	Average	1	2	3	Average	1	2	3
1486	5.945	5.369	4.307	5.207	5.156	22.385	5.998	11.180	Fail	5.324	33.790
1488	5.966	5.348	4.289	5.201	5.171	22.264	5.956	11.130	Fail	5.383	33.790
1490	5.938	5.344	4.324	5.202	5.170	22.563	5.975	11.236	Fail	5.311	33.601
1492	5.966	5.365	4.305	5.212	5.168	22.409	5.977	11.185	Fail	5.347	33.402
1494	5.938	5.363	4.288	5.196	5.168	22.474	5.977	11.206	Fail	5.321	33.711
1496	5.985	5.342	4.303	5.210	5.124	22.385	5.975	11.161	Fail	5.296	33.523
1498	5.959	5.363	4.301	5.208	5.183	22.442	5.927	11.184	Fail	5.296	33.468
1500	5.912	5.363	4.317	5.197	5.159	22.377	5.951	11.162	Fail	5.289	33.644
1502	5.938	5.363	4.301	5.201	5.178	22.442	5.951	11.190	Fail	5.304	33.468
1504	5.933	5.363	4.351	5.216	5.115	22.329	5.927	11.124	Fail	5.292	33.511
1506	5.908	5.360	4.300	5.189	5.151	22.288	5.949	11.129	Fail	5.293	33.523
1508	5.931	5.363	4.280	5.191	5.112	22.385	5.951	11.149	Fail	5.269	33.457
1510	5.903	5.361	4.299	5.188	5.152	22.409	5.975	11.179	Fail	5.307	33.523
1512	5.926	5.361	4.297	5.195	5.125	22.320	5.951	11.132	Fail	5.256	33.391
1514	5.901	5.361	4.295	5.186	5.164	22.288	5.949	11.134	Fail	5.275	33.511
1516	5.926	5.360	4.295	5.194	5.139	22.320	5.951	11.137	Fail	5.270	33.019
1518	5.976	5.360	4.294	5.210	5.159	22.353	5.972	11.161	Fail	5.223	33.445
1520	5.922	5.360	4.293	5.192	5.139	22.224	5.951	11.105	Fail	5.284	33.379
1522	5.901	5.360	4.293	5.185	5.115	22.368	5.972	11.152	Fail	5.256	32.881
1524	5.899	5.336	4.275	5.170	5.091	22.168	5.951	11.070	Fail	5.250	33.194
1526	5.920	5.358	4.290	5.189	5.112	22.280	5.927	11.106	Fail	5.219	32.945
1528	5.894	5.379	4.291	5.188	5.108	22.136	5.925	11.056	Fail	5.262	33.074
1530	5.913	5.360	4.273	5.162	5.125	22.160	5.925	11.070	Fail	5.267	33.488
1532	5.917	5.354	4.306	5.192	5.127	22.280	5.944	11.117	Fail	5.246	33.129
1534	5.913	5.335	4.288	5.179	5.142	22.192	5.972	11.102	Fail	5.243	33.174
1536	5.913	5.356	4.304	5.191	5.118	22.424	5.946	11.163	Fail	5.218	33.119
1538	5.957	5.358	4.284	5.200	5.098	22.216	5.970	11.095	Fail	5.213	33.119
1540	5.890	5.356	4.286	5.177	5.113	22.160	5.944	11.072	Fail	5.189	33.119
1550	5.906	5.358	4.301	5.190	5.091	22.248	5.970	11.103	Fail	5.180	33.000
1552	5.906	5.331	4.263	5.167	5.078	22.248	5.985	11.108	Fail	5.177	32.936
1554	5.904	5.354	4.284	5.181	5.110	22.033	5.944	11.117	Fail	5.214	33.055
1556	5.899	5.354	4.283	5.180	5.088	22.272	5.991	11.062	Fail	5.144	32.745
1558	5.897	5.331	4.278	5.169	5.056	22.033	5.968	11.019	Fail	5.181	32.154
1560	5.920	5.331	4.277	5.176	5.074	22.120	5.939	11.044	Fail	5.152	32.390
1562	5.902	5.331	4.276	5.160	5.091	22.089	5.991	11.057	Fail	5.192	32.872
1564	5.904	5.354	4.281	5.179	5.078	22.033	5.944	10.993	Fail	5.129	32.626
1566	5.867	5.329	4.292	5.163	5.046	22.002	5.944	10.997	Fail	5.161	32.376
1568	5.893	5.350	4.275	5.173	5.061	22.113	5.965	11.046	Fail	5.156	32.438
1570	5.918	5.348	4.257	5.174	5.041	22.208	5.965	11.071	Fail	5.130	32.673
1572	5.863	5.327	4.273	5.154	5.081	21.883	5.963	10.976	Fail	5.125	32.673

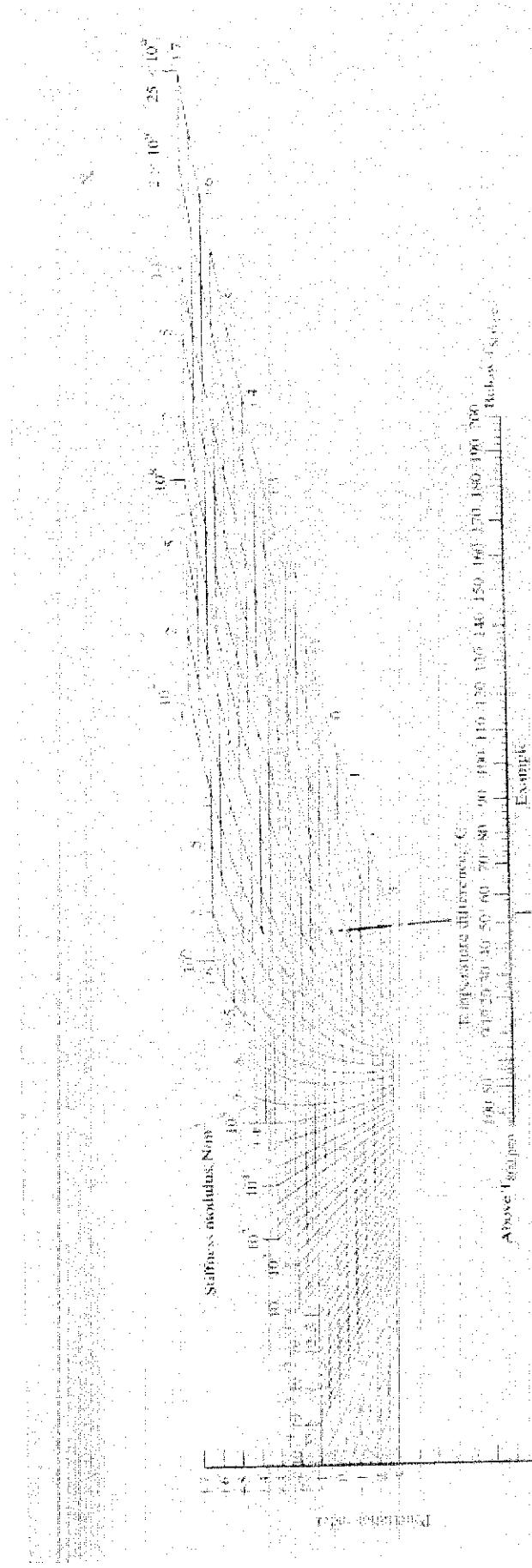
Appendix 4
Dynamic Creep Test results

Cycle	Gap Graded			Continuous Graded			Open Graded			Dense Graded		
	1	2	3	Average	1	2	3	Average	1	2	3	Average
1574	5.881	5.350	4.305	5.179	5.076	22.033	5.916	11.008	Fail	5.138	32.611	20.850
1576	5.884	5.352	4.271	5.169	5.053	21.939	5.939	10.977	Fail	5.113	32.493	20.900
1578	5.884	5.350	4.270	5.168	5.050	22.050	5.965	11.022	Fail	5.106	32.431	20.724
1580	5.881	5.348	4.269	5.166	5.048	22.145	5.942	11.046	Fail	5.123	32.197	20.773
1582	5.879	5.348	4.251	5.159	5.070	21.877	5.963	10.970	Fail	5.138	32.252	20.699
1584	5.905	5.371	4.251	5.176	5.043	21.995	5.963	11.000	Fail	5.095	32.486	20.724
1586	5.877	5.325	4.250	5.151	5.065	21.995	5.942	11.001	Fail	5.110	32.307	20.572
1588	5.886	5.347	4.267	5.167	5.047	21.846	5.918	10.936	Fail	5.083	32.246	20.647
1590	5.875	5.325	4.247	5.149	5.061	21.846	5.916	10.941	Fail	5.076	32.246	20.748
1592	5.875	5.347	4.247	5.156	5.033	21.932	5.939	10.968	Fail	5.073	32.246	20.647
1594	5.847	5.345	4.246	5.146	5.030	21.901	5.961	10.964	Fail	5.068	32.246	20.797
1596	5.872	5.348	4.262	5.161	5.031	21.932	5.939	10.967	Fail	5.060	32.246	20.671
1598	5.872	5.345	4.261	5.159	5.028	21.901	5.961	10.963	Fail	5.036	31.891	20.522
1600	5.891	5.345	4.261	5.166	5.026	21.870	5.911	10.936	Fail	5.035	32.007	20.697
1602	5.872	5.325	4.261	5.153	5.026	21.901	5.961	10.963	Fail	5.045	32.007	20.521
1604	5.891	5.345	4.260	5.165	5.025	21.870	5.961	10.952	Fail	5.041	32.117	20.497
1606	5.866	5.341	4.258	5.155	5.048	21.839	5.937	10.963	Fail	5.037	32.002	20.497
1608	5.864	5.364	4.274	5.167	4.995	21.808	5.937	10.913	Fail	5.012	31.836	20.546
1610	5.868	5.318	4.240	5.142	4.997	21.784	5.914	10.898	Fail	5.008	32.239	20.621
1612	5.832	5.343	4.256	5.144	5.015	21.722	5.937	10.891	Fail	5.022	31.711	20.521
1614	5.861	5.324	4.237	5.141	5.033	21.753	5.914	10.941	Fail	5.019	31.711	20.670
1616	5.852	5.341	4.265	5.149	5.008	21.722	5.937	10.889	Fail	5.009	31.941	20.422
1618	5.834	5.343	4.254	5.144	5.010	21.895	5.937	10.947	Fail	5.007	31.826	20.571
1620	5.880	5.343	4.254	5.159	5.019	21.778	5.911	10.903	Fail	4.979	31.821	20.471
1622	5.867	5.364	4.253	5.161	5.004	21.747	5.958	10.903	Fail	4.956	31.941	20.471
1624	5.827	5.320	4.268	5.138	5.002	21.545	5.984	10.844	Fail	4.994	31.821	20.645
1626	5.852	5.360	4.250	5.154	4.997	21.631	5.935	10.854	Fail	5.004	31.996	20.595
1628	5.852	5.362	4.249	5.154	5.015	21.600	5.958	10.858	Fail	4.981	31.881	20.545
1630	5.827	5.362	4.249	5.146	4.995	21.747	5.956	10.899	Fail	4.995	31.762	20.521
1632	5.850	5.341	4.265	5.162	5.014	21.600	5.980	10.866	Fail	4.969	31.529	20.620
1634	5.825	5.337	4.248	5.137	5.012	21.600	5.958	10.857	Fail	4.965	31.766	20.496
1636	5.850	5.337	4.247	5.145	4.991	21.600	5.982	10.858	Fail	4.961	31.529	20.421
1638	5.821	5.358	4.230	5.136	5.006	21.686	5.958	10.883	Fail	4.977	31.415	20.471
1640	5.818	5.360	4.278	5.152	4.998	21.686	5.909	10.861	Fail	4.949	31.698	20.594
1642	5.839	5.316	4.260	5.138	4.999	21.656	5.905	10.853	Fail	4.945	31.643	20.569
1644	5.821	5.339	4.243	5.134	4.966	21.625	5.911	10.834	Fail	4.938	31.529	20.545
1646	5.839	5.339	4.241	5.140	4.960	21.600	5.909	10.823	Fail	4.919	31.588	20.495
1648	5.835	5.316	4.242	5.131	4.976	21.656	5.933	10.855	Fail	4.910	31.529	20.446
1650	5.828	5.314	4.239	5.127	4.994	21.540	5.956	10.830	Fail	4.926	31.529	20.520
1652	5.868	5.357	4.239	5.151	4.973	21.450	5.933	10.785	Fail	4.878	31.353	20.520
1654	5.835	5.335	4.221	5.130	4.993	21.595	5.930	10.839	Fail	4.894	31.126	20.347
1656	5.812	5.357	4.237	5.136	4.988	21.741	5.933	10.887	Fail	4.907	31.068	20.298
1658	5.810	5.337	4.237	5.128	4.966	21.480	5.954	10.800	Fail	4.904	31.181	20.643
1660	5.833	5.335	4.236	5.135	4.965	21.600	5.926	10.830	Fail	4.880	31.350	20.372

Cycle	Gap Graded			Continuous Graded			open cored			Average	
	1	2	3	Average	1	2	3	Average	1	2	3
1662	5.828	5.313	4.236	5.126	4.983	21.505	5.930	10.806	Fail	4.896	31.295
1664	5.828	5.337	4.234	5.133	4.940	21.480	5.933	10.784	Fail	4.929	31.467
1666	5.826	5.376	4.233	5.145	4.939	21.480	5.909	10.776	Fail	4.881	31.237
1668	5.801	5.311	4.232	5.115	4.957	21.395	5.930	10.761	Fail	4.900	31.009
1670	5.824	5.332	4.248	5.135	4.955	21.335	5.975	10.755	Fail	4.873	31.350
1672	5.845	5.353	4.232	5.143	4.970	21.395	5.954	10.773	Fail	4.845	30.952
1674	5.845	5.355	4.230	5.143	4.951	21.420	5.928	10.766	Fail	4.862	31.179
1676	5.799	5.332	4.228	5.120	4.969	21.420	5.928	10.772	Fail	4.837	31.007
1678	5.819	5.332	4.246	5.132	4.946	21.475	5.930	10.784	Fail	4.815	31.179
1680	5.817	5.332	4.228	5.126	4.964	21.475	5.930	10.790	Fail	4.846	31.122
1682	5.792	5.332	4.243	5.122	4.939	21.505	5.930	10.791	Fail	4.822	31.232
1684	5.811	5.330	4.225	5.122	4.917	21.446	5.928	10.764	Fail	4.833	30.838
1686	5.813	5.330	4.241	5.128	4.938	21.276	5.928	10.714	Fail	4.848	31.232
1688	5.794	5.332	4.225	5.117	4.938	21.505	5.928	10.790	Fail	4.846	30.837
1690	5.790	5.330	4.258	5.126	4.954	21.446	5.954	10.785	Fail	4.802	30.838
1692	5.786	5.330	4.222	5.113	4.948	21.500	5.954	10.801	Fail	4.814	30.837
1694	5.829	5.349	4.221	5.133	4.930	21.441	5.926	10.766	Fail	4.809	31.062
1696	5.850	5.326	4.221	5.132	4.925	21.446	5.924	10.765	Fail	4.803	30.782
1698	5.827	5.328	4.220	5.125	4.904	21.272	5.952	10.709	Fail	4.799	30.949
1700	5.804	5.330	4.219	5.118	4.961	21.441	5.949	10.784	Fail	4.797	31.004
1702	5.783	5.326	4.237	5.115	4.920	21.416	5.928	10.765	Fail	4.812	30.781
1704	5.781	5.326	4.234	5.114	4.942	21.386	5.952	10.760	Fail	4.784	30.669
1706	5.779	5.328	4.218	5.108	4.974	21.247	5.924	10.715	Fail	4.778	30.669
1708	5.775	5.307	4.217	5.100	4.914	21.323	5.926	10.721	Fail	4.756	30.502
1710	5.829	5.347	4.217	5.121	4.913	21.100	5.902	10.638	Fail	4.788	30.892
1712	5.821	5.326	4.217	5.121	4.931	21.298	5.926	10.718	Fail	4.781	30.558
1714	5.793	5.326	4.213	5.111	4.906	21.298	5.949	10.718	Fail	4.758	30.724
1716	5.770	5.344	4.214	5.109	4.923	21.184	5.971	10.693	Fail	4.753	30.502
1718	5.770	5.323	4.195	5.096	4.924	21.298	5.949	10.724	Fail	4.766	30.724
1720	5.768	5.324	4.212	5.101	4.884	21.298	5.924	10.702	Fail	4.721	30.033
1722	5.798	5.324	4.226	5.116	4.920	21.100	5.947	10.656	Fail	4.737	30.779
1724	5.812	5.303	4.209	5.108	4.916	21.269	5.947	10.711	Fail	4.726	30.668
1726	5.766	5.321	4.194	5.094	4.916	21.155	5.947	10.673	Fail	4.706	30.558
1728	5.764	5.321	4.209	5.098	4.913	21.269	5.947	10.710	Fail	4.718	30.668
1730	5.787	5.342	4.205	5.117	4.881	21.210	5.898	10.687	Fail	4.713	30.613
1732	5.762	5.323	4.206	5.097	4.910	21.155	5.900	10.655	Fail	4.710	30.558
1734	5.760	5.300	4.206	5.089	4.909	21.014	5.945	10.623	Fail	4.684	30.613
1736	5.785	5.342	4.204	5.110	4.923	21.014	5.924	10.620	Fail	4.697	30.890
1738	5.805	5.342	4.205	5.117	4.881	21.152	5.945	10.659	Fail	4.691	30.337
1740	5.787	5.296	4.219	5.101	4.864	21.097	5.921	10.627	Fail	4.686	30.613
1742	5.760	5.300	4.219	5.093	4.862	21.210	5.945	10.672	Fail	4.663	30.448
1744	5.755	5.298	4.202	5.085	4.896	21.069	5.919	10.628	Fail	4.676	30.448
1746	5.755	5.321	4.219	5.098	4.854	21.181	5.919	10.651	Fail	4.650	30.393
1748	5.799	5.300	4.201	5.100	4.871	21.181	5.898	10.650	Fail	4.680	30.228

Cycle	Gap Graded			Continuous Graded			Open Graded					
	1	2	3	Average	1	2	3	Average	1	2	3	Average
1750	5.799	5.298	4.167	5.088	4.869	21.123	5.898	10.630	Fail	4.677	30.448	19.938
1752	5.749	5.319	4.182	5.083	4.869	21.069	5.945	10.628	Fail	4.636	30.558	19.940
1754	5.751	5.298	4.198	5.082	4.884	21.210	5.921	10.672	Fail	4.626	30.229	19.987
1756	5.749	5.338	4.181	5.089	4.866	21.040	5.945	10.617	Fail	4.624	30.339	19.870
1758	5.749	5.338	4.181	5.089	4.845	21.236	5.924	10.668	Fail	4.653	30.448	19.821
1760	5.774	5.296	4.196	5.089	4.879	21.011	5.919	10.603	Fail	4.629	30.339	19.940
1762	5.792	5.317	4.195	5.101	4.860	21.069	5.919	10.616	Fail	4.624	30.068	19.773
1764	5.763	5.315	4.195	5.091	4.865	21.152	5.945	10.651	Fail	4.583	30.068	19.868
1766	5.811	5.294	4.227	5.111	4.875	21.094	5.895	10.621	Fail	4.630	30.178	19.798
1768	5.774	5.315	4.226	5.105	4.852	21.037	5.940	10.610	Fail	4.610	30.068	19.775
1770	5.765	5.315	4.207	5.096	4.867	20.897	5.942	10.589	Fail	4.602	30.123	19.895
1772	5.763	5.338	4.176	5.092	4.888	20.925	5.919	10.577	Fail	4.600	30.014	19.985
1774	5.763	5.334	4.207	5.101	4.845	20.840	5.895	10.527	Fail	4.600	30.232	19.729
1776	5.745	5.336	4.175	5.085	4.859	20.871	5.895	10.542	Fail	4.599	30.016	19.871
1778	5.740	5.312	4.174	5.075	4.837	20.897	5.895	10.543	Fail	4.578	30.286	19.848
1780	5.779	5.315	4.188	5.094	4.840	21.008	5.940	10.596	Fail	4.598	30.233	19.680
1782	5.734	5.313	4.203	5.083	4.839	20.925	5.917	10.560	Fail	4.596	30.071	19.870
1784	5.761	5.312	4.188	5.087	4.855	21.008	5.942	10.602	Fail	4.594	30.016	19.777
1786	5.736	5.291	4.186	5.071	4.856	20.842	5.945	10.548	Fail	4.612	29.908	19.777
1788	5.757	5.292	4.186	5.078	4.852	21.008	5.940	10.600	Fail	4.573	29.802	19.705
1790	5.777	5.291	4.202	5.090	4.812	20.897	5.942	10.550	Fail	4.606	30.018	19.682
1792	5.734	5.312	4.185	5.077	4.832	20.869	5.940	10.547	Fail	4.591	30.286	19.543
1794	5.752	5.312	4.200	5.088	4.804	20.952	5.917	10.558	Fail	4.588	29.857	19.588
1796	5.798	5.310	4.182	5.097	4.825	20.840	5.917	10.527	Fail	4.570	29.857	19.637
1798	5.748	5.333	4.182	5.088	4.822	20.980	5.938	10.580	Fail	4.587	29.805	19.565
1800	5.725	5.291	4.181	5.046	4.838	20.923	5.893	10.551	Fail	4.605	29.752	19.591

APPENDIX 5: Nomograph for stiffness modulus of bitumen



The stiffness modulus is defined as the ratio of stress to strain at a given

time of loading, i.e., stiffness = stress/strain = $E = \frac{\sigma}{\epsilon}$

where σ is the stress and ϵ is the strain.

This is obtained by extrapolating the experimental log penetration

versus temperature law to the penetration value 0.00.

At low temperatures and at high frequencies the stiffness modulus

of all bitumens asymptotically approaches 10^8 N/mm^2 .

Example:

Operating conditions

Temperature $T_1 = 11^\circ\text{C}$
Loading time $t = 200$ seconds

Characteristics of the bitumen sample

Two test temperatures at which the penetration

is 0.001 mm

(Penetration thickness)

Contact Q.D. at lower test temperature

Difference $\Delta T = 15^\circ\text{C}$ in temperature scale

Record drift between drift $\Delta t = 0$

Stiffness, N/mm^2	Time of loading, sec.	Temperature difference, $^\circ\text{C}$
10^3	0	0
10^4	10	50
10^5	20	100
10^6	30	150
10^7	40	200
10^8	50	250
10^3	100	300
10^4	110	350
10^5	120	400
10^6	130	450
10^7	140	500

The stiffness of the bitumen is recorded when the operating conditions

are met. The stiffness is read off the left scale.

For example, if the operating conditions are $T_1 = 11^\circ\text{C}$, $t = 200$ sec., $\Delta T = 15^\circ\text{C}$, $\Delta t = 0$, the stiffness is 10^6 N/mm^2 .

If the operating conditions are $T_1 = 11^\circ\text{C}$, $t = 200$ sec., $\Delta T = 15^\circ\text{C}$, $\Delta t = 10$ sec., the stiffness is 10^7 N/mm^2 .

If the operating conditions are $T_1 = 11^\circ\text{C}$, $t = 200$ sec., $\Delta T = 15^\circ\text{C}$, $\Delta t = 20$ sec., the stiffness is 10^8 N/mm^2 .

If the operating conditions are $T_1 = 11^\circ\text{C}$, $t = 200$ sec., $\Delta T = 15^\circ\text{C}$, $\Delta t = 30$ sec., the stiffness is 10^9 N/mm^2 .

If the operating conditions are $T_1 = 11^\circ\text{C}$, $t = 200$ sec., $\Delta T = 15^\circ\text{C}$, $\Delta t = 40$ sec., the stiffness is 10^{10} N/mm^2 .

If the operating conditions are $T_1 = 11^\circ\text{C}$, $t = 200$ sec., $\Delta T = 15^\circ\text{C}$, $\Delta t = 50$ sec., the stiffness is 10^{11} N/mm^2 .

If the operating conditions are $T_1 = 11^\circ\text{C}$, $t = 200$ sec., $\Delta T = 15^\circ\text{C}$, $\Delta t = 60$ sec., the stiffness is 10^{12} N/mm^2 .

If the operating conditions are $T_1 = 11^\circ\text{C}$, $t = 200$ sec., $\Delta T = 15^\circ\text{C}$, $\Delta t = 70$ sec., the stiffness is 10^{13} N/mm^2 .

If the operating conditions are $T_1 = 11^\circ\text{C}$, $t = 200$ sec., $\Delta T = 15^\circ\text{C}$, $\Delta t = 80$ sec., the stiffness is 10^{14} N/mm^2 .

If the operating conditions are $T_1 = 11^\circ\text{C}$, $t = 200$ sec., $\Delta T = 15^\circ\text{C}$, $\Delta t = 90$ sec., the stiffness is 10^{15} N/mm^2 .

If the operating conditions are $T_1 = 11^\circ\text{C}$, $t = 200$ sec., $\Delta T = 15^\circ\text{C}$, $\Delta t = 100$ sec., the stiffness is 10^{16} N/mm^2 .

If the operating conditions are $T_1 = 11^\circ\text{C}$, $t = 200$ sec., $\Delta T = 15^\circ\text{C}$, $\Delta t = 110$ sec., the stiffness is 10^{17} N/mm^2 .

If the operating conditions are $T_1 = 11^\circ\text{C}$, $t = 200$ sec., $\Delta T = 15^\circ\text{C}$, $\Delta t = 120$ sec., the stiffness is 10^{18} N/mm^2 .

If the operating conditions are $T_1 = 11^\circ\text{C}$, $t = 200$ sec., $\Delta T = 15^\circ\text{C}$, $\Delta t = 130$ sec., the stiffness is 10^{19} N/mm^2 .

If the operating conditions are $T_1 = 11^\circ\text{C}$, $t = 200$ sec., $\Delta T = 15^\circ\text{C}$, $\Delta t = 140$ sec., the stiffness is 10^{20} N/mm^2 .

If the operating conditions are $T_1 = 11^\circ\text{C}$, $t = 200$ sec., $\Delta T = 15^\circ\text{C}$, $\Delta t = 150$ sec., the stiffness is 10^{21} N/mm^2 .

If the operating conditions are $T_1 = 11^\circ\text{C}$, $t = 200$ sec., $\Delta T = 15^\circ\text{C}$, $\Delta t = 160$ sec., the stiffness is 10^{22} N/mm^2 .

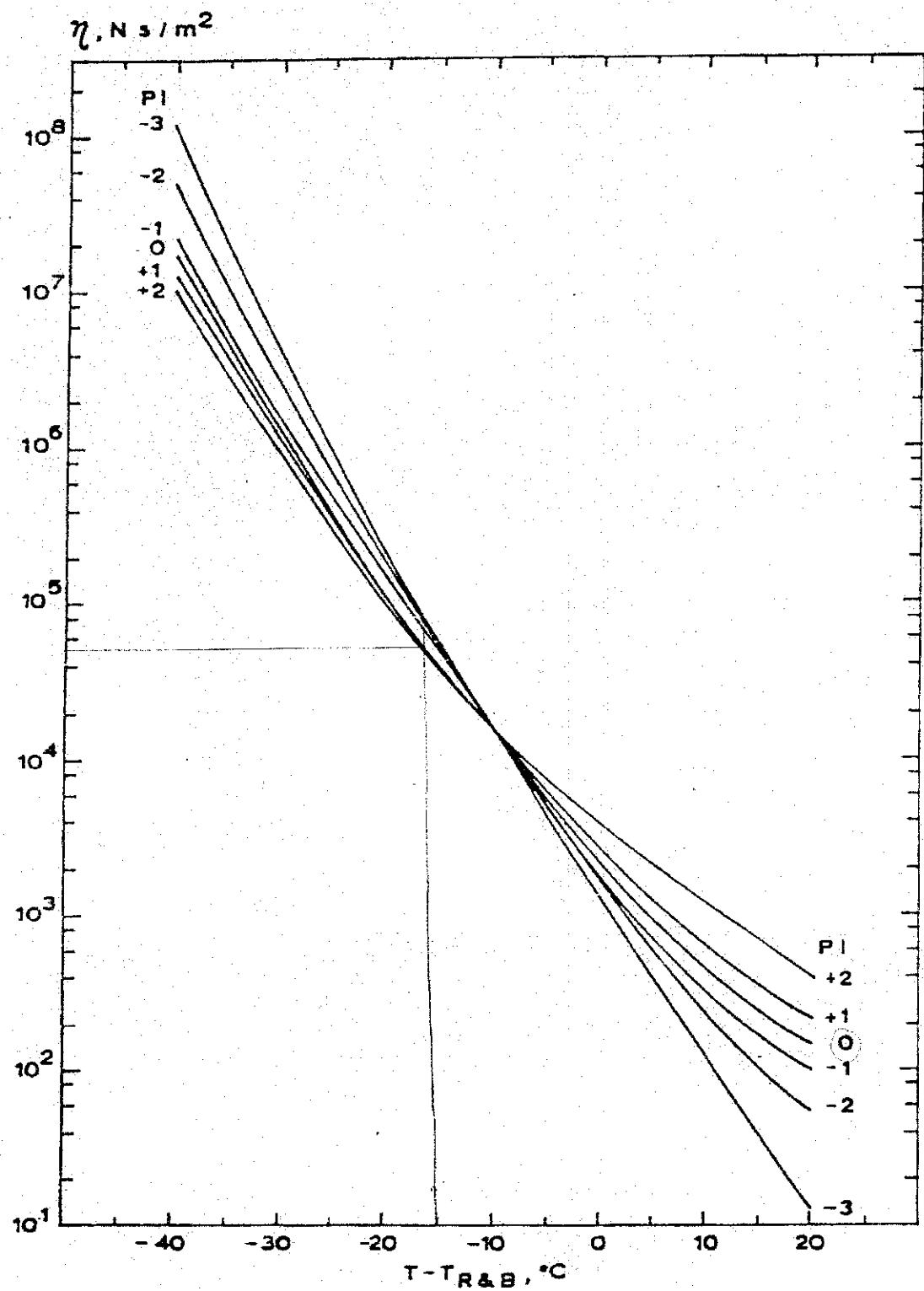
If the operating conditions are $T_1 = 11^\circ\text{C}$, $t = 200$ sec., $\Delta T = 15^\circ\text{C}$, $\Delta t = 170$ sec., the stiffness is 10^{23} N/mm^2 .

If the operating conditions are $T_1 = 11^\circ\text{C}$, $t = 200$ sec., $\Delta T = 15^\circ\text{C}$, $\Delta t = 180$ sec., the stiffness is 10^{24} N/mm^2 .

If the operating conditions are $T_1 = 11^\circ\text{C}$, $t = 200$ sec., $\Delta T = 15^\circ\text{C}$, $\Delta t = 190$ sec., the stiffness is 10^{25} N/mm^2 .

If the operating conditions are $T_1 = 11^\circ\text{C}$, $t = 200$ sec., $\Delta T = 15^\circ\text{C}$, $\Delta t = 200$ sec., the stiffness is 10^{26} N/mm^2 .

APPENDIX 6



Viscosity of bitumen as a function of $(T - T_{R\&B})$ and PI

PENDIX 7

in nomograph, get the Stiffness modulus of bitumen, Sbit

For 80 pen bitumen, the softening point is 44 °C
 Dynamic creep test operating temperature is 40 °C
 Thus, the temperature difference is 4 °C
 Iteration index is at 0

Mix average is taken from Dynamic Creep Test result at specified cycles.

Cycles	Time of loading (s)	Smix average (Mpa)				Sbit (Mpa)
		Gap	Continuous	Open	Dense	
10	20	19.562	44.531	29.904	150.433	1.50E-03
20	40	15.275	35.38	19.761	117.495	1.00E-03
30	60	13.365	30.989	17.479	103.902	7.50E-04
60	120	10.873	25.259	14.338	82.997	5.00E-04
180	360	8.126	18.848	10.63	56.016	1.00E-04
420	840	6.618	15.273	7.947	40.488	8.00E-05
600	1200	6.115	14.028	6.761	34.908	7.00E-05
1200	2400	5.356	11.831	5.327	25.576	1.05E-05
1800	3600	5.066	10.551	4.605	20.786	1.00E-05

Graph Smix average vs. Sbit is plotted based on the above table (Figure 31).

Id Stiffness modulus of bitumen viscosity, (Sbit)v

$$\text{Sbit } v = \frac{3\eta}{NT} \quad \text{where} \quad \eta \text{ from Graph viscosity of bitumen (Appendix 6),}\\ \text{get } 5 \times 10^3 \\ N = \text{number of wheels passes standard axle, in million} \\ T_w = \text{loading time (0.02 s)}$$

: For $N = 1$

$$\text{Sbit}v = \frac{3 * (5E-03)}{(1)^*(0.02)} = 0.75$$

The new Smix is calculated based on the linear equation shown in Figure 31.

Mix, y:

Gap	$y = 81.663(x)^{0.2500}$	where $x = (\text{Sbit})v$
Continuous	$y = 208.93(x)^{0.2635}$	
Open	$y = 196.37(x)^{0.3283}$	
Dense	$y = 1467.4(x)^{0.3681}$	

For $N = 1$, $(S_{bit})v = 0.75$

Gap $y = 81.663(7.50E-01)^{0.25} = 75.996$
 Continuous $y = 208.93(7.50E-01)^{0.2635} = 193.678$
 Open $y = 196.37(7.50E-01)^{0.3283} = 178.673$
 Dense $y = 1467.4(7.50E-01)^{0.3681} = 1319.953$

N	Smix (Mpa)				$(S_{bit})v$ (Mpa)
	Gap	Continuous	Open	Dense	
1.E+00	75.996	193.678	178.673	1319.953	7.50E-01
1.E+01	42.736	105.579	83.899	565.534	7.50E-02
1.E+02	24.032	57.555	39.397	242.303	7.50E-03
1.E+03	13.514	31.375	18.499	103.815	7.50E-04
1.E+04	7.600	17.103	8.687	44.479	7.50E-05
1.E+05	4.274	9.324	4.079	19.057	7.50E-06
1.E+06	2.403	5.083	1.915	8.165	7.50E-07
1.E+07	1.351	2.771	0.899	3.498	7.50E-08
1.E+08	0.760	1.510	0.422	1.499	7.50E-09

Depth calculation:

$$d = C_m \times H \times \left(\frac{\sigma_{av}}{S_{mix}} \right)$$

where C_m = dynamic efficiency (1.0 - 2.0), take 1.5
 H = height of pavement between 60 - 70 mm, take 65 mm
 σ_{av} = 2.5 Mpa

: For $N = 1$

Gap $R_d = (1.5 \times 65 \times 2.5) / 75.996 = 3.207$ mm
 Continuous $R_d = (1.5 \times 65 \times 2.5) / 193.678 = 1.259$ mm
 Open $R_d = (1.5 \times 65 \times 2.5) / 178.673 = 1.364$ mm
 Dense $R_d = (1.5 \times 65 \times 2.5) / 1319.953 = 0.185$ mm

N	Rut depth (mm)			
	Gap	Continuous	Open	Dense
1.E+00	3.207	1.259	1.364	0.185
1.E+01	5.704	2.309	2.905	0.431
1.E+02	10.143	4.235	6.187	1.006
1.E+03	18.037	7.769	13.176	2.348
1.E+04	32.074	14.252	28.060	5.480
1.E+05	57.037	26.144	59.757	12.790
1.E+06	101.427	47.958	127.259	29.853
1.E+07	180.366	87.976	271.011	69.676